## LAC STE. ANNE COUNTY PROVINCE OF ALBERTA BYLAW \#28-2017

## A BYLAW TO CONTROL LAND USE.

Whereas, under the provisions of the Municipal Govermment Act, being Chapter M-26.1, Division S, Sections 639 and 640 of the Revised Statutes of Alberta 2000 R.S.A, a municipality may adopt an Area Structure Plan.

AND Whereas the Council of Lac Ste. Anne County has decided to amend the Island View Area Structure Plan as a means to clarify the development use.

NOW THEREFORE the Council duly assembled hereby enacts as follows:

1. Lac Ste. Amene County Byluw 10-2013 is hereby amended in accordance with attached Schedule "A":
2. That this Bylaw comes into full force and effect upon third reading of this Bylaw and the registration of the amended Condominium Bylaw.

First Reading carried the $4^{\text {th }}$ day of October, A.D. 2017.


Read a second time the $1^{19}$ day of November, AD. 2017.


Read a thind and final time this the $1^{8 x}$ day of November, A.D. 2017.


SCHFDTILE: ***
byLAW NO. 2H-2017
A) Pravidonits be adandad

That the Eollanwing bectoras be arexded as fedouns:

## SECTION S.0 Devdopmeat PJan





## Sexflon 5.4 Skuronality


 culendar your.



 oftertating streets to traximize solar griv and reduce worthery exposures.

 preitut of $t=$ ary.
 use fuite envirnmert.


### 6.0 CONCEPT PLAN


 inflastructare for the Plan Ater

It is acknowkedged that matin of hezlan aras it considered a: la:wh possessing limited


 use oft the lands.


 arco to take advantage of uggaded mitas nucure.
 Figue S (tand Uye Gonceg: Hund.

 Mrnide









## 0. 1 PHAふEり DEVEDATMEYT





- Aluricel condidors.




## rnatageneryt and shallow untiry installacion.

 developmerl and the limitations of firmot-foading fingrainl ablipations.

- The completinn of tion Resonvea Fxt:actica Remilaration Plan propered by Aspen C.and Otonp lace, on hehait of Weranck Aggegates Lid. dated Ducterbes 20:3, ataghed twerith, and references as Alpendix " $E$ '.


### 6.2 PLAN PGLICIES
















6. 3 LAND ISF.

### 6.3.1 LAND USE P(JIICIRS

Newiliostanding the atove, an titu-c subdivikion ani devele.pment within the Plon Atea

Dasthopment Pian

 Wse Concept Pans.









 bodiks and enviranatifity seasitive areas

 Comidy anay *









 as will res fred Cortept Phent

## 

cosl end experse of the Dowalapor.

Use Conkepp P?(ri).
6. 4.1.4 The Deve: oper shall iff uecessaly] dext.cate lar.d fir the upprading of the couch of intersection of tre propoged inhriol read wht Rerge Roac 20 as


### 6.5 ENYIRONMENTALLY SLGNIFICANT FEATURES


 envinumental fealrrea.

### 6.5.1 ENYIROMMENTALLY SIMUSBICAMT FEATLR

 GLSTADMABLTY POLICIEA
 mproach to devedikneul.類




*)


 north bespinary of their lot c prowide asd:tianal protecticn tras the Matlens
6.5.1.5 Wher,


### 6.5 MURICIPAL RESERVE KPPEN SPACE

If: Laubch Veutures \&
 conezuous craí system





## 6．7」 POTADLE WATER FOLICTES



## 673 SAMITARYSEXER SYSTEM




## 6．73．1 SANTTARY SEWER SXSTEM PGIHELES



 fidtires．

 ＇ Bl 山⿱一⿻上丨⿱⿰㇒一乂心，









 ured to codet



 telp with scatan wase：guality：

## S＇IULIX WAILR MANAGEMENT POTICLFS


 satiafaction of the－ac Ste Amue Coun＝r．

## 


 clliont ixa animimus.
 and plentivy led merarial.

 provided for cach loe created.




 TABLE OF CONT'LVIS
L. 1 DTRODIECTION ..... 1
:. 1 Plaat Jurisuiclion ..... 1
:. 2 Flan Cumpliarsa. ..... 1
: 3 Key Elements of the pima ..... 2
i. 4 Polisy Interprelation .....  2
:. 5 FLar Amendments ..... 3
i. 6 Mup Isterpretation: .....
:. 7 Consigtency fuld Mirnitcriay of the Der: ..... 3
2.0 MXAN AKEA ..... s
Z. 1 Regicsal Context ..... 4
2. 0 EXISTING SITR FT:A TLTES ..... 5
3.1 SìeChrracteristics .....  .5
3.2 Drainage Bastu .....  5
1.3 Pipeltres if Rip̣hts of Wray ..... $\underline{2}$
3.4 Adiacent '.and [rems ..... 5
1.5 Euldings ..... 6
?, (i) Vegeratime \& Suilu ..... 6
3.7 Scricial Goology .....  6
3.8 Evircng Tranconisdon Ketwnk ..... 6
3.9 Ascess ..... 7
3. 11 Hajur Frantoried Uibinies ..... ${ }^{7}$
3.12 Tencumbrancet: .....  .7
4.0 STR $\$$ TENTY ..... ?
4.1 Flen K:irkipulb ..... 7
4? Plumerex ..... 8
5.0 PLAN OBNECIIVES .....  .8
6.0 GOKCLIFTPLAN ..... 13
6.1 Plas Policita ..... 13
t. 2 Pb:afed Derelypmext ..... 1
63 Land Use. ..... 14
5.4 Trampentaninn Nework ..... 15
 ..... 16
6.6i Minaicigel Kromye ..... !
द. 7 Servicinq̣ Tлfi:kтח:етиге ..... 17
kipures

[^0]Ftanee 1 Acgionai Contical
Figure 1 Hiar. Acta

4 Pipelines and Righte oi Way Pigure 3
Lamd Us: Concept
Figare if Stanim Prite: Minngentend Plex

## ryperalizx

## Aprodis ' $A$ '

Riophyaical Liverelory, S.E. I-54-2-WIM. danuncy, 2016. Piepased by MCA Esmiennerazi Mangetmant dated Javany 20 ! C .

## Appendix ' $B$ '




## Apperdix 'C'

 dated Decembar 73. 2015.

## Ayprulir 'D:




## Appendix ' $\mathbf{E}$ '


 Group Irc. dated Deventre: 20:3.

### 1.0 PURPOSE


 ']'ennchic 54, Mange 2, Whest of the Sill Meidiar. wich lies :o the west of Range Rond 20












1.1 PLAN AREA JURISDICTIOS

> 多


- <


1.2 PLAN COWh




 crabling the areation of Atea Sl:uctine Pluse slewa:

 plar_
(2) Autatistrclure pitu
(a) must deccribe
(i) and sepuence of deverop. neut proposed for the area;
 resyect to specitic parts of tie area;
(iii) the deasity of prap;ilatinapropod with respest to eperific patis effitiontra:
 untilites, and



### 1.3 KEY RLEMENTS OF THE PLAN















 lise geared intent of tive policy is still achieved.



 achieve the generil irimen if the palisy.

### 1.5 PLAN ARIBNDMENIE

 to the Plan pill ke required to be approved ioy Hylaw, An anendmemi wi!l require the unlding
 procoluce establisind by Las ste ime Corncy.

Where an amendment in the Pint is trinested, the applicant rill 'oe requised to su'smit:

 reanid accurate.
1.6 MAP DTERRRETATION



 nighas.uf way.

### 1.7 CANSISTENCY AND MONI解RING OE THE PLHEN








### 2.0 PLAFA AREA

## 24 RECIONAL CONTEXT



 Ancedotally, Wetaskivin is a stage coash dop fetween Fdongnu:ncud Cilguy.

 rivers, and alose to dut twedrlils.




## 3.U EXIST'LNE: SITE FEATURES \& CHARACTERISTICS


 Rraiderdial / Publ: 2 L/tilitics, as.d Respeve.

### 3.1 SITE CHARACTERISTICS





 re:mander of the flan area.

### 3.2. DRAINAGE BASTN


















 unciiuteol abong the sonth roundary of the plan etes, is obined operated by the tac Ste Ange
 losatsf within the plan area

### 3.4 LXISTTNG USE OF LARDOS

 use reflects the existrnce of a provel txtaction Tporition which bas ceased to exis: and is awairiag rectamaion as fer an reciarcintion penvit



㫙prowed an indertrial storage frcirity cn the gite as a dirirctionary use．The balante of the lated is in agritelthrat production．

## T．$\overline{3}$ ．AD．AAC．ANT LURD USES



 culcixated lank

agmegate extracton egeraion．

## 












䝜

 thichess，mune fipeskik beds．

紋年

## 3．4 F．XISTING TR．ANBPORTATION NETWORK




 to an｀all worllen traxed aludurd．

### 3.9 ACChs

 divect aceess is Ruege Rood 20 whice pisess to be bularie of the pared as gainet via
 plate .eter

### 3.10 MANOK KIKANLHISED LTTLLTES


 Live for elatatul setricitive.





### 3.11 ENT!IJMBERANCES


 frture resiforphat develj



### 4.0 Sl'EATEGY

### 4.1 PLAN PRDNCIPLES

### 4.1.1 SUSTAINABLE DEVKIOPMEXT


 and exirozical indeyrity.

 lests well inlo the rithere.



 features of signifignote





 yadely ispatst ard fortmos


 Coumly in a manner ing linitz the sexatido of new development on tive County's existing

 oíserviec zani the Comby of lac Ste Alith.



### 4.1.5 GEOGLAYHICAL JMEORIANCE AND YISLAL APPEAL



- Frouring tiat the niture of devolopinetd is ore thal roflects

* Developictut eqpotiaes the valces of a prosc, prisperpus; heallhy, and vibeant cornatrir::

 coviruntranali, swaimab:


### 4.2 FLAN FROCEBS



 Lis Plon.

## 

 cruvermantale


Five rows






, tiss

- To ilentify upplicable Iepisintirn; and



### 42.2 GEOIECHNLCAL LWYESTIGATIOH



 assetsment were:










## 

(4)


 develupiueat



$4 *$.




### 4.2.4 HYDAYAOGIGATASSESEMENT





### 5.0 PLAN OBJECTIVES

In the preparation of the ASP a detailed review of existing planning policy was conducted to ensure alignment with Lac Ste Anne County planning and policy principles. Without restricting the generality of the foregoing the following documents were referenced within the ASP:

- Lac Ste Ame County Land Use Bylaw;
- Lae Ste Anne County Municipal Development Plan;
- Lac Ste Anne County Development and Design Standards;
- Lae Ste Anne County Corporate Strategic Plan;
- Lac Ste Anne Rural Road Study;
- Lac Ste Anne Master Transportation Plan;

Having regard for the Lac Ste Anne County planning and policy principles and based on the premise of the triple bottom line as the foundation for creating a sustainsble developuent the objectives of the ASP are to:

- Prepare a future development concept for the plan area and provide policy direction describing the manner in which land may be developed.
- Promote a transportation network that includes multiple modes of transportation which shall include automobiles, cyeling and walking.
- Identify a strategy for providing open space and trails, including linkage to the existing open spaces and trail system and promote interconnected road and path systems that facilitate efficient provision of municipal services und maintenance.
- Provide for the efficient and phased conceptual design of development.
- Identify lands suitable for pablic recreational opporrunities and include both active and passive recreational opportunities.
- Ensure proper protection for environmentally sensitive areas.
- Recognize and maintain the landscape and other environmental qualities of the plan area.
- Promote the diversification of the local economy by examining economic opportunities and economic viability.
- Incorporate diversity of use that may include and promote home, work and play in close proximity.


## CLEARVIW RIDGE

AREA

## STRUCTURE PLAN

JANUARY 2016

# CLEARVIEW RIDGE AREA STRUCTURE PLAN 

## Within the

S.E. 1-54-2-5

## COUNTY OF LAC ST ANNE

## PREPARED FOR

LAUNCH VENTURES LTD.

BY

Robert Wescott, B.Sc. AICP

Wescott Consulting Group Ltd.
CLEARVIEW RIDGE AREA STRUCTURE PLAN - FEBRUARY 2016 TABLE OF CONTENTS
1.0 INTRODUCTION ..... 1
1.1 Plan Jurisdiction ..... 1
1.2 Plan Compliance ..... 1
1.3 Key Elements of the Plan ..... 2
1.4 Policy Interpretation ..... 2
1.5 Plan Amendments .....  3
1.6 Map Interpretation ..... 3
1.7 Consistency and Monitoring of the Plan ..... 3
2.0 PLAN AREA ..... 4
2.1 Regional Context ..... 4
3.0 EXISTING SITE FEATURES ..... 5
3.1 Site Characteristics ..... 5
3.2 Drainage Basin ..... 5
3.3 Pipelines \& Rights of Way ..... 5
3.4 Adjacent Land Uses ..... 5
3.5 Buildings ..... 6
3.6 Vegetation \& Soils ..... 6
3.7 Surficial Geology ..... 6
3.8 Existing Transportation Network ..... 6
3.9 Access .....  7
3.11 Major Franchised Utilities ..... 7
3.12 Encumbrances ..... 7
4.0 STRATEGY ..... 7
4.1 Plan Principals ..... 7
4.2 Plan Process ..... 8
5.0 PLAN OBJECTIVES ..... 8
6.0 CONCEPT PLAN ..... 13
6.1 Plan Policies ..... 13
6.2 Phased Development ..... 13
6.3 Land Use ..... 14
6.4 Transportation Network ..... 15
6.5 Environmentally Significant Features ..... 16
6.6 Municipal Reserve ..... 16
6.7 Servicing Infrastructure ..... 17

Figure 1 Regional Context
Figure 2 Plan Area
Figure 3 Topographical Features
Figure 4 Pipelines and Rights of Way
Figure 5 Land Use Concept
Figure 6 Storm Water Management Plan

## Appendices

## Appendix ' $A$ '

Biophysical Inventory, S.E. 1-54-2-W5M, January, 2016. Prepared by MCA Environmental Management, dated January 2016.

## Appendix ' $B$ '

Aggregate Characterization Assessment for Wes Erickson, S.E. 1-54-2W5M prepared by Twerdoff \& Associates Inc., and dated June 2014.

## Appendix 'C'

Traffic Impact Assessment, D\&A Paulichuk Consultants Ltd. S.E. 1-54-2-W5M dated December $7^{\text {th }}, 2015$.

## Appendix 'D'

Hydrological Assessment for W.E. 1-54-2-W5M prepared by Groundwater Information Technologies Ltd., dated May 6 ${ }^{\text {th }}, 2015$.

## Appendix ' $E$ '

Pit Registration Application, Yeoman/Parker Pit in the E 1.2 Section 1, Township 54, Range 2, West of the $5^{\text {th }}$ Meridian prepared by Aspen Land Group Inc. dated December 2013.

### 1.0 PURPOSE

The Clearview Ridge Area Structure Plan provides for the orderly and economic approach to the subdivision and development of the lands within that portion of the S.E. $1 / 4$ Section 1, Township 54, Range 2, West of the 5th Meridian which lies to the west of Range Road 20 and north of Township Road 540, (commonly referred to as the Heatherdown Road).

The Clearview Ridge Area Structure Plan is intended to identify key issues such as land use, servicing, aesthetic design, transportation network and reserve issues, and to provide viable options in the solution of those issues.

This Area Structure Plan is intended to establish a process of sequencing to ensure that development occurs in a logical, efficient and sequential manner.

### 1.1 PLAN AREA JURISDICTION

The Plan Area (as shown in Figure 1 Regional Context) is located within the municipal jurisdiction of Lac Ste Anne County. This plan consists of policy statements and conceptual representations that provide the framework to promote the following principles:

- Promote sustainable development;
- Promote the expansion of the tax base within Lac Ste Anne County;

Acknowledge and promote the development potential of the lands while recognizing the geographical importance within both the local and regional context;

- Create a visually appealing mixed use built environment.


### 1.2 PLAN COMPLIANCE

This Area Structure Plan hereinafter referred to as the Plan Area', has been prepared as a requirement of Lac Ste Anne County and in particular the Municipal Development Plan \# 23-2014 which requires that such plans be prepared for select study areas.

The Area Structure Plan is also prepared in accordance with the requirements as stipulated of the Municipal Government Act. The specific legislation under Section 633 of the MGA enabling the creation of Area Structure Plans states:
(1) For the purpose of providing a framework for subsequent subdivision and development of an area of land, a council may, by bylaw, adopt an area structure plan.
(2) An area structure plan
(a) must describe
(i) the sequence of development proposed for the area;
(ii) the land uses proposed for the area, either generally or with respect to specific parts of the area;
(iii) the density of population proposed for the area either generally or with respect to specific parts of the area;
(iv) the general location of major transportation routes and public utilities, and
(b) may contain any other matters the council considers necessary.

### 1.3 KEY ELEMENTS OF THE PLAN

The two key elements in the Area Structure Plan are:

- A process that is structured to coordinate development intensification in concert with transportation capacities, environmental compatibility and servicing capabilities.
- Policy Statements supplemented with a Land Use Concept Plan to establish a logical and sequential pattern of development.


### 1.4 POLICY INTERPRETATION

The explanatory text accompanying a policy within the Plan is provided for information purposes only to enhance the understanding of the policy. If an inconsistency arises between this text and a policy, the policy will take precedence.

Where "shall" is used in a policy, the policy is considered mandatory. However, where actual quantities or numerical standards are contained within the policy, such quantities or standards may be varied, provided that the variance is necessary to address unique circumstances that would otherwise render compliance impractical or impossible, and the general intent of the policy is still achieved.

Where "should" is used in a policy, the intent is that the policy is to be complied with. However, the policy may be varied in a specific situation provided that the variance is necessary to address unique circumstances that will otherwise render compliance impractical or impossible, or to introduce an acceptable alternate means to otherwise achieve the general intent of the policy.

### 1.5 PLAN AMENDMENTS

In order to amend this Plan, including any changes to the text or maps within, an amendment to the Plan will be required to be approved by Bylaw. An amendment will require the holding of a statutory public hearing together with public notification carried out in accordance with procedures established by Lac Ste Anne County.

Where an amendment to the Plan is requested, the applicant will be required to submit supporting information necessary to evaluate and justify the amendment. Such changes will be made from time to time as determined necessary to ensure that the text and maps remain accurate.

### 1.6 MAP INTERPRETATION

Unless otherwise specified within the Plan, the boundaries or locations of any symbols or areas shown on a map are approximate only, not absolute, and shall be interpreted as such. They are not intended to define exact locations except where they coincide with clearly recognizable physical features or fixed boundaries, such as property lines or road and utility rights-of-way.

### 1.7 CONSISTENCY AND MONITORING OF THE PLAN

It is intended that consistency between the Plan and any other policy directives which have been approved by Council be maintained, including but not limited to, the Municipal Development Plan \# 23-2014 of Lac Ste Anne County.

In order to ensure the Plan remains current and relevant, it will be monitored over time. If any changes are deemed necessary as a result of future monitoring, the Plan will be modified through the amendment process.

### 2.0 PLAN AREA

### 2.1 REGIONAL CONTEXT

The Lac Ste Anne County is located at the junction of Highway 2A, Highway 13 and the Canadian Pacific railroad and approximately 12 kms east of Queen Elizabeth II Highway. In addition, the city is approximately 43 miles south of the City of Edmonton and the Edmonton International Airport. Anecdotally, Wetaskiwin as a stage coach stop between Edmonton and Calgary.

The Lac Ste Anne County is the center of a thriving mixed farming and oil and gas region, and is the gateway to some excellent recreational facilities centrally located amongst many lakes and rivers, and close to the foothills.

Based on the 2014 census the Lac Ste Anne County supports a primary trading area of 50,264 people and a secondary trading area population of 93,637 .

### 3.0 EXISTING SITE FEATURES \& CHARACTERISTICS

The Plan Area (as shown in Figure 2) consists of approximately 54.47 ha. (134.47Acres) of land intended to accommodate a mixed use development which shall include Industrial / Country Residential / Public Utilities, and Reserve.

### 3.1 SITE CHARACTERISTICS

The Plan Area as shown in Figure 3 (Topographical Features) has varied relief. The plan area varies approximately 25 meters in elevation with the high point located in the north west quadrant of the plan area to the lowest point which is the south east quadrant of the plan area. Within the north quadrant of the parcel and north or the proposed development area is a ravine or channel which severs approximately 9 hectares of land from the remainder of the plan area.

### 3.2. DRAINAGE BASIN

The plan area is part of the Kilini Creek drainage system and as is the case with most undeveloped parcels of land the plan area is the subject of imperfect drainage.

Drainage within the parcel flows in an easterly direction. Historically there are two drainage patterns within the plan area. The first drainage pattern is the flow of surface drainage from higher elevation lands in the north west to lower elevation lands in the extreme south east. Again, this is surface drainage with no clearly defined channelization. The second drainage channel is clearly defined and is located within the north quadrant of the parcel and flows in an easterly direction towards lands east of the development. This drainage channel traditionally facilitated the flow of drainage from lands to the west of the plan area, however, aggregate extraction on lands west of the plan area have now altered that historic drainage channel and only drainage from the existing parcel are now captured within this channel. This view is supported through the interpretation of both current and historical aerial photography.

### 3.3 UTILITY RIGHTS OF WAY \& ROAD DEDICATIONS

Road Right of Way Plan 782-0078 has been removed from the lands in title. This Road Plan consists of approximately 2.0 acres (more or less) for road widening was removed from the east boundary of the parcel.

In addition, a Natural Gas Right of Way, as shown in Figure 4 (Pipelines and Rights of Way) is dedicated along the south boundary of the plan area, is owned operated by the Lac Ste Anne Gas Coop and serves the existing country residential parcels located within the plan area.

### 3.4 EXISTING USE OF LANDS

The existing use of lands can best be described as a mixed land use. This mixed use reflects the existence of a gravel extraction operation which has ceased to exist and is awaiting reclamation as per an reclamation permit
issued by Alberta Environment and Parks. There are three (3) country residential parcels under separate certificates of title located in the south east corner of the quarter section. There is a dwelling unit located within proposed Lot \# 8 as shown within Figure 5 (Conceptual Plan). In addition, Lac Ste Anne County has recently approved an industrial storage facility on the site as a discretionary use. The balance of the land is in agricultural production.

### 3.5 ADJACENT LAND USES

Lands to the north of the plan area have been reclaimed from aggregate removal activities and are in cultural production. Lands to the east of the plan area are Industrial (aggregate extraction and wetlands) while south of the plan area the lands are predominantly cultivated lands.

The area to the west of the plan area contains both cultivated soils and a large aggregate extraction operation.

### 3.6 VEGETATION \& SOILS

The CLI capability for agriculture is rated as class $4 \mathrm{M}(60 \%)$ and $\mathrm{O}(40 \%)$ within the Plan Area. Soils that are rated as class 4 have severe limitations and have restrictions on the range of crops or require special conservation practices due to moisture limitations. The areas that are rated as O are considered organic soils.

### 3.7 SURFICIAL GEOLOGY

Based on the Quarternary Geology, Central Alberta map (I. Shetsen 1990) the Plan Area is located in an area described as a stagnation moraine which typically exhibits till of uneven thickness with local water sorted material up to 30 m thick. This stagnation moraine also exhibits undulating to hummocky topography reflecting variations in till thickness. In this area, the topography is described as undulating with local relief generally less than 3 m . Bedrock at the base of the deposit consists of grey feldspathic, clayey sandstone, grey bentonitic mudstone and carbonaceous shale of the Horseshoe Canyon Formation of the Cretaceous age. The bedrock contact is non marine and is often characterized by a concretionary ironstone beds, scattered coal and bentonite beds of variable thickness, minor limestone beds.

### 3.8 EXISTING TRANSPORTATION NETWORK

Figure 3 (Pipelines \& Rights of Way) illustrates the major features of the area's existing transportation network. The system impacting the plan area is comprised of Range Road 20 which is constructed to a paved industrial road standard to the east and Township Road 540 to the south and is constructed to an 'all weather' gravel standard.

### 3.9 ACCESS

There are two access points serving the plan area. The area north of the ravine has a direct access to Range Road 20 while access to the balance of the parcel is gained via Township Road 540 with an existing approach located along the west boundary of the plan area.

### 3.10 MAJOR FRANCHISED UTILITIES

Fortis Alberta owns an overhead power transmission line, which is located on the west and south boundary of the Plan area. Future subdivision will require connection to this line for electrical servicing.

Lac Ste Anne Gas Coop is presently located along the south boundary of the plan area and serves the three existing country residential lots which are under separate titles. Future subdivision and subsequent development will require connection to this line for natural gas service.

### 3.11 ENCUMBERANCES

A restrictive Covenant pursuant to the Municipal Government Act is registered against the lands contained within the 'Plan Area'. The intent of the covenant was to restrict future residential development until the aggregate mining activity which was located in the north quadrant of the quarter section had been exhausted.

### 4.0 STRATEGY

### 4.1 PLAN PRINCIPLES

### 4.1.1 SUSTAINABLE DEVELOPMENT

The first principle is focused on a sustainable community or sense of place that will encompass the core values of a 'Triple Bottom Line' consisting of social responsibility, economic viability and ecological integrity.

- All development shall be in an environmentally sustainable manner, which includes the protection of groundwater supply to ensure that this resource lasts well into the future.
- Development shall be restricted to non-polluting uses and practices.


### 4.1.2 SIGNIFICANT ENVIRONMENTAL FEATURES

The second principle is one of identifying and protecting environmental features of significance.

- Low lying areas, which at the present time serve as a seasonal surface runoff retention area, need be protected where possible.


### 4.1.3 INTEGRITY OF TRANSPORTATION INFRASTRUCTURE

The third principle is maintaining the integrity of the transportation network:

- Ensuring that development is accommodated in a fashion that public safety is first and foremost.


### 4.1.4 EXPANSION OF THE TAX BASE

The fourth principle is taking the necessary steps to broaden the tax base of Lac Ste Anne County in a manner that limits the demands of new development on the County's existing infrastructure.

- Proposing the uses of land that demand reasonable and practical levels of service from the County of Lac Ste Anne.
- Encourage the location of land uses in which assessment are sustained at a high level (minimal depreciation).


### 4.1.5 GEOGRAPHICAL IMPORTANCE AND VISUAL APPEAL

The last principle relates to the highest and best use of lands:

- Ensuring that the nature of development is one that reflects positively on the County of Lac Ste Anne.
- Development espouses the values of a proud, prosperous, healthy, and vibrant community.
- Consistent site development guidelines are implemented to ensure that development within the Plan Area is visually appealing and environmentally sustainable.


### 4.2 PLAN PROCESS

The Plan preparation process began by gathering, reviewing and analyzing all relevant information pertaining to future development options within and around the Plan Area. In addition, inventories relating to hydrology and soil permeability are referenced in support of this Plan.

### 4.2.1 BIOPHYSICAL ASSESSMENT

The overall purpose of the Biophysical Assessment (Appendix ' $A$ ') was to determine the environmental features found on the subject property, including any watercourses or wetlands subject to the Alberta Water Act or the Public Lands Act.

- To determine the importance and conservation value of various natural areas located in the study area with respect to future development;
- To determine the value of existing wetlands, woodlots and other ecological features within the study area;
- To assess the connectivity with other surrounding ecological features;
- To identify potential environmental and municipal reserves;
- To identify applicable legislation; and
- Provide recommendations for conservation measures.


### 4.2.2 GEOTECHNICAL INVESTIGATION

In compliance with the requirements stipulated within Lac Ste Anne County Design Guidelines and Construction Standards for Development, a geotechnical investigation was undertaken by Twerdoff \& Associates Inc. (Appendix ' $B$ ') The objectives of the aggregate assessment were:

- Determine subsurface soil and groundwater conditions. This was completed by drilling a series of boreholes within the property and installing groundwater monitor wells.
- Evaluate the proposed work plan and make recommendations on soil densification and/or preloading for the foundation system.
- Provide a summary of the subsurface soil and groundwater conditions, summary of stratigraphy, suitability as fill soils, and make recommendations with respect to foundation types, pavement design and lateral earth pressure calculations to be used in the design limitations for local infrastructure and buildings.


### 4.2.3 TRAFFIC IMPACT ASSESSMENT

The purpose of the Traffic Impact Assessment (Appendix 'C') prepared by D \& A Paulichuk Consulting Ltd. was to assess the potential impact of additional traffic on both the existing local and regional transportation network as a direct result of the proposed development.

The assessment identified and defined the study area, the planning horizon, the analysis period and estimated traffic demand coupled with existing traffic conditions.

In addition, the assessment incorporated a safety analysis, site access analysis, traffic collision analysis, sight distance evaluation and provides overall recommendations for addressing local and regional traffic impacts for incorporation into the ASP.

### 4.2.4 HYDROLOGICAL ASSESSMENT

A hydrological assessment was prepared by Groundwater Information Technologies Ltd. to determine if a safe 20 yield of potable water was available to sustain the proposed development. A copy of the analysis is attached hereto and referenced as Appendix 'D'.

### 5.0 PLAN OBJECTIVES

In the preparation of the ASP a detailed review of existing planning policy was conducted to ensure alignment with Lac Ste Anne County planning and policy principles. Without restricting the generality of the foregoing the following documents were referenced within the ASP:

- Lac Ste Anne County Land Use Bylaw;
- Lac Ste Anne County Municipal Development Plan;
- Lac Ste Anne County Development and Design Standards;
- Lac Ste Anne County Corporate Strategic Plan;
- Lac Ste Anne Rural Road Study;
- Lac Ste Anne Master Transportation Plan;

Having regard for the Lac Ste Anne County planning and policy principles and based on the premise of the triple bottom line as the foundation for creating a sustainable development the objectives of the ASP are to:

- Prepare a future development concept for the plan area and provide policy direction describing the manner in which land may be developed.
- Promote a transportation network that includes multiple modes of transportation which shall include automobiles, cycling and walking.
- Identify a strategy for providing open space and trails, including linkage to the existing open spaces and trail system and promote interconnected road and path systems that facilitate efficient provision of municipal services and maintenance.
- Provide for the efficient and phased conceptual design of development.
- Identify lands suitable for public recreational opportunities and include both active and passive recreational opportunities.
- Ensure proper protection for environmentally sensitive areas.
- Recognize and maintain the landscape and other environmental qualities of the plan area.
- Promote the diversification of the local economy by examining economic opportunities and economic viability.
- Incorporate diversity of use that may include and promote home, work and play in close proximity.
- Promote the inclusion of various energy efficiencies in the design of the community by orientating streets to maximize solar gain and reduce northern exposures.
- Promote crime deterrence through safe neighbourhood design by incorporating into the design visible public spaces, clear boundaries between public and private spaces, and identifiable points of entry.
- Promote development which advocates live, work and play opportunities within a mixed use built environment.


### 6.0 CONCEPT PLAN

The land use prescribed for the site is reflected in Figure 5 (Land Use Concept Plan). This concept plan outlines the land uses, transportation network, municipal reserve, open space and utility infrastructure for the Plan Area.

It is acknowledged that much of the plan area is considered as lands possessing limited agricultural capabilities. It is further acknowledged that existing development patterns, development pressures, existing parcel configuration, general public input and the needs of the community to broaden the economic base of the County suggests strong support for the proposed use of the lands.

As such, the plan area has been identified as having characteristics favorable for future development as per Figure 5 (Land Use Concept Plan).

Development of the Plan Area could provide the opportunity for properties adjacent to the plan area to take advantage of upgraded infrastructure.

The Land Use Concept Plan proposed for the Lac St Anne Area Structure Plan is shown on Figure 5 (Land Use Concept Plan).

Specifically, the Land Use Concept Plan responds to the following critical factors:

- The Lac Ste Anne County existing statutory plans and in particular the Municipal Development Plan.
- Acknowledgment of the existing conditions, such as natural features, current uses of land, parcel boundaries, and subdivision and development opportunities, which result in both opportunities and constraints for future land use.
- The existing transportation network infrastructure including proposed improvements.
- Acknowledgement that infrastructure services are readily available.
- The existence of an aggregate extraction activity which has ceased operation.


### 6.1 PHASED DEVELOPMENT

The Clearview Ridge Area Structure Plan envisions that the Plan Area will be a phased development, the timing of which is influenced by several factors notwithstanding:

- The immediate needs of the landowner.
- Market conditions.
- A coordinated approach in the provision and construction of 'on-site' infrastructure requirements such as road design, storm water
management, and shallow utility installation.
- The establishment of an economic model that reflects the nature of the development and the limitations of front-loading financial obligations.
- The completion of the Resource Extraction Reclamation Plan prepared by Aspen Land Group Inc. on behalf of Westrock Aggregates Ltd. dated December 2013, attached herewith, and referenced as Appendix ' $E$ '.


### 6.2 PLAN POLICIES

The policies listed below are unique to the Clearview Ridge Area Structure Plan and are to be applied at the time of the new subdivision and development. All existing statutory plans and policies, particularly those policies referenced in the Lac Ste Anne County Municipal Development Plan, as well as the Land Use Bylaw shall be applied.

Municipal officials and industry representatives have indicated that the demand for a demand for lots ranging from two acres to parcels in excess of five acres in size. To address the need for flexibility in parcel size, a 'scalable' development is proposed in which parcels may be consolidated to achieve specific needs

### 6.2.1 PHASE I

Phase I shall consist of 9 Country Residential lots, Municipal Reserve dedication and a Public Utility lot, as well as, the provision of a road system necessary to support the phase I development containing approximately 18.24 ha ( 45.07 acres) as shown in Figure 5 (Future Land Use Concept Plan).

### 6.2.2 FUTURE PHASING

Future Phasing shall include the subdivision and development of the residual lands in title as shown in Figure 5 (Land Use Concept Plan).

### 6.3 LAND USE

### 6.3.1 LAND USE POLICIES

Notwithstanding the above, all future subdivision and development within the Plan Area shall have regard to the spirit and intent of the Lac Ste Anne County Municipal Development Plan.
6.3.1.1 All future subdivision and development within the Plan Area shall comply with the generalized Land Use Concept shown in Figure 5 (Land Use Concept Plan).
6.3.1.2 All future subdivision and development within the Plan Area shall be in accordance with the requirements of the Lac Ste Anne County.
6.3.1.3 Prior to approval of future phasing as shown in Figure 5 (Land Use Concept Plan), a comprehensive geotechnical investigation may be required by the developer in support of future phasing development.
6.3.1.4 The introduction of future phasing shall be contingent on the completion of the reclamation works as identified in the reclamation plan as prepared by Aspen Land Group on behalf of Westrock Aggregate Ltd.
6.3.1.5 Development of land within the plan area shall be in strict accordance with provincially mandated setbacks from water bodies and environmentally sensitive areas.
6.3.1.6 The Developer shall be required to enter into a deferred services agreement with the County prior to receipt of subdivision approval of Phase I, and the County may register a caveat respecting the said deferred services agreement for the purpose of informing lot owners of the requirement to connect to a municipal water and/or wastewater system when such services become available.
6.3.1.7 The re-subdivision of lands contained within proposed Industrial Lot \# 16, as shown in Figure 5 (Land Use Concept Plan), the size of which are to be determined at the time of subdivision. The future industrial uses are intended to promote live, work and play opportunities.

### 6.4 TRANSPORTATION NETWORK

The proposed development is to be served by both a proposed access to Township Road 540 as well as an additional access point on Range Road 20 as shown in Figure 5 (Land Use Concept Plan).

### 6.4.1 TRANSPORTATION POLICIES

6.4.1.1 All road improvements, including new construction shall be constructed to municipal standards as stipulated in the Lac Ste Anne County General Municipal Servicing Standards dated January 2008, and shall be at the sole cost and expense of the Developer.
6.4.1.2 Required future road widening to any existing roads including Township Road 540 shall be dedicated at the time of subdivision.
6.4.1.3 All internal development (Future Phasing) within the Plan Area will access a proposed internal road system as shown in Figure 5 (Land

## Use Concept Plan).

6.4.1.4 The Developer shall (if necessary) dedicate land for the upgrading of the point of intersection of the proposed internal road with Range Road 20 as shown in Figure 5 (Land Use Concept Plan).

### 6.5 ENVIRONMENTALLY SIGNIFICANT FEATURES

Policies are proposed to ensure that the existing environmentally significant features within the Plan Area are protected, while also providing opportunities for the development of new environmental features.

### 6.5.1 ENVIRONMENTALLY SIGNIFICANT FEATURES \& SUSTAINABLITY POLICIES

6.5.1.1 All development within the plan area shall be the subject of Smart development principles and the application of the Triple Bottom Line approach to development.
6.5.1.2 The use of high efficiency and energy efficient building materials, fixtures and appliances shall be encouraged.
6.5.1.3 The individual harvesting of rain water for irrigation purposes on each lot shall be encouraged.
6.5.1.4 Where possible buildings shall be orientated to provide the greatest exposure to the sun and create solar heating and solar capture opportunities.
6.5.1.5 Each lot owner shall be encouraged to plant shelter belts along the north boundary of their lot to provide additional protection from the northern winds.
6.5.1.6 When feasible the harvesting of surface runoff and rainfall captured from buildings and used in the business process shall be encouraged.

### 6.6 MUNICIPAL RESERVE /OPEN SPACE

The Launch Ventures Area Structure Plan supports the policies within the Lac Ste Anne County Municipal Development Plan with respect to encouraging development of an integrated and contiguous trail system.
6.6.1 Municipal Reserve, as shown in Figure 5 (Land Use Concept Plan) will be dedicated in a manner which will enhance and compliment both the Plan Area and the community at large.
6.6.2 Municipal Reserve shall be provided by either the dedication of land, disposition through cash in lieu, or a combination of both. The disposition of reserve including the disposition of the existing reserve caveat shall be at the sole discretion of the Lac Ste Anne County at the time of subdivision.
6.6.3 Pedestrian and bicycle linkages to the adjacent community shall be incorporated into each phase of the development.
6.6.4 Pathways/trail systems shall be aligned with existing and future external pathway/trail systems.
6.6.5 Pathway/trail systems shall, where possible, be incorporated into landscape buffers and sound attenuation berms thereby increasing both the utility and the aesthetics of the berms and buffers.
6.6.6 Pathway/trail systems shall where possible serve as a 'transition' between differing proposed and existing land uses.
6.6.7 The location of a proposed community mail box as well as a school bus collection site are as shown in Figure 5 (Land Use Concept Plan).

### 6.7 SERVICING INFRASTRUCTURE

The purpose of the strategy is to provide for the installation of appropriate utility infrastructure necessary to support the Plan area. Water and Waste Water will be the responsibility of the individual lot owner. Storm water ponding will be constructed within the Plan area to prevent downstream impacts due to surface water runoff.

Sizing and detailed design of the servicing infrastructure including the storm management will be completed in conjunction with the subdivision and development of the project and administered through the development agreement process.

All infrastructure service development shall be designed and constructed in accordance with the specifications and guidelines as provided in the Lac Ste Anne County General Municipal Servicing Standards dated January 2008.
6.7.1.1 All infrastructural development within the plan area shall have regard for the possible extension of services to adjacent lands.
6.7.1.2 Where identified, oversizing of infrastructural services within the plan area will be incorporated into the design and construction process.
6.7.1.3. Where possible, the developer in conjunction with the Lac Ste Anne County shall examine an innovative approach to infrastructure improvements and services that are based on sound economic business practices.

### 6.7.2 POTABLE WATER POLICIES

All subdivisions and development shall be serviced with individual water wells.

### 6.7.3 SANITARY SEWER SYSTEM

All subdivision and development shall be serviced with individual Private Sewage Disposal Systems and shall include secondary waste water treatment.

### 6.7.3.1 SANITARY SEWER SYSTEM POLICIES

6.7.3.1 Prior to the issuance of a development permit for any construction on each specific lot, the Developer shall provide the approved plans of the proposed construction including a site plan indicating the topographical features.
6.7.3.2 Design for constructed waste water collection system shall follow the 'Standards and Guidelines for Private Sewage Disposal Systems, Alberta Municipal Affairs, and latest edition as a minimum.

### 6.7.4 STORM WATER MANAGEMENT

The Storm water management system will consist primarily of piping and overland drainage. Storm water run-off from the Plan Area will be restricted to pre-development flows in accordance with the County and Alberta Environment and Parks standards unless otherwise approved by the Lac Ste Anne County. This will be accomplished by using dedicated public utility lots, joint use recreational lands, roadway ditches, culverts, and drainage swales along lot lines. Individual lots will be graded to direct runoff water to the drainage swales, ditches or catchment basins. These ditches will be used to convey water from the lots to the storm pond.

The layout of the overland flow system is designed to work closely with existing topography, as well as the lot layout. The slopes of the ditches and the drainage swales should be maintained between $0.5 \%$ and $2.0 \%$. The ditches and swales should be protected with grass vegetation as soon as possible to reduce erosion, and help with storm water quality.

## STORM WATER MANAGEMENT POLICIES

6.7.4.1 Prior to the subdivision and development of 'Future Phasing' the Developer shall implement a storm water management plan to the satisfaction of the Lac Ste Anne County.
6.7.4.2 Design for constructed storm water management facilities shall follow the 'Standards and Guidelines for Municipal Waterworks, Wastewater and Storm Drainage Systems,' Alberta Environment and Parks, latest edition as a minimum.
6.7.4.3 Native soils shall be salvaged and stockpiled and reused as topsoil and planting bed material.
6.7.4.4 Prior to the subdivision and development of each lot a 'lot grading plan' in accordance with the Storm Water Management Pan will be provided for each lot created.
6.7.4.5 Prior to the subdivision and development of Phase I as shown in Figure 6 - (Storm Water Management Plan) the developer and the Lac Ste Anne County shall enter into an agreement with respect to the implementation of a phased storm water management plan.
6.7.4.6 Storm water retention devices such as 'dry ponds' may be designed as joint utility/recreational uses.







# "LAC ST ANNE Biophysical Assessment" 

S.E. 1-54-2-5

## PREPARED BY

MCA Environmental Management

## APPENDIX 'A'

January 2016


Biophysical Inventory
For SE $1 / 4$ Section 1-54-2 W5M
County of Lac Ste. Anne
MCA File\#2015-1222

## Submitted to: <br> Wescott Consultants Ltd.

## Executive Summary

This biophysical assessment has been completed for Launch Venture Ltd. and their consultants Wescott Consulting Group (Cochrane, Alberta), as part of the County of Lac Ste. Anne planning requirements. The client's objective is to develop a parcel of land that is approximately 44 ha at the northeast corner of Lac Ste. Anne, Alberta.

The land description of the property is a portion of the SE 1-54-2 W5M in the County of Lac Ste. Anne.

The biophysical assessment will serve as the base for planning the future development of the property as required by an area structure plan (ASP).

A biophysical assessment is a necessary requirement for the approval of a more detailed ASP for the proposed project site, as well as approval of the storm water management plan for the subdivision, under the Alberta Water Act and the Environmental Protection and Enhancement Act.

A biophysical assessment is conducted to identify significant and sensitive environmental components on the project site prior to the development of an ASP, and to make recommendations on the sustainability of the site, whether parts of it can or should be preserved in the natural state, and if so, what mitigation and monitoring measures are necessary to achieve sustainability. The Assessment provides recommendations for dedication of lands to be conserved in their existing state within the context of the proposed development project, for the purposes of conservation of habitat, hydrology, and protection of erodible land, water quality or other environmental needs.

The results of the biophysical assessment indicate that the property presents two distinct biological areas. The upland crop area where there is very low diversity with a small wetland on the east border and the creek ravine where there is a great deal of biological diversity. There were no indications of potential species at risk on either site however a detailed investigation on the south side of the ravine was not completed due to the naturally occurring steep slope. The ravine provides a significant ecological linkage with adjacent properties and is considered valuable not only to the property but also to the regional movement of species.

The County of Lac St Anne has identified this particular ravine as a main drainage area (creek) and requires setbacks of 70 meters from the top of the ravine. The ravine has a very steep banks and drivers for this designation may include value of the ecological setting as well as the lack of stability on the slope.
A former gravel pit in the north west corner of the property is registered under the name of Westrock Aggregates Ltd. The mine is exhausted and the pit owner has submitted a pit registration application to Alberta Environment in order to pursue pit reclamation. The reclamation plan encompasses an area of approximately 54 ha which includes the pits located in NE 1 and the SE $1-54-2 W 5 M$. Specifics related to the proposed reclamation are provided in the registration application on file with Wescott.
The subject property has been in use for agricultural purposes since at least 1949 and consists of productive agricultural land in the form of a class 2 soil. While there are no specific limitations related to the development of the land for wildlife habitat, regulators and the developer will likely recognize that development of the site will result in a decrease to local productive agricultural land.

Recommendations for future studies prior to the development of the land include the following.

- Identify timing of the pit reclamation program
- Develop a master drainage plan prior to site development as per the Storm water management guidelines
- Fire prevention and control strategies
- Site hazard management planning
- Weed management and control

These recommendations are not intended to be exclusive. Regulators may have additional requirements not listed here. Reference should be made to the recommendations provided in this report along with the Lac St Anne Municipal Development Plan.

## Table of Contents

1.0 INTRODUCTION ..... 1
1.1 Project Purpose ..... 1
1.2 Project Overview and Site Location ..... 1
1.3 Scope of the Assessment ..... 4
2.0 Approach and Methodology ..... 4
3.0 Regulatory requirements ..... 6
3.1 Federal Legislation. ..... 6
3.1.1 Migratory birds Convention Act 1994 ..... 6
3.1.2 Fisheries Act ..... 7
3.1.3 Species at Risk Act ..... 7
3.1.4 Federal Policy on Wetland Conservation ..... 8
3.2 Provincial Legislation ..... 8
3.2.1 Public Lands Act ..... 8
3.2.2 Environmental Protection and Enhancement Act 1983 ..... 8
3.2.3 Water Act ..... 9
3.2.4 Alberta Weed Control Act. ..... 10
3.2.5 Wildlife Act ..... 11
3.2.6 Historical Resources Act ..... 11
3.2.7 Species At Risk Program ..... 11
3.3 Standards Policies and Guidelines. ..... 12
3.3.1 Standards and Guidelines for Municipal Waterworks and wastewater and stormwater drainage systems ..... 12
3.3.2 Wastewater and Storm drainage regulation and wastewater and storm drainage (ministerial) regulation ..... 12
3.3.3 Stormwater Management Guidelines for the Province of Alberta ..... 13
3.3.4 Municipal Government ..... 13
3.3.5 Code of Practice for Pits ..... 15
4.0 Current and Historical Land Use ..... 16
4.1 Site Visit. ..... 16
4.2 Historical Aerial Photo Review ..... 17
5.0 The Existing Environment ..... 20
5.1 Climate and Air Quality ..... 20
5.2 Physiography and Topography ..... 20
5.3 Geology and Soils ..... 22
5.4 Surface Water ..... 22
5.4.1 Drainage Patterns on the Property ..... 22
5.4.2 Drainage around the property ..... 22
5.5 Groundwater ..... 22
5.6 Vegetation ..... 23
5.6.1 Regional Context ..... 23
5.6.2 Site Context ..... 23
5.7 Wetlands and Other water bodies ..... 24
5.8 Wildlife ..... 24
5.8.1 Regional Context ..... 24
5.8.2 Wildlife on the property ..... 24
6.0 Biodiversity ..... 24
6.1 Species Richness ..... 24
6.2 Species at Risk ..... 25
6.3 Threats to Biodiversity ..... 25
7.0 Sustainability ..... 26
7.1 Ecological linkages ..... 26
7.2 Mapped and Classed Waterbodies in the Area ..... 27
8.0 Recommendations ..... 28
8.1 Residential subdivision design ..... 28
8.2 Opportunities for habitat conservation ..... 28
8.3 Surface water and groundwater ..... 28
8.4 Environmental sustainability and Community Participation ..... 29
8.5 Ecological linkages ..... 29
8.6 Hazards Wastes and Disturbances ..... 29
8.7 Gravel Pit Reclamation ..... 29
9.0 Conclusions ..... 30
10.0 Closure ..... 31

## Figures

Figure 1: $\quad$ Site Location Map
Figure 2: Aerial photo of site
Figure 3: Land titles Site Survey

Figure 4: Proposed Site Development
Figure 5: $\quad$ Aerial photo of the site 1967
Figure 6: $\quad$ Aerial photo of the site 2005
Figure 7: Site Drainage
Figure 8: Potential Ecological Linkages

Tables
Table 1: Aerial Photo Review

## APPENDICIES

Appendix A: Site Photos and Site photo location Figure
Appendix B: List of vegetation species observed on site
Appendix C: Code of Practice for Pits - reference to the website provided
Appendix D: ACIMs database search results
Appendix E: FWIMT database search results
Appendix F: Site Development plan - Possible lot layout
Appendix G: Alberta Groundwater Well Reconnaissance Report
Appendix E: Pit registration application

### 1.0 INTRODUCTION

### 1.1 Project Purpose

This biophysical assessment has been completed for Launch Ventures Ltd. and their consultant Wescott Consulting Group (Cochrane, Alberta), as part of the County of Lac Ste. Anne Planning requirements. The client objective is to develop a parcel of land comprising approximately 55 ha. The proposed development will entail residential acreage properties on the central portion of the site and potential gravel extraction on the northwest portion of the property.
The legal land description of the property is SW 1-54-2 W5M. The land is located within the jurisdiction of County of Lac Ste. Anne. In order to allow for the land to be designated as country residential (for the purposes of subdivision), the County requires that an Area Structure Plan (ASP) be developed for the site.
A biophysical assessment is a necessary requirement for the approval of a more detailed ASP for the proposed project site, as well as approval of the storm water management plan for the subdivision, under the Alberta Water Act and the Environmental Protection and Enhancement Act.

A biophysical assessment is normally conducted in order to identify significant and sensitive environmental components on the project site prior to development. The biophysical assessment will provide recommendations on the sustainability of the site, whether parts of it can or should be preserved in the natural state, and if so, what mitigation and monitoring measures are necessary to achieve sustainability. The Assessment provides recommendations for dedication of lands to be conserved in their existing state within the context of the proposed development project, for the purposes of conservation of habitat, hydrology, and protection of erodible land, water quality or other environmental needs.

Accordingly, the purpose of this Assessment is:

- To identify and evaluate existing ecological features on the site as they appear at the present time;
- To provide practical recommendations for preserving or enhancing ecologically significant features within the context of the ASP;
- To provide general recommendations for mitigation of potential adverse environmental effects resulting from the development, on the site and on surrounding lands;
- To identify potential environmental and/or municipal reserves;
- To identify applicable legislation; and,
- Provide recommendations for conservation or mitigation measures.


### 1.2 Project Overview and Site Location

The quarter section of land containing the proposed development site is located at the northwest corner of the intersection of range road 20 and township road 540. The legal land description is SE 1-54-2 W5M containing approximately 55 ha. According to the Lac Ste. Anne Municipal Development Plan (Map 2) the site is currently located in an area identified for future rural
residential development. This site however is still classified as agricultural however although there are two subdivided lots in the southeast corner of the site and a new residence located near the west access road.

Figure 1 Shows the location of the project site in regional contexts. Figure 2 shows the site location on the County Map. Figure 3 shows a legal survey of the property.


Figure 1: Site location


Figure 2: Site Location Lac Ste. Anne County Map


Figure 3: Legal Survey of Site Boundaries

### 1.3 Scope of the Assessment

The biophysical assessment addresses all parts of the natural environment, and includes:

- Topography, geology and soils;
- Hydrology (surface water, ground water);
- Vegetation (terrestrial, wetland);
- Wildlife (birds, fish, herptiles, invertebrates, mammals) and potential habitat;
- Sustainability of ecosystems;
- Linkages with adjacent ecosystems (connectivity); and,
- Biodiversity and species at risk (rare, threatened and endangered species).

The geographical scope of the assessment is the proposed project site, where boundaries are shown in Figure 3. The assessment also takes into account adjacent land uses and ecological linkages with the subject property in a regional context if warranted.

### 2.0 APPROACH AND METHODOLOGY

The biophysical assessment was conducted to describe and interpret site features as they existed at the time of the field reconnaissance, which took place on Tuesday August 25, 2015. The study included the following activities:

- Consultation with Wescott Consulting Group undertaking the detailed design of the proposed site development;
- Discussions with site owner on proposed future activities;
- Review of any maps, previous reports, etc., completed for this project;
- Examination of historical aerial photographs, to assess surrounding land use, vegetation areas, developments, etc.;
- Database searches, e.g., ANHIC database for tracked and listed species;
- Field reconnaissance of the site; and,
- Analysis of the information, and drafting the biophysical assessment report.

The scope of the field reconnaissance was to observe features of vegetation, drainage, wildlife and other components of the natural environment, as existed at the time of the field reconnaissance. The types and distribution of vegetation, the type of forest, drainage features, wildlife and wildlife signs (e.g., tracks, feces, hair, burrows, nests, rubs, scrapes, etc.), and any other environmental features, were noted by location and documented with photographs.
The contours of the site were observed on topographical map sheets and directly in the field, and note was made of any depressions which might form a wetland, gully or natural drainage course.

Digital historical aerial photos were obtained from Alberta Sustainable Resource Development (ASRD) and examined at intervals of 5 to 10 years from July 1949, through to August 2011 to learn about past and present vegetation cover, earthworks, human activities or places on the site water tends to collect. Potential wetlands were also identified by noting lower topographical areas, and dark-shaded areas in the aerial photographs. The potential wetlands were then observed in the field, and the vegetation growing there was examined. Classification of these areas as potential wetlands was done mainly according to the Stewart and Kantrud Wetland Classification System (Stewart and Kantrud, 1971). The historical aerial photographs were examined to determine the configuration of each low, wet area and the amount of surface water present in various years.

### 2.1 Previous Assessments and reports

Background information for the site were provided by Wescott Consulting and included the following.

- Aggregate Characterization Assessment
- Gravel permit (not reviewed)
- Pit registration application (Aspen Land Group Ltd. )


### 2.1.1 Aggregate Characterization Assessment

An aggregate characterization assessment was completed by Twerdof and Associates Inc. (June 2014). The characterization revealed gravel resources beneath the site at a depth of 7.6 to 11.3 meters ( 25 t 037 feet) below overburden. The sand and gravel seam appears to be approximately $3-6$ meters (10-20 feet) thick. The overburden to gravel ratio is estimated at around 3:1.

### 2.1.2 Pit registration application

A pit registration application was prepared for an exhausted gravel pit located in the northwest corner of the site. The former gravel pit is registered under the name of Westrock Aggregates Ltd. Aspen Lands Group Inc. prepared the pit registration submission in December 2013. The mine is exhausted and the pit owner has submitted a pit registration application to Alberta Environment in order to pursue pit reclamation. The reclamation plan encompasses an area of approximately 54 ha which includes the pits located in NE 1 and the SE $1-54-2 W 5 M$ referred to as the Yeoman and Parker Pits. Specifics related to the proposed reclamation are provided in the registration application on file with Wescott. It is unclear whether the application was accepted by the Alberta government. No dates appear to have been identified for the reclamation. Access into the site for reclamation was proposed from the NE 1 over the culvert in the ravine. The culvert was proposed to remain in place.

### 2.1.3 Site Development Plans

Plans for future development of the site entails rural residential development as per the plan below. A larger version of the plan is contained in Appendix F.


Figure 4: Phase 1 Proposed Site Development

### 3.0 REGULATORY REQUIREMENTS

The following is a listing of the primary Acts and Regulations at federal, provincial or municipal levels, which could be relevant to various aspects of the proposed development and possible effects on the environment or natural resources. Not all of the following legislation may be directly applicable to the development and the following is not a comprehensive list of any Act or regulation that could apply in any circumstance. Awareness of legislative requirements can be useful when timing site development.

### 3.1 Federal Legislation

### 3.1.1 Migratory birds Convention Act 1994

Under the Migratory Birds Regulation (under MBCA), no person shall hunt a migratory bird except under authority of a federal permit under this Act/Regulation. Subject to subsection 5(9), no person shall (a) disturb, destroy or take a nest, egg, nest shelter, eider duck shelter or duck box of a migratory bird, or (b) have in his possession a live migratory bird, or a carcass, skin, nest or egg of a migratory bird, except under authority of a permit licensed under this Act/Regulation. This Act and its Regulation become particularly important when removing trees or clearing open pasture to facilitate development, to landscape, or to regrade the land. The lack of tree cover on the property combined with the consistent agricultural land use appear to negate concerns related to migratory birds nesting and rearing on the upland area however, there is tree cover and bird habitat in the ravine to the north where birds may nest and rear young.

### 3.1.2 Fisheries Act

If there is any proposed activity that would destroy or adversely affect fish or fish habitat, such proposed activity would require an Authorization from Fisheries and Oceans Canada (DFO) under the Fisheries Act (FA). The Fisheries Act \{R.S. 1985, c. F14\}, applies to all Canadian fisheries waters and assigns the Department of Fisheries and Oceans Canada (DFO) the responsibility to administer and enforce the conservation and protection of fish habitat on private property and on provincial and federal lands. Section 35 of the FA states " No person shall carry on any work, undertaking or activity that results in serious harm to fish that are part of a commercial, recreational or Aboriginal fishery, or to fish that support such a fishery".
A recent change to the FA defers fisheries management to local provinces where equivalent fisheries protection measures are in place. Section 36 of the fisheries act prohibits the deposit of deleterious substances. Environment Canada is responsible for administering this subsection.

There is no water body on this parcel of land to which the fisheries act would apply.

### 3.1.3 Species at Risk Act

The Species at Risk Act (SARA) was passed in 2002 as part of Canada's commitment to the international Convention on Biological Diversity. The intent of the SARA legislation is to prevent species that are listed in Schedule 1 of the Act from becoming extinct, threatened, or extirpated. Additionally, SARA strives to help in the recovery of any listed species through protecting the critical habitats of at-risk species. Under SARA, it is illegal to kill or harm any listed species, or to destroy the residences of any listed species that occur on federal lands. For listed species that that are found outside of federal lands, it is the duty of the province or territory to protect listed species through legislation. This legislation covers birds, plants, fish, mammals, insects, amphibians and reptiles.

In the present case, no rare or endangered species were expected or observed on the portion of the land in which development is proposed. The south slope in the ravine is quite steep and may have species located in areas that were not accessible during the biophysical assessment site visit. The preservation of the ravine as a wildlife corridor should offset any potential concerns related to species at risk in the area.

### 3.1.4 Federal Policy on Wetland Conservation

The Federal Policy on Wetland Conservation was passed by Cabinet in 1991, with the objective of promoting "the conservation of Canada's wetlands to sustain their ecological and socioeconomic functions, now and in the future" (Government of Canada, 1991). The Federal Policy on Wetland Conservation applies to the full range of federal activities and drives management decisions regarding the protection of wetland habitat on federal lands.

The two key commitments of the Federal Policy on Wetland Conservation include:

- no net loss of wetland functions on federal lands and waters through mitigation of all impacts of development related to these wetlands; and
- enhancement and rehabilitation of wetlands in areas where the continuing loss or degradation of wetlands has reached critical levels

There is one small wetland observed on the subject property.

### 3.2 Provincial Legislation

### 3.2.1 Public Lands Act

The bed and shores of all watercourses and water bodies are considered public lands unless the Government of Canada owns them. As such, approvals under the Public Lands Act \{R.S.A. 2000, c. P-40\} are required for any activity on the bed or shore of Crown owned rivers, streams, or lakes. Any activity that alters or occupies the bed and shore of a water body may be done only after written approval.

A Licence of Occupation (LOC) would be required for in stream structures and permanent or temporary facilities on Crown Land. A Licence of Occupation (LOC) is required under the Public Lands Act to build any structures that could have a negative impact on the bed and shore of a water body (e.g., retaining walls, boat launching facilities, breakwater structures, and causeways).

There were no flowing creeks, streams or rivers observed on the subject property. The drainage area in the ravine on the north end however is shown on topographical map sheets.

### 3.2.2 Environmental Protection and Enhancement Act 1983

The Alberta Environmental Protection and Enhancement Act (EPEA) support and promote the protection, enhancement and wise use of the environment. It recognizes the impact of development, polluters paying for their actions, and other such acts.

The Act deals with the release of substances into the environment, regulating releases, and creating general prohibitions with respect to substance release, and also provide the necessary powers to regulate the handling of storm drainage and wastewater. A key part (Section 109) states that no person shall release or permit the release into the environment of a substance in an amount, concentration, or level or at a rate of release that causes or may cause a significant adverse effect, thus covering a very broad range of anti-pollution prohibitions.

Under the Wastewater and Storm Drainage Regulation, EPEA gives powers to Alberta Environment for the regulation of storm water drainage and wastewater systems. The Wastewater and Storm Drainage Regulation and the Wastewater and Storm Drainage (Ministerial) Regulation enable the Department to regulate the operation of storm drainage and wastewater systems and establish standards for such facilities and their operators. This legislation sets out requirements for the construction and operation of municipal plants for handling of storm water drainage and wastewater.

Among other things that the Act covers are the following:

- Harmful emissions to the air (Air Emissions Regulation);
- Release of harmful/toxic substances to the environment (Substance Release Regulation);
- Reclamation of disturbed lands (Conservation and Reclamation Regulation);
- Ozone-depleting substances (Ozone-Depleting Substances Regulation);
- Handling, use and application of pesticides and herbicides (Pesticide Sales, Handling, Use and Application Regulation);
- Potable water (Potable Water Regulation); and,
- Reporting of releases to the environment (Release Reporting Regulation).

EPEA allows for anti-litter orders to be issued for the control of waste on highways, water, ice and public and municipally owned land (which is referred to as enforcement orders). Orders for the cleanup of unsightly property are referred to as environmental protection orders. The forms of both types of orders are set out in the regulations.

EPEA regulates the handling, storage and disposal of hazardous wastes under the Waste Control Regulation. Hazardous wastes are defined in the Regulation.
The Wastewater and Storm Drainage Regulation under AEPEA gives Alberta Environment the responsibility of regulating storm drainage and wastewater systems, including the establishment of standards for such facilities in their operation. This includes naturalized wetlands, other storm water management facilities, outfalls and related piping.

### 3.2.3 Water Act

The Alberta Water Act, which came into force in 1999, supports and promotes the conservation and management of water. It regulates withdrawals and diversions of water, including drilling water wells, through a licensing and authorization system. It regulates water management works and undertakings, and authorizes temporary diversions through a licensing process.

Watercourse crossings (road, bridge, pipeline, telecommunications, etc.) are authorized/regulated through the Alberta Watercourse Crossings Codes of Practice. A Notification must be submitted to Alberta Environment detailing any watercourse crossing structures, and explaining how the construction and operation of the crossing meets the requirements of the Code.
Approval would be required under the Water Act from Alberta Environment in respect to any watercourses or wetlands that might be affected, or whose flows may be affected, by the proposed development.
An approval is required to conduct an activity in a water body. An activity is defined generally to include placing or constructing works within a water body, removing or disturbing ground and/or
vegetation that results in altering the flow, level, direction and/or location of a water body. A license is required to divert or transfer water from a water body.

Construction of an outfall would also require that the Code of Practice for Outfall Structures on Water bodies under the Water Act be followed. This Code of Practice dictates restricted activity periods on water bodies, and requires that certain design standards be followed. The Code of Practice also requires that notice be issued to the Director, Alberta Environment, prior to commencement of the work. Hydrological issues are discussed later in this report.

Approvals would be required under the Water Act to modify or fill any wetlands that might occur on the Property, and to construct any outfall or drainage channel into a water body.

The Act would also apply if withdrawal of water from a Lake/wetland is being proposed as part of the operation, such as for a domestic water supply or other use.

There is one small wetland observed on the east side of the property.

### 3.2.4 Alberta Weed Control Act

On June 16, 2010, the new Alberta Weed Control Act received Proclamation and came into force. The new Act is a comprehensive re-write of the old Act for the purposes of re-organizing, updating, and providing greater clarity to the existing provisions of the latter.

This Act aims to regulate noxious weeds, prohibited noxious weeds, and weed seeds through various control measures, such as inspection and enforcement, together with provisions for recovery of expenses in cases of non-compliance. Additionally, it mandates the licensing of seed cleaning plants and mechanisms. The Act requires that the owner or occupant of lands control noxious weeds and destroy prohibited noxious weeds on land the person owns or occupies. In 2012 the Alberta Invasive Plant Identification Guide was published to facilitate the identification of prohibited noxious and noxious plants.
Under provincial legislation, only pesticides that have been registered for use in Canada by the Pest Management Regulatory Agency under the Canada Pest Control Products Act can be used in Alberta. Pesticides and herbicides are regulated in Alberta under the Alberta Environmental Protection and Enhancement Act and supporting regulation (Pesticide Sales, Handling, Use and Application Regulation).
This legislation provides for the regulation of sales (pesticide vendors) and use (pesticide applicators) of pesticides in Alberta. In addition, there is the Environmental Code of Practice for Pesticides which provides more detailed direction for pesticide sales and use in Alberta.

In the field reconnaissance, various weedy species were observed in the laydown area, but for the most part the land was either plowed for cultivation or dominated by grass species. It is recommended that invasive plant species be managed on site and during future site development (e.g., filling and stockpiling of soil) and that where feasible existing weeds be controlled. Manual and cultural methods should be the priority, but where this is not practical, chemical weed control should follow the above regulations and standards; and in such a manner as not to affect the vegetation of any wetland areas within or near the property.

### 3.2.5 Wildlife Act

Alberta's Wildlife Act is the main piece of provincial legislation that deals with wildlife. Under the Act, hunting without a licence or out of season is prohibited, as is the possession of wildlife and controlled animals (defined in the Act). The Act also covers diseased animals, damage or threat caused by private animals, and the closing of areas to the public to protect wildlife, where necessary. Licences and permits are issued under the Act to regulate hunting or other activities, as outlined above.

### 3.2.6 Historical Resources Act

Section 37 of the Historical Resources Act provides the framework for Historical Resources Impact Assessments (HRIAs) and mitigative studies. When, in the opinion of the Minister of Alberta Community Development (ACD), an activity will or will likely result in the alteration, damage or destruction of an historic resource, the person or company undertaking the activity can be required to

- conduct an HRIA on lands that may be affected by the activity;
- submit to ACD a report discussing the results of the HRIA;
- avoid any historic resources endangered by activity; or
- mitigate potential impacts by undertaking comprehensive studies.

HRIAs and mitigative studies are paid for by the person or company undertaking or proposing to undertake the activity. ACD regulates archaeological and paleontological fieldwork through a permit system. All decision-making concerning the management of historical resources rests with ACD.

One of the requirements of an HRIA is to address compliance requirements associated with the Historical Resources Act of Alberta. As such, it becomes incumbent for the proponent to comply with any government requirements that result from a Historical Overview if one has been done. At a minimum, ACD should be contacted, to determine if they have any concerns about the Property in question, from an historical, archaeological or related perspective.

### 3.2.7 Species At Risk Program

Alberta has a Species at Risk Program, which was initiated as a response to the provinces commitment to the Accord for the Protection of Species at Risk in Canada. The intent of the Accord is to prevent species in Canada from becoming extinct as a consequence of human activity. As part of the assessment procedure, all species of concern are generally assessed and are classified as one of the following categories 1) At Risk; 2) May Be at Risk; 3) Sensitive; 4) Undetermined; and 5) Secure. Any species that is designated as "At Risk" or "May Be at Risk" undergoes a detailed status assessment and is formally designated as Endangered, Threatened, Special Concern, Data Deficient, or Not At Risk. Any species that is designated as Endangered or Threatened becomes legally protected under Alberta's Wildlife Act \{R.S.A 2000, c.W-10\}. This legal designation prohibits the disturbance, killing or trafficking of these species, and provides immediate protection of nests and den sites. Any species that is designated as "Sensitive" after
a general assessment, or as "Special Concern" after a detailed assessment becomes eligible for special management actions designed to prevent the species from becoming "At Risk".

### 3.3 Standards Policies and Guidelines

Alberta Wetland policy (2013)
Interim policy for wetland management in the settled areas of Alberta and Guidelines for wetland habitat Compensation

Initially developed in 1993, this interim policy provides direction on the management of wetlands in the settled areas (white zone) of Alberta (Alberta Water Resources Commission, 1993). The primary goal of the policy is to "sustain the social, economic, and environmental benefits that functioning wetlands provide, now and in the future" by conserving wetlands in a natural state, mitigating the degradation and loss of wetlands, and enhancing, restoring, or creating wetlands in areas where they have been depleted or degraded.

If the flow of surface water is altered or blocked, or if a wetland is being altered or destroyed by filling in or draining, the Alberta Policy on Wetlands and the Alberta Water Act would apply. The Policy, in essence, requires that there be no net loss in wetland habitats in Alberta. If a wetland is destroyed, then compensation must be provided by the person or persons responsible for affecting such damage. Under the policy, compensation can be through directly creating equivalent wetland habitat in another location, or by paying a recognized wetland manager (e.g., Ducks Unlimited) to accomplish this. If the compensation sites are within a certain distance from the affected one, the compensation ratio is 3 hectares of new wetland to 1 hectare of affected wetland. Beyond a certain distance between compensated wetlands and affected wetlands, the ratio becomes higher, and increases with progressive distance. This would apply if any wetland that might occur on the Property was affected by the development or related construction activities.

### 3.3.1 Standards and Guidelines for Municipal Waterworks and wastewater and stormwater drainage systems

Alberta Environmental Protection has established standards and guidelines for the design and operation of municipal waterworks, wastewater and storm drainage systems (Alberta Environmental Protection, 1997). These standards and guidelines outline four types of requirements: Performance Standards, Design Standards, Design Guidelines, and Operating and Monitoring Requirements and Guidelines. These requirements are all directed towards ensuring public health and environmental protection.

### 3.3.2 Wastewater and Storm drainage regulation and wastewater and storm drainage (ministerial) regulation

The Wastewater and Storm Drainage Regulation \{AR 119/93\} and the Wastewater and Storm Drainage (Ministerial) Regulation \{AR 120/93\} fall under Part 4, Division 1 of the Environmental Protection and Enhancement Act \{R.S.A. 2000, c. E-12\}. These regulations create general
prohibitions with respect to substance release to the environment and provide powers to regulate the handling of wastewater and storm drainage

### 3.3.3 Stormwater Management Guidelines for the Province of Alberta

These guidelines were developed as a result of increased urbanization and public expectation for improved runoff control. These guidelines direct the planning, analysis, design, construction, operation, and maintenance of storm water management systems to address concerns associated with storm water runoff and its impact on urban and rural development, and aquatic resources. These guidelines include Best Management Practices for storm water management and quality control.

### 3.3.4 Municipal Government

Most Municipal Government bodies in Alberta now require that a biophysical assessment and/or Environmental Impact Assessment or the equivalent be done prior to subdivision of land, and before the completion of an ASP or other site-specific development plan. One of the purposes of this Assessment is to provide a specific assessment process for dedication of Environmental Reserve, Municipal Reserve and Conservation Easement based on municipal, community and environmental needs. In addition, it makes recommendations as to how to avoid, minimize or control adverse effects on the existing environment resulting from the development, if it is to proceed, and how to incorporate the principles of sustainability in designing and constructing the development.

The County of Lac St Anne currently has a draft municipal development plan (MDP) on the public website. The MDP Map 12 Setbacks from Key Water Bodies, identifies the ravine as a key waterbody requiring a 70 meter setback from the top of the ravine. Other relevant parts of the MDP for this site include the following sections as an example.
Part II - Administrative Procedures section 3.7.1 The Development Authority may require a complete Hydrological Ground Water Impact Report for any commercial, industrial, or multi-parcel development.
Part IV - Development Regulations

## Aggregate Resource Extraction and Aggregate Resource Processing

11.1.9 All sites must be re-districted as AR - Aggregate Resources Extraction and Processing prior to any application for Aggregate resource extraction and/or processing and will be subject to the regulations as set out in that land use district.
11.1.10 Aggregate resource extraction operations are classified as follows:
a) Class I: Operations equal to or greater than 5.0 ha ( 12.35 acres) of disturbed area on a site. Operations under this classification shall require municipal development approval. Reclamation plans shall be under the direction of AESRD and the Development Authority.
b) Class II: Operations less than 5.0 ha ( 12.35 acres) of disturbed area on a site. Operations under this classification shall require municipal development approval. Reclamation plans for existing pits shall be under the direction of County administration. New Class II pits shall be prohibited in Lac Ste. Anne County.

## Minimum Separation Distance and Restrictive Covenants

11.1.11 For lands districted as country residential, all uses within the district shall comply with the following minimum separation distances from all aggregate resource extraction or aggregate resource processing uses based on the nature of the resource development. The minimum separation distance shall be measured from the property line of the lands districted as country, residential. The minimum separation distances are outlined in Table 11.1.1 and Figure 11.1.1.
Table 11.1.1 - Minimum Separation Distances from Aggregate
Operations

## Operation Separation Distance

Extraction: 800.0 m (2,624.67 ft)
Reclamation: 800.0 m (2,624.67 ft)
Crushing: $1,500.0 \mathrm{~m}(4,921.26 \mathrm{ft})$
Wash Plant: $1,500.0 \mathrm{~m}(4,921.26 \mathrm{ft})$
Asphalt Plant: 1,500.0 m (4,921.26 ft)


Figure 11.1.1 Setbacks from Aggregate Resource Extraction
(Upon receipt of scaled drawings of existing sites, the minimum separation distance can be determined. Measurement is related to the distance that a new dwelling can be from an aggregate area.)

Land Use By-Law 24-2014
10.2.19 Development shall not be allowed to detrimentally affect natural features such as nonartificial ponds, streams, wetlands and forested areas, but shall preserve and incorporate such features into the site design so that their key functions can be maintained. In addition:
a) Development of, or in proximity to, wetland areas shall only be undertaken where:
i) It minimizes alterations in the natural flow of water, including surface and groundwater sources, which nourishes the wetlands; and
ii) It protects wetlands from adverse dredging or infilling practices, situation, or the addition of pesticides, salts, or toxic materials.
b) The location of natural features and the site's topography shall be considered in designing and siting of all physical improvements.
10.2.20 Developments must adhere to the following land management practices:
a) Stripping of vegetation or grading shall be done in a manner that will minimize soil erosion by ensuring that the extent of the disturbed area and the duration of its exposure is minimized, and that all grading work should be designed to blend with the natural contours of the land;
b) Natural vegetation shall be retained and protected whenever possible;
c) Natural drainage patterns should not be disturbed and changes to watercourses shall be avoided except where controlled improvements are warranted subject to approval from AESRD; and
d) Developments shall not adversely affect groundwater resources or increase storm water runoff velocity in a way that water levels on other lands are substantially raised or the danger from flooding increased.

This report should address the County of Lac Ste. Anne requirements for a biophysical assessment. The investigation also provides information that would be useful for regulatory determinations under the Water Act, the Public Lands Act and other applicable acts and policies as outlined above.

### 3.3.5 Code of Practice for Pits

Gravel pits 5 ha (12.5 acres) or larger on private land are regulated through the Code of Practice for Pits. The Code of Practice outlines the requirements for all phases of pit development, operation, and responsibilities. Registration holders are required to meet the requirements of the code of practice and must obtain written permission from the current land owner prior to any activity or access to the pit. A new land owner means new written permission must be obtained. The end use of the pit must be discussed with the land owner to ensure they concur.
The code of practice outlines the requirements to be met in order ensure compliance with current environmental legislation and approvals. Setbacks from unstable slopes, drainage areas and
sensitive areas are specifically addressed. A copy of the code of practice has been provided as reference in Appendix C.

### 4.0 CURRENT AND HISTORICAL LAND USE

### 4.1 Site Visit

A site visit was completed in August, 2015 by Melinda McLauchlin of MCA Environmental Management. Observations were made from the road side on the east and south borders of the property and from on site in the north and central portion of the site. The site is bounded on the east by range road 20 and further east by a gravel pit and Kilini Creek. The gravel pit to the east of the site goes north for another quarter section. To the south is township road 540, and agricultural lands and further south more agricultural land. To the west is agricultural land and more gravel extraction to the north and west of the site. The activities in the surrounding area is predominantly gravel extraction and smaller acreage sized developments.

The site was accessed from the west boundary of the property. The access road provides access to a new residential unit near the center of the west side of the site and continues north to a laydown area for heavy equipment.

The site is discussed in five general areas for ease of reference. North ravine, laydown area, former gravel pit, central crop and south farm land.

The North ravine is a steeply sloped forested corridor with the signs of a dry creek bed at the base. There are access trails cut through the forest along the top of the ravine on the north side. The current owner indicated that past owners had cut the trails for hiking or horseback riding. The ravine appears to provide surface water drainage across the northern portion of the site from west to east. Drainage from the west contributes to the ravine flows which ultimately end up at Kilini Creek in the section to the east.

There are berms just south of the forested area on the top of the ravine on the south side (Figure 2). The current owner said he was told the berm was created from topsoil or overburden taken from the gravel pit in the northwest.

The laydown area in the northwest of the property has been cleared in order to create visual barriers in the form of berms and therefore most of the productive soils have been scraped into the surrounding berms. This has opened the site up to a range of weed species. The equipment storage area contains a range of heavy equipment, culverts metal towers or scaffolds, small sheds for additional storage and various other materials. There are a few piles of debris in the form of wood and sheet metal noted on site as well.

The former gravel pit is located north of the laydown area and was previously accessed from the north quarter. Access to the gravel pit from the north crosses a shallow portion of the ravine that
may have been previously filled. A large diameter culvert was observed with some flowing water draining to the east. Access to the site from the north was blocked by large concrete barriers put in place by the current owner. The gravel pit appears to have been mined out with only fine grained materials observed on site.

The central portion of the site is currently canola crop. A new residence was observed in the central west portion of the site just south of the laydown area. There were no other structures or storage of materials or equipment noted in the crop portion of the site.

The southeast portion of the quarter shows two privately owned acreage properties as shown on the survey plan in Figures $3 \& 4$.

### 4.2 Historical Aerial Photo Review

A random sample of historical aerial photos was selected for examination based on the best available scale, variation in the time of year, and level of coverage. The earliest photo on file at ASRD was 1949 and the most recent was from 2011. An aerial photo from Wescott Consulting was examined from 2012 as well. The following paragraphs provide a summary of relevant site observations. A table of the aerial photo review is provided as Table 1.0.


Figure 5: Aerial photo gravel extraction 1967


Figure 6: Aerial photo showing gravel pit on site in 2000

Table 1: Aerial Photo Review

| Year | Observations | Scale |
| :--- | :--- | :--- |
| 1949 | No time of year indicated but leaves are visible. The north side of the ravine is <br> sparsely treed. From the Ravine south the majority of the central portion of the <br> quarter section appears fully cleared and cultivated but growth is not yet evident | $1: 40,000$ |
| (Spring?). The south end and southeast is still forested and there is a farm <br> located to the southeast. There is a building or something at the central portion <br> of the west side of the quarter. The majority of the site appears well drained <br> Township road 540 is fully developed to the south and range road 21 is fully <br> developed to the east. No sign of gravel extraction in the surrounding quarters. <br> Agriculture dominates the surrounding area. |  |  |
| 1967 | (August) Date on the photo is August. The north side of the ravine appears to <br> be infilling with vegetation. The site is cultivated with little change from 1949. <br> The central west portion of the site has a cluster of trees where an anomaly <br> appeared in 1949. In the centre of the quarter there is a disturbance visible and <br> may be equipment storage. The farm in the southeast now appears to have <br> two separate driveways and maybe two farms. Two entrances are on the <br> southeast and the other is on the east side. There is gravel extraction east of <br> the site in the adjacent quarter. Agriculture is on all other surrounding lands. |  |


| Year | Observations | Scale |
| :--- | :--- | :--- |
| 1976 | (September) Site is fully cultivated and very little change on site is evident from <br> previous photo with the exception of increased density of trees on the north <br> side of the ravine. The gravel extraction to the east appears to have expanded <br> north and there are several setting ponds adjacent the Kilini Creek. | $1: 20,000$ |
| 1984 | (June) The site is cultivated. Looks like there has been some clearing of trees <br> on the south side of the ravine at the north end of the site to expand cultivated <br> lands. There is a wetland at the east central portion of the site visible at this <br> scale and very clear. There is extensive sand and gravel extraction to the east <br> still and now in the quarter to the north. | $1: 15,000$ |
| 1992 | (May) The same cultivated lands are still visible and there appears to be more <br> clearing at the south end of the site around the farm. The north and east side <br> of the ravine shows signs of an extensive trail network in the forest. The gravel <br> extraction from the north has dipped into the north west corner of the quarter <br> on the north side of the ravine. | $1: 10,000$ |
| 2000 | (August) The quarter is cultivated. There is a gravel extraction in the northwest <br> corner on the south side of the ravine and access to the pit is from the north. A <br> crossing over the creek/ravine has been constructed from the north. Clearing <br> for the gravel extraction area appears to have created stockpiles of soil near <br> the middle of the quarter at the north end adjacent the ravine. Long berms are <br> evident and they appear to be vegetated. The wetland at the east side of the <br> quarter is visible as a depression but there does not appear to be open water. <br> Many of the trees in the southeast around the farm have been cleared and it <br> appears there are two distinct access routes to the farm in the south east one <br> from the south and one from the east. There appears to be another residence <br> in the central part of the south side of the quarter. The residences are <br> separated by cultivated and cleared lands where previously there was forest. <br> The gravel pit to the east of the site appears to be | $1: 20,000$ <br> 2005(May) The gravel extraction in the northwest corner is still visible. The majority <br> of the land is still cultivated. The wetland re are rows of round bales visible. No <br> sign of wet area in the north anymore. |
| $1: 20,000$ |  |  |
|  | (Time of year unknown) Fully cultivated. No signs of wet area but vegetation is <br> evident in the low area. Gravel pit does not appear to be in use. | $1: 20,000$ |

Aerial photos reveal that the site has been cultivated since prior to 1949 and gravel extraction in the quarter to the east was observed as early as 1967. The ravine across the north boundary provides a history of intermittent drainage to the east. Observations also seem to indicate that the wetland in the east portion of the site may once had been a class V with open water however field observations in August of 2015 show cattails dominate the vegetation across the entire low area. Periods of low precipitation have been evident this summer impacting wetlands, local agriculture and crops in Central Alberta.

### 5.0 THE EXISTING ENVIRONMENT

### 5.1 Climate and Air Quality

The proposed development site lies very near the division between the Dry Mixedwood SubRegion of the Boreal Forest Natural Region of northeastern Alberta, and the Central Parkland Sub-Regions (Alberta Environmental Protection, 2006 Natural Regions and Sub-Regions of Alberta (map).
The climate is sub-humid, continental with short, cool summers and long, cold winters. The mean May - September temperature within the Sub-Region is about 11-13 degrees celcius and the growing period is about $80-90$ days. Annual precipitation averages about 380 mm with June and July the wettest months. Winters are relatively dry with about 60 mm of precipitation.
The most frequent wind direction varies minimally over the year. Wind directions are mainly from the northwest except during February, April, July and December when they are predominately from the west. Monthly wind speeds are fairly consistent throughout the year ( 9.3 to $11.9 \mathrm{~km} / \mathrm{hr}$.).
Climatic data from Environment Canada was compiled for the 30 year period, 1978 to 2008, from the Edmonton Stony Plain Station; located approximately 10 km southwest of the Property.
Over the period 1978 to 2008, the average monthly temperature for the winter months (December to February) was $-9^{\circ} \mathrm{C}$ with extreme monthly temperature fluctuations ( $-17.5^{\circ} \mathrm{C}$ to $+39^{\circ} \mathrm{C}$ ) over the 30 year period. Average monthly winter precipitation is $19.15 \mathrm{~mm} ; 92 \%$ of which consists of snow.
The average monthly temperature for the spring months (March to May) was $4.3^{\circ} \mathrm{C}$; increasing from a March average of $-2.7^{\circ} \mathrm{C}$ to a May average of $10.8^{\circ} \mathrm{C}$, with extreme (average) monthly temperature fluctuations of $-11^{\circ} \mathrm{C}$ to $+20^{\circ} \mathrm{Cover}$ the 30 year period. Average monthly precipitation is $33 \mathrm{~mm} ; 67.6 \%$ of which consists of rain.

The average monthly temperature for the summer months (June to August) was $15.8^{\circ} \mathrm{C}$; with mean minimum and maximum temperatures of $10^{\circ} \mathrm{C}$ and $20^{\circ} \mathrm{C}$, respectively, over the 30 year period. Average monthly precipitation is 85 mm ; all of which consists of rain.

The average monthly temperature for the fall months (September to November) was $4.1^{\circ} \mathrm{C}$; decreasing from a September average of $-11^{\circ} \mathrm{C}$, an October average of $5.6^{\circ} \mathrm{C}$, to a November average of $-4^{\circ} \mathrm{C}$ over the 30 year period. Average monthly precipitation is 32 mm ; with $97 \%$ falling as rain in September, $54 \%$ as rain in October and $<1 \%$ as rain in November.

Gravel extraction related industrial activities would be expected to contribute to local air emissions. Higher particulate air quality levels may occur on a temporary basis as a result of agricultural and industrial (e.g., construction, road dust, fires) activity. Vehicle exhaust fumes may be another source of minor air pollution, given the proximity to populated areas and roads.

### 5.2 Physiography and Topography

Dominant Landforms of the Dry mixed wood region include undulating plains and hummocky uplands.
The slope of the Property is generally trending to the southeast. At the north end of the site the land drops steeply into the Ravine that bisects the northern portion of the quarter from west to east. The base of the ravine shows evidence of an intermittent drainage course flowing to the east. The 1:50:000 topographic map sheet $83 \mathrm{G} / 9$ shows prior to any manmade intervention, elevations on site ranged from 740 in the northwest to 720 m in the southeast.

One of the lowest drainage areas on site is near the wet land on the east boarder of the property. This low area acts as a minor collection point for surface water drainage during spring melt and major precipitation events.


Figure 7: Site Drainage (2011 Aerial photo)

On-site topography was recently confirmed through a survey completed by Airborne Imaging. The site surveys indicate drainage is toward the south and southeast towards areas of lower elevation on site. A copy of the survey figures is contained in Appendix B.

### 5.3 Geology and Soils

Surficial materials are dominantly medium to moderately fine textured, moderately calcareous glacial till that may be a thin (less than 2 m ) blanket over bedrock in some of the low-relief plains. In the eastern part of the Natural Subregion, about 15 percent of the area is covered by glaciolacustrine and glaciofluvial sediments occurring as inclusions within the till plains.

The AGRISID database search was completed for the parcel of land. The database listed the central portion of site as polygon 19125 WSR16/IUh. Based on published information within Agrisid, native soils on site are expected to consist of Orthic Gray Luvisol (CL, SCL, SiCL) on moderately fine textured sediments deposited by water. This area also includes Chernozemic soils (16).
The land suitability rating is listed as 2 H 10 which indicates the site is $80 \%$ class 2 soils sub-class H , and $20 \%$ class 2 sub-class W . As mentioned previously the soils on site are considered productive for agricultural land.

### 5.4 Surface Water

### 5.4.1 Drainage Patterns on the Property

Surface water drainage on site appears to follow topographic contours directing overland flows toward the southeast portion of the site. The ravine provides periodic drainage service for the regional set to the north and west.

From the recent contour data it appears that generally the surface drainage flows in a south to east direction however local drainage does accumulate in the east border of the site as a small wetland. Over time the wetland appears to have transitioned from an open water wetland to more of a depression. This may be the result of lower than average ranges of precipitation during 2014 and 2015.

### 5.4.2 Drainage around the property

The closest mapped water body within 5 km of the site is Kilini creek, located approximately 2 km to the east. Based on a review of elevations on the topographic map sheet for the area it appears that the general regional trend of surface water drainage is to the north and east of the subject site towards Coal Lake. Highway 13 shows a definite drop in elevation as it moves east from Lac Ste. Anne. This tends to be reflected in the contours on the map sheet of the area as well

### 5.5 Groundwater

A search of the Alberta water well database revealed six groundwater well records for the SE 1-54-2 W5M. Based on an overview of the six records on file groundwater could be encountered at depths of approximately 8.5 meters to 20 meters below ground level. A summary of the wells reconnaissance report is listed in Appendix G.

### 5.6 Vegetation

The biophysical assessment was conducted in part to describe and interpret vegetation communities as they existed at the time of the field reconnaissance, which took place in late August 2015. Most species of plants are visible and identifiable at that time of year but because they are not all in flower some may have been missed. In some cases, while the genus of the plant was recognized, it may not have been possible to identify it to species. In addition the slope on the south side of the ravine was too steep to closely inspect vegetation.
This section provides a description of the vegetation communities observed on the site in the field reconnaissance, preceded by a general description of vegetation in a regional context

### 5.6.1 Regional Context

The Property is located within the Dry mixed wood Natural Sub-region. This sub-region is characterized by warm summers and a long growing season. Aspen forests with mixed understories are normally found in the uplands and fens with sedges or shrubs commonly found in the low areas

### 5.6.2 Site Context

The following basic types of vegetation community were observed on the study site:
Central crop land - one new residential house, canola and various weeds such as Canada thistle, scentless chamomile, and

Ravine - the ravine consists of an Aspen and Balsam forest ranging in size from 2-3 cm to 40 cm at breast height (ABH). A few white spruce were noted in the base of the ravine. Understory species included a combination of beaked hazelnut, highbush cranberry, wildrose and bearberry with sasparilla, horsetail and fern. Ground cover included strawberry and grasses.

North side of the Ravine - the north side of the ravine has a gradual slope where the south side is very steep. Both sides are forested but understory vegetation appears more dense on the north side likely due to the more gradual and stable slope. There is a range of trails on this side of the ravine providing access east and west across the top of the ravine as well as offshoots that provide access south into the ravine base. There is a good range of size in Aspen and Balsam poplar from 2-3 cm up to 30 cm ABH. Suckering of the trees is evident into the trails and open cut areas. The understory here is similar to the lower ravine but includes Saskatoon berry, as well. No spruce was observed on the upland sites on either side of the of the ravine. For the most part, the trees in this stand appeared to be vigorous, without signs of excessive or abnormal dieback, breakage or signs of disease. There were some snags and hanging branches and some trees showing signs of fungus which did not seem abnormally represented. The canopy seems robust and healthy.
Northwest side of the ravine - there is an open area just north of the forest where the grade and elevation of the area is higher than the surrounding. It is possible that topsoil or subsoils were located in this area when gravel was extracted. The site is now occupied by willow, purple pea vine and low brush species.

Northwest laydown area - extensive weeds such as thistle, scentless chamomile, meadow hawkweed, dandylion, common tansy and leafy spurge.

Wetland - cattails, sedges, and willow.

### 5.7 Wetlands and Other water bodies

There is one wetland in the east central portion of the site where historically it appears surface water periodically collects and pools. The vegetation on site includes cat tails, grasses sedges and willow. The wetland was not closely assessed on foot for additional detailed species list. Historical aerial photos show open water during past years and the presence of Typha latifolia (cattails) around the perimeter of the site would indicate a class V wetland. The lower levels of precipitation this year however appear to have reduced the amount of open water and cat tails appear to be filling in toward the middle of the wetland thus it could be considered a class IV.

### 5.8 Wildlife

### 5.8.1 Regional Context

The subject Property is located in the Dry mixed wood sub-region of Alberta. Bird species typically inhabiting forested areas in the area include, warblers, like Black-And-White Warblers, American Redstart, Song Sparrow, Northern Water Thrush, Fox Sparrow and Philadelphia Vireo.

Some of the species listed in this sub-region include, Canadian Toad, Northern Leopard frog, redsided garter snake. Also expected in the area would deer, coyote, cougar, and moose.

### 5.8.2 Wildlife on the property

With the exception of birds, warbler and woodpecker, wildlife was not observed on the property. Signs of scat from coyotes and ungulates were observed in various locations. The Fish and wildlife internet mapping tool was used to generate a list of species expected in the area. Animals in the area include Canadian Toad, Red Sided Garter Snake and Northern Leopard Frog. A copy of the results is provided in Appendix E .

### 6.0 BIODIVERSITY

### 6.1 Species Richness

Site observations were made in August, when flowers were still evident and in some cases seeds. The three areas on site offer very different ecological settings. The central agricultural lands under crop provides little variation in ecological setting and therefore low biodiversity.

The ravine presented a wide range of species from the upland slopes down into the base of the ravine. There is a good amount of succession evident in the deciduous tree species with diameter at breast height ranging from 3 cm to 40 cm . The understorey is well developed with a healthy ground cover presenting a wide range of species including fungi and lichen. There are several fallen logs and leaners left to decay offering a suitable habitat for a range of plant and animal species.
The wetland to the east is somewhat isolated from the ravine and is bordered by the cropland. There is a bit of a buffer around the wetland where some transitional vegetation such as willow are evident.

### 6.2 Species at Risk

No rare, threatened or endangered species of plants or animals were observed during the field surveys however site visits are limited by the time of year and the extent of the survey limited to a one-day visit.
A database search for rare, endangered or listed species by the Alberta Conservation Information Management System (ACIMS) was completed for this Property. In its response, ACIMS showed no sensitive occurrences noted in the study area. Nonsensitive occurrence's included Cladonia stygia commonly referred to as reindeer lichen. No provincially protected areas (PPA's) were returned by the inquiry. A copy of the ACIMS generated report is provided as Appendix D .
In respect to animal species, there were no reported occurrences received from the ACIMS enquiry. This information indicates that the site has little potential to provide habitat for sensitive species.

### 6.3 Threats to Biodiversity

In general, disturbance to, or loss of undisturbed vegetation translates into loss of ecological habitat niches, resulting in a decline in biodiversity. The more sensitive and the less resilient the predominant vegetation community is, the greater the risk of biodiversity loss in a given area. The ravine provides a unique and meaningful area for biodiversity and an important natural corridor for wildlife to travel safely and take cover.

Risks to biodiversity include the following:

- Soil erosion and instability near the top of the ravine bank,
- Weed infestations, and;
- Changes in run off patterns which could affect the trees and other vegetation through desiccation or flooding or;
- Run off patterns that would divert drainage from the wetland at the east central portion of the site.


### 7.0 SUSTAINABILITY

Ecosystem sustainability can be impacted by several variables such as, the size of the system (e.g., the length and width of a stand of trees), the health and sensitivity of the soil, land slope angles and aspect, soil drainage and supply of moisture, the depth of the water table and supply of groundwater, and the exposure and susceptibility of the system to the extremes of weather. The ravine at the north end of the site provides a setting for a sustainable ecosystem to exist. The preservation of buffer zones around the top of the Ravine will help maintain structural stability of the ravine as well as provide a valuable drainage area and support diversity in the overall ecosystem for the long term. The maintenance of the feature as a drainage route will also help preserve the ecological setting. The ecosystem has evolved to act as a drainage route during higher than usual precipitation events, meltwater and heavy overland flows.

The crop land in the central area does not represent a sustainable natural ecosystem as it has been cultivated since prior to 1949. The wetland at the east border of the site has appeared over time to act as a natural drainage feature which is protected through legislation. The depression in that area appears to have recently been impacted by lower than average precipitation reducing its visible surface water size, however vegetation observed on site indicate that it is a permanent pond and should be considered a class $\vee$ wetland. The wetland has the potential to expand during years of increased precipitation. The sustainability of a wetland is dependent on the maintenance of the surrounding lands that contribute drainage to the site. The key concern in future development of the central portion of the site will be to design storm water management plans in such a manner that post-development surface water flows are approximately similar in flow rates and periodicity to what was prevalent in the pre-development condition.

### 7.1 Ecological linkages

In the central parkland ecological region, ecological connectivity has been impacted by the extensive agricultural and resource extraction land uses. Linkages that do exist rely mainly on forested areas, river and creek valleys which provide visual and thermal cover for many forms of wildlife, as well as nutritional support. Moose and deer, for example, move readily along corridors of forest, because it provides them with visual cover from predators, as well as providing them with a source of food in the form of leafy vegetation. Although they venture out into the open to travel or to access other sources of food, deer prefer to stay within about 180 m of forest cover. Extensive forest areas, therefore, serve as movement corridors for these animals.

Similarly, birds use forest corridors for habitat, visual and thermal cover and the acquisition of food, and because they can fly, easily disperse among patches of forest on the landscape. Mammalian wildlife such as moose, deer, coyotes, furbearers and even amphibians, also use patches of treed areas to disperse with greater safety. These adjacent patches are referred to as "stepping stones", while long bands of forest are called "corridors". Patches of forest that is sufficiently large to support all of the life-cycle activities of animals are equally important. Wetland habitats are also important as stepping stones to habitat with secure brood water. Generally, smaller temporary wetlands are used by waterfowl to disperse pairs into nesting habitat on the landscape, thereby maximizing breeding opportunity. However, after nests have hatched broods are often moved to more secure water until young have fledged.
Amphibians, generally, require upland sites in which to feed and over-winter, but must return to water to reproduce. The distance between wetlands or other waterbodies, and suitable undisturbed upland sites becomes critical when land developments threaten amphibian habitats of one type or another.

In the broader context, aerial photos of the property and surrounding lands as they appeared in 1949 and 2011, shown in Figures 4 and 5 respectively, indicate that the ravine has offered sustainable ecological linkages to adjacent areas.


Figure 8: Potential ecological linkages, based on wooded areas and wetland/thicket areas in the vicinity of the project site (imagery from Google Earth, 2011). White arrows indicate potentially "permeable" wooded corridors or stepping stone patches amongst core habitats (implies travel in both directions). Orange line indicates boundary of the study site

### 7.2 Mapped and Classed Waterbodies in the Area

The Alberta Environment classification system for waterbodies in Alberta is based generally on the abundance and quality of fish habitat. Class $A$ is the highest priority class, and class $D$ is the lowest (except for "unmapped" watercourses which are not shown on the maps at all). Any mapped waterbody must follow the Code of Practice for Watercourse Crossings, which falls under the Alberta Water Act. Any classed waterbody must also observe a Restricted Activity Period specified for it, for conducting in-stream construction or related activities.
The only mapped waterbody within 5 km of the site is Kilini Creek, which is 0.8 km east of the subject site. Drainage from the ravine is expected to flow to the east toward Kilini Creek.

### 8.0 RECOMMENDATIONS

The following recommendations are made for consideration in the possible future development of residential lots at the site for design, construction and post-construction phases. If gravel extraction is to be pursued at the site the specific requirements for pit development, operation and management can be found at Alberta Environment and Parks. A copy of the Code of Practice for pits has been included as Appendix $B$ as part of this report.

### 8.1 Residential subdivision design

To the extent feasible, the natural contours of the land should be considered in order to conserve the natural drainage patterns and flows, and to moderate storm water drainage patterns, thus damping out extremes of overland flow, avoiding erosion and promoting the settling of solid particulate matter.
Gradual slopes and vegetated swales should be incorporated in the contouring and landscaping of the development in the flat areas surrounding the laydown area. By slowing down the runoff, this provides more opportunity for particulate matter to settle, and nutrients to be removed.

### 8.2 Opportunities for habitat conservation

The ravine and surrounding upland should be protected with a development buffer. Opportunities to enhance habitat around the subdivision development can be taken through the addition of trees as part of site restoration and landscaping efforts.
Invasive weedy plants should be prevented upon construction completion through the use of revegetation strategies where applicable. The use of native grasses will reduce future management requirements if pursued immediately upon construction completion

### 8.3 Surface water and groundwater

In addition to any Lac St Anne MDP requirements, a master drainage plan as per the Storm water Management Guidelines for the Province of Alberta would be beneficial as part of the overall storm water surface water management plan for any developed area on the Property. The plan should aim at maintaining overland runoff at approximately pre-development conditions, to reduce the risk of flood on adjacent properties as a result of altered runoff patterns. Consideration of changing weather systems as a result of climate change should be incorporated into future designs. This may require anticipation of major rainfall events and related impacts on surface water drainage.
Preservation of the existing wetland on the east central portion of the site should be made a priority by preserving existing drainage to and around the area.
The subdivision should be designed to achieve effective settlement of suspended solids in accordance with the current Alberta Environment guidelines (see above). Otherwise, discharge points from the ditches should be into a vegetated or other area where energy dissipation will occur, or the equivalent, in order to prevent local erosion. Where necessary, roads and their
ditches should have flow-limiting structures (e.g., ditch blocks) along sloped stretches so that water draining off or along them does not cause erosion.

Surface paving should be minimized, other than what is necessary for transportation, parking and other infrastructure in order to maintain permeability and thus groundwater re-charge.
Where excavated, topsoil should be conserved and stockpiled with protection from wind and water erosion, and then replaced after construction, being careful not to mix topsoil and subsoil layers
Where any clearing or earthworks are necessary, strict erosion control and reclamation measures should be taken. Contouring should avoid significantly increasing the angle of slopes, such as would lead to erosion and instability.

In order to conserve water landscaping should be completed through the use of native, droughttolerant plants. This will reduce the need to water plants and reduce the need to use chemical fertilizers.

### 8.4 Environmental sustainability and Community Participation

The use of fixtures that reduce light pollution, especially avoiding projecting light into natural areas. Fixture design and placement should ensure that most of the light produced should be projected downwards, rather than laterally into the surrounding environment. The use of long wavelength bulbs should be encouraged, as they have less impact on wildlife.

### 8.5 Ecological linkages

The ravine located on the north end of the property offers important ecological linkages to the properties to the east and west of the site. The ravine provides a movement corridor, food and shelter for wildlife. The Lac St Anne MDP has identified this creek as an important waterway with a 70 meter set back buffer.

### 8.6 Hazards Wastes and Disturbances

The laydown area in the northwest contains heavy equipment such as graders, backhoes, forklifts, trucks and trailers. There are also wooden rig matts, metal towers, plastic containers, metal culverts, and smaller storage sheds. Various piles of debris on site appeared to contain metals and wood. There is also an ATCO trailer on site. Should the land use in this area change it may be necessary to complete a phase I ESA to verify the environmental integrity of the area as this land use offers the potential to cause contamination as a result of leaks or spills from equipment and storage.

### 8.7 Gravel Pit Reclamation

Prior to gravel pit reclamation the registration application must be accepted by the Alberta Government and the plan deemed acceptable. Access to the site will need to be coordinated with the current land owner. Due to the ecological significance of the drainage area and the extremely
steep slope, adequate buffers from the top of the ravine must be maintained. The site reclamation plan has been outlined in the Pit registration application mentioned in section 2.1.2.

### 9.0 CONCLUSIONS

Due to the agricultural activities on site, the majority of the upland property presents a lack of biological diversity with poor range of habitat available for plant and animal species. There do not appear to be any biological restrictions to the development of this area.

The ravine across the north end of the property does provide an ecologically diverse setting that provides valuable habitat and cover for a wide range of species. The area serves as a movement corridor providing cover and habitat as well as an ecological linkage across property borders. This area should be protected from future development through the maintenance of setbacks and buffers. There were no species at risk noted for the property

There is one small wetland located on the east border of the property that has been evident in aerial photos since 1949. The wetland is a naturally occurring low area and serves as a catchment for the north and east side of the property. Drainage toward the wetland should not be significantly modified so as to allow for the wetland to continue to thrive and provide a catchment for local overland flows.

The subject property has been in use for agricultural purposes since at least 1949 and consists of productive agricultural land in the form of a class 2 soil. While there are no specific limitations related to the development of the land for wildlife habitat, regulators and the developer will likely recognize that development of the site will result in a decrease to local productive agricultural land. This can be considered a common consequence of development however it should be planned around compatible land uses at appropriate times.
Recommendations for future studies prior to the development of the land include the following.

- Master drainage plan and erosion control strategies
- Fire prevention and control strategies
- Site hazard management planning
- Gravel extraction planning and timing of activities

These recommendations are not intended to be exclusive. Regulators may have additional requirements not listed here. Reference should be made to the recommendations provided in this report along with various other planning tools during future site development including the municipal Development Plan.

### 10.0 CLOSURE

We trust this report meets your current requirements. If you require any additional information please do not hesitate to contact the undersigned.


Melinda McLauchlin, C.Tech
MCA Environmental Management
RR2 Bluffton, Alberta TOC OM0
Phone: (403) 843-2960
Cell: (780) 266-7245

Paul McLauchlin, P. Biol
Senior Review
Environmental Leadership Matters
Phone: (403) 843-2675
Cell: (780) 995-7339

1. Alberta Culture, 1984. A Checklist of the Rare Vascular Plants in Alberta.
2. Alberta Environmental Protection, 1993. Alberta Plants and Fungi - Master Species List and Species Group Checklists.
3. Alberta Environmental Protection. 1996. The Status of Alberta Wildlife. Natural Resources Service, Wildlife Management Division.
4. Alberta Vegetation Inventory Standards Manual. February 2003. SRD Public Lands Division. Resource Data Branch.
5. Lac Ste. Anne County | Land Use Bylaw No. 24-2014 \& Development Regulation and Administrative Procedures
6. Committee on the Status of Endangered Wildlife in Canada, COSEWIC). 1997. Current list of vulnerable, threatened and endangered species in Canada.
7. Federation of Alberta Naturalists/ City of Edmonton, 2006. Living near Urban Lakes.
8. Kershaw, L., Gould, J., Johnson, D. and Lancaster, J., Rare Vascular Plants of Alberta. University of Alberta Press, 2001.
9. Primeau, S., Bell, M., Riopel, M., Ewaschuk, E., and Doell, D. 2009. Green Communities Guide: Tools to Help Restore Ecological Processes in Alberta's Built Environments. Land Stewardship Centre of Canada. Edmonton, Alberta.
10. Provincial Wetland Restoration/ Compensation Guide (Alberta Environment/NAWMP, February 2007)
11. Stewart, R.E. and H.A. Kantrud, 1971. Classification of Natural Ponds and Lakes in the Glaciated Prairie Region. US Department of the Interior, Fish and Wildlife Service. Resource Publication No. 92.
12. Twerdoff \& Associates Inc. Aggregate Characterization Assessment SE 1-54-2 W5M, June 2014
13. The 2005 General Status of Alberta Wild Species: Website http://www.srd.gov.ab.ca/fishwildlife/wildspecies/search.htm
14. Alberta. Fiera Biological Consulting Report Number 1305.Environmentally Significant Areas of Alberta Update - September 2014
15. Government of Canada. Fisheries Act. R.C.S 1985, c. F14. Section 35. Serious Harm to Fish. www.canada.ca
16. Government of Canada. Species at Risk Act (SARA). S.C 2002, c. 29 www.canada.ca
17. Government of Canada. Federal Policy on Wetland Conservation, 1991. www.canada.ca
18. Province of Alberta. Public Lands Act, R.S.A. 2000, c. P-40. www.alberta.ca
19. Province of Alberta. Environmental Protection and Enhancement Act (EPEA). 2000 s. 109 Prohibited release where no approval or regulation. www.alberta.ca
20. Province of Alberta. Water Act. Revised statutes of Alberta 2000. www.alberta.ca
21. Province of Alberta. Weed Control Act. Statutes of Alberta 2008, amended 2010. www.alberta.ca
22. Province of Alberta. Wildlife Act. Revised Statutes of Alberta 2000. www.alberta.ca
23. Province of Alberta. Historical Resources Act. Revised Statutes of Alberta 2000. www.alberta.ca
24. Province of Alberta. Wetland Policy. 2013. www.alberta.ca
25. Province of Alberta. Environmental Protection and Enhancement Act (EPEA). Wastewater and Storm Drainage Regulation (AR 119/93) and the Wastewater and Storm Drainage (Ministerial) Regulation (AR 120/93) Part 4, Division 1 www.alberta.ca
26. Province of Alberta. Stormwater Management Guidelines.1999. www.alberta.ca
27. Lac Ste. Anne County. DRAFT Municipal Development Plan. http://www.Isac.ca/public/download/documents/13233

## APPENDIX A



Photo 1: Southeast corner of the property has a sign warning against the spread of weeds.


Photo 2: North east side of the laydown area in the northwest corner of the property. Various weed species present due to clearing of topsoil.


Photo 3: Berm surrounding the laydown area provides a visual barrier between cropland and weeds.


Photo 4: Forested area at the base of the ravine where evidence of cut trails exist.


Photo 5: Typical understory in the base of the ravine.


Photo 6: Larger diameter trees in the base of the ravine $\sim 45 \mathrm{~cm} \mathrm{ABH}$


Photo 7: Creek in this portion of the ravine did not have running water evident. Portion of the creek in the base of the ravine.


Photo 8: Healthy detris in the base of the ravine provides valuable habitat.


Photo 9: Sample of the understorey in the ravine on the north side of the drainage course.


Photo 10: North side of the ravine part way up the hill. Slope on the north side is gradual.


Photo 11: Upland species on the north side of the ravine surrounding the trail system.


Photo 12: Trees are starting to show some succession but average DBH is about 20 cm .


Photo 13: Diverse health understory at the junction of trails to the north and south as
well as east and west.


Photo 14: Trail system on the north side of the ravine shows smaller diameter trees similar in diameter.


Photo 15: At the edge of the forested ravine on the north side Peavine.


Photo 16: At the northwest side of the ravine. Forest to field transition with willow.


Photo 17: Snags provide habitat.


Photo 18: Looking south at the ravine/creek crossing at the north west.


Photo 19: Water in the culvert at the crossing site in the northwest.


Photo 20: Looking north from the NW gravel pit at access road blocked by concrete. .


Photo 21: Gravel pit in the northwest has not been reclaimed.


Photo 22: Equipment storage in the laydown area.


Photo 23: Sheds and material storage.


Photo 24: Looking south on the access road from the laydown area.


Photo 25: New house at the west side of the crop land.


Photo 26: Wetland at the east side of the property. Cattails are infilling.


Photo 27: Shrubs surrounding the wetland at the east side of the property.

## APPENDIX B

APPENDIX B: List of plant species observed in vegetation survey of Lac St. Anne property, August 2015. Arranged in alphabetical order by scientific name

## Common Name

A. Trees and Shrubs:

## Alder

Saskatoon
White birch
Siberian Peashrub
Red-osier dogwood
Beaked Hazelnut
White Spruce
Balsam poplar
Trembling aspen
Chokecherry
Beaked Willow
Sandbar Willow
Western Shining Willow
High bush cranberry

Scientific Name

Alnus rugosa
Amelanchier alnifolia
Betula papyrifera
Caragana arborescens
Cornus stolonifera
Corylus cornuta
Picea glauca
Populus balsamifera
Populus tremuloides
Prunus virginiana
Salix bebbiana
Salix exigua
Salix lasiandra
Viburnum opulus
B. Forbs, Herbs and Other Vascular Plants:

| Yarrow | Achillea millefolia |
| :--- | :--- |
| Baneberry | Actaea rubra |
| Slender wheat grass | Agropyron trachycaulum |
| Rough hair grass | Agrostis scabra |
| Canada anemone | Anemone canadensis |
| Wild sarsaparilla | Aralia nudicaulis |
| Fringed aster | Aster ciliolatus |
| Milk vetch | Astragalus sp. |
| Slender wheat grass | Agropyron trachycaulum |
| Short-awned foxtail | Alopecurus aequalis |
| Blunt-leaved sandwort | Arenaria lateriflora |
| Brome grass | Bromus spp.. |
| Marsh Reed Grass | Calamagrostis |
| canadensis | Capsella bursa-pastoris |
| Shepherd's purse | Carex atherodes |
| Awned Sedge | Cirsium arvense |
| Canada thistle | Cornus canadensis |
| Bunchberry | Disporum trachycarpum |
| Fairy bells | Dryopteris spinulosa |
| Shield fern |  |


| Spike rush | Eleocharis sp. | Wild raspberry | Rubus idaeus |
| :--- | :--- | :--- | :--- |
| Fireweed | Epilobium angustifolium | Trailing raspberry | Rubus pubescens |
| Common Horsetail | Equisetum arvense | Arum-leaved arrowhead | Sagittaria cuneata |
| Swamp Horsetail | Equisetum fluviatile | Buffaloberry | Shepherdia canadensis |
| Meadow Horsetail | Equisetum pratense | Star-flowered False Solomon's Seal Smilacina stellata |  |
| Fescue | Festuca sp. | Goldenrod | Solidago canadensis |
| Wild strawberry | Fragaria virginiana | Sow thistle | Sonchus arvensis |
| Narrow-leaved Hawkweed | Hieracium umbellatum | Bur-reed | Sparganium sp. |
| Foxtail barley | Hordeum jubatum | Marsh Hedge Nettle | Stachys palustris |
| Creamy Peavine | Lathyrus ochroleucus | Chickweed | Stellaria, Cerastrium sp. |
| Twining honeysuckle | Lonicera dioica | Snowberry | Symphoricarpos albus |
| Bracted honeysuckle | Lonicera involucrata | Dandelion | Taraxacum officinale |
| Wild lily-of-the-valley | Maianthemum canadense | Veiny meadow rue | Thalictrum venulosum |
| Yellow sweet-clover | Melilotus officinalis | Cattail | Typha latifolia |
| Tall Lungwort | Mertensia paniculata | Stinging nettle | Urtica dioica |
| Timothy | Phleum pratense | Wild vetch | Vicia americana |
| Common Plantain | Plantago major | Western violet | Viola canadensis |
| Fowl bluegrass | Poa palustris | Kidney-leaved Violet | Viola renifolia |
| Wintergreen, Pink | Pyrola asarifolia |  |  |
| Yellow Water-crowfoot | Ranunculus gmelinii | Ribes glandulosum | C. Non-Vascular Plants: |

## APPENDIX C

## Alberta Code of Practice for Pits

http://www.qp.alberta.ca/documents/codes/PITS.PDF
Guide to the Code of Practice for Pits
http://environment.gov.ab.ca/info/library/5997.pdf

## APPENDIX D

## Search ACIMS Data



Select Reason for Request: * Land Use Planning
(3) sEC

(option)
Convert Lat/Long to Township

Submit
Layers
$\nabla \square$ Element Occurrences (part one, non-sensitive)
$\square \square$ Element Occurrence (part two, sensitive)
$\checkmark \square$ Protected Areas
$\nabla \square$ Crown Reservation/Notation
$\nabla \square$ 100 m Proximity - Protected Areas

- $\square 100$ m Proximity - Crown Reservation/Notation
* Required

Table of Results Print Preview
Date: 16/9/2015
Requestor: Consultant
Reason for Request: Land Use Planning
SEC: 01 TWP: 054 RGE: 02 MER: 5


Updated: July 22, 2015
Today: September 16,

| Submit |  |
| :---: | :---: |
| Layers <br> $\square$ Element Occurrences (part one, non-sensitive) | ( $)$ |
| $\square \square$ Element Occurrence (part two, sensitive) | ( 3 |
| $\checkmark \square$ Protected Areas | (3) |
| $\checkmark \square$ Crown Reservation/Notation | ( $)$ |
| $\checkmark \square 100 \mathrm{~m}$ Proximity - Protected | ( $)$ |
| $\boxtimes \square 100 \mathrm{~m}$ Proximity - Crown Reservation/Notation | ( $)$ |

Non-sensitive EOs: 1 (Data Updated:July 2015)
M-RR-TTT-SS EO_ID ECODE S_RANK SNAME SCOMNAME LAST_OBS_D

## Table of Results Print Preview

Date: 16/9/2015
Requestor: Consultant
Reason for Request: Land Use Planning
SEC: 01 TWP: 054 RGE: 02 MER: 5
$\square$ Non-sensitive EOs: 1 (Data Updated:July 2015)

| M-RR-TTT-SS | EO_ID | ECODE | S_RANK | SNAME | SCOMNAME | LAST_OBS_D |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5-02-054-01 | 6059 | NLTEST7190 | S2S4 | Cladonia stygia | reindeer lichen | 1-Jun-87 |

Next Steps: See FAQ
$\square$ Sensitive EOs: 0 (Data Updated:July 2015)
M-RR-TTT EO_ID ECODE S_RANK SNAME SCOMNAME LAST_OBS_D

No Sensitive EOs Found: Next Steps - See FAQ

Protected Areas: $\mathbf{0}$ (Data Updated:May 2015)
M-RR-TTT-SS PROTECTED AREA NAME TYPE IUCN

No Protected Areas Found

## Crown Reservations/Notations: $\mathbf{0}$ (Data Updated:May 2015)

$\square$ Sensitive EOs: 0 (Data Updated:July 2015)
M-RR-TTT EO_ID ECODE S_RANK SNAME SCOMNAME LAST_OBS_D

No Sensitive EOs Found: Next Steps - See FAQ
Protected Areas: $\mathbf{0}$ (Data Updated:May 2015)

| M-RR-TTT-SS PROTECTED AREA NAME | TYPE |  |  |
| :--- | :--- | :--- | :--- |
| No Protected Areas Found |  |  |  |

Crown Reservations/Notations: 0 (Data Updated:May 2015)

```
M-RR-TTT-SS NAME
TYPE
```

[^1]
## APPENDIX E

Fish and Wildlife Internet Mapping Tool


## Legend

Stocked Waterbodies
FWMIS River and Stream Label
FWMIS Lake Label
Other Waterbody Label
FWMIS River and Stream
River or Stream
Indefinite Stream
Canal
. Aqueduct
Representational Flow
FWMIS Major Waterbody
Perennial LakeIntermittent Lak
Diver
Other Waterbody
Perennial Oxbow
-. Intermittent Oxbow
(7) Wetland

國 Lagoo
Reservoir
Dugout
Provincial Sanctuary Corridor Wildlife
Provincial Sanctuary Game Bird
Provincial Sanctuary Restricted Area
Provincial Sanctuary Seasonal
Paved Road Label (20K)
Paved Road (20K)

- Primary Divided
- Primary Undivided 4L
- Primary Undivided 2 L

Primary Undivided 1 L

- Interchange Ramp
- Secondary Divided
- Secondary Undivided 4L
- Secondary Undivided 2L

Secondary Undivided 1L

## Gravel Road Label (20K)

Gravel Road (20K)

- Primary Undivided 2 L
- Primary Undivided 1L
- Secondary Undivided 2L
- Secondary Undivided 1L

Lake/River (20K)
Lake or RiverReservoirIcefieldMajor Canal
Oxbow
Quarry
Dugout
Intermittent Lake (20K)
Intermittent Lake
Intermittent Oxbow
Sandbar / Wetland / Lagoon
Sandbar
(2) Wetland

Lagoon
Stream (20K)
Stream

- Canal
- Oxbow

Ditch
Intermittent Stream / Aqueduct / Spillway
Recurring Stream
Indefinite Stream
Arbitrary Flow
Recurring Oxbow
Aqueduct
Spillway
ATS Township Index
ATS Section with Road Allowance
ATS Quarter Section with Road Allowance
ATS Legal SubDivision with Road AllowanceTown
Urban Service Area
City
Exploration Restricted Area Label
Exploration Restricted Area

- Active

CancelledEnvironment and Sustainable Resource Development Operations

## APPENDIX F



## APPENDIX G



## Lac St Anne

```
Projection
    Web Mercator (Auxillary Sphere)
    Legend
    - Groundwater Drilling Report
    Datum
        WGS 84
    Date
    9/9/2015 11:10:08 AM http://groundwater.alberta.ca/WaterWells/d/
    Information as depicted is subject to change, therefore the Government of Alberta assumes no responsibility for discrepancies at time of use
    @ 2009 Government of Alberta
    @ Government of Alberta | Copyright Government of Alberta | Esri, HERE,DeLorme, NGA, USGS
```

Please click the water Well ID to generate the Water Well Drilling Report. Groundwater Wells

| Well ID | LSD | SEC | TWP | RGE | M | DRILLING COMPANY | DATE COMPLETED | DEPTH <br> (ft) | TYPE OF WORK | USE | CHM | LT | PT | WELL OWNER | STATIC LEVEL <br> (ft) | TEST <br> RATE (igpm) | SC_DIAM <br> (ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 362774 | SE | 01 | 054 | 02 | 5 | GERALD MCGINN DRILLING LTD. | 1992-01-23 | 105.00 | New Well | Domestic \& Stock |  | 8 |  | PARKER, CHARLES | 40.00 | 7.00 | 6.00 |
| 364955 | SE | 01 | 054 | 02 | 5 | UNKNOWN DRILLER |  | 110.00 | Chemistry | Domestic |  |  |  | DUBE, LEO |  |  | 0.00 |
| 444095 | SE | 01 | 054 | 02 | 5 | GERALD MCGINN DRILLING LTD. | 1974-10-23 | 94.00 | New Well | Domestic |  | 11 |  | PARKER, CHARLIE | 38.00 | 7.00 | 4.50 |
| 444096 | SE | 01 | 054 | 02 | 5 | GERALD MCGINN DRILLING LTD. | 1971-05-08 | 106.00 | New Well | Domestic |  | 12 |  | PARKER, CHARLES | 68.00 | 3.00 | 4.50 |
| 444097 | SE | 01 | 054 | 02 | 5 | GERALD MCGINN DRILLING LTD. | 1983-09-30 | 94.00 | New Well | Domestic \& Stock |  | 7 |  | PARKER, CHARLES | 40.00 | 3.50 | 5.56 |
| 495878 | SE | 01 | 054 | 02 | 5 | RODCO DRILLING | 1999-09-14 | 108.00 | New Well | Domestic |  | 11 | 7 | DUBE, LEO | 50.80 | 9.00 | 6.00 |

# "AGREGATE \& GEOTECHNICAL ASSESSMENT" 

S.E. 1-54-2-5

PREPARED BY

TWERDOFF \& ASSOCIATE

## APPENDIX ‘B’

June 2014

# Aggregate Characterization Assessment 

Prepared for:<br>Wes Erickson<br>On<br>SE 1-54-2-W5M<br>Prepared by:<br>TWERDOFF \& ASSOCIATES INC.<br>Dennis Twerdoff, M.Sc., P.Geol., P.Ag.<br>Senior Project Manager

June 2014

## Table of CONTENTS

1 INTRODUCTION ..... 1
1.1 Objective ..... 1
1.2 SCOPE ..... 1
1.3 GEOLOGY ..... 2
2 METHODS. ..... 3
2.1 DRILL PROGRAM ..... 3
3 RESULTS. ..... 4
3.12014 DRILL PROGRAM ..... 4
4 DISCUSSIONS AND CONCLUSIONS ..... 5
4 CLOSURE ..... 7
5 REFERENCES ..... 8

## List of Appendices

APPENDIX A
APPENDIX B

Figures
Testhole Logs

## 1 Introduction

Twerdoff \& Associates (Twerdoff) was retained by Wes Erickson determine the location and extent of potential aggregate reserves within SE 1-54-2-W5M (Figure 1). This land is surrounded on 3 sides by aggregate operations. An old pit is present in the far northwest corner of the property.

The property is located approximately 5 miles southeast of the town of Onoway, Alberta. The land is currently agricultural with areas being developed into a lay-down yard. There are also 2 subdivision lots with 3 residences on the south and southeast portions of the $1 / 4$ section.

### 1.1 Objective

The objective of the Aggregate Characterization Assessment was to delineate potential aggregate reserves, to assess the general aggregate quality and provide a general estimate of potential reserves.

### 1.2 Scope

The scope of work for this Assessment included the following principle tasks:

- Review Alberta Geological Survey (ASG) sand and gravel information for the area,
- Design a rough exploration plan,
- Attend auger drilling and log test hole lithology,
- Prepare an Aggregate Characterization Assessment report to summarize findings.


### 1.3 Geology

A search of surficial geology on the property was conducted. The AGS identified areas within the SE 1-54-2-W5M along with surrounding land as having Dirty Sandy Gravel with an average of 5 m of aggregate (AGS, 2014) (Figure 2).

Quaternary geology consists Stagnation Moraine. The Stagnation Moraine is characterized by till of uneven thickness, local water sorted material, it is up to 30 m thick, and is undulating hummocky reflecting variations in till thickness. Topography is undulating with local relief of up to 3 m (Shetson, 1990).

A Water Well search was conducted on the Alberta Environment Water Well Information Database website for wells located within SE 1-54-2-W5M. Five (5) water well records were returned. Well lithology on records that recorded lithology did not show any layers of sand and gravel.

Bedrock geology consists of the Horseshoe Canyon Formation of Mesozoic age (Abadata, 2013).

## 2 Methods

### 2.1 Drill Program

Mobile Augers from Edmonton Alberta conducted auger drill testing from May 5 to May 9, 2014. A $6^{\prime \prime}$ auger was used to drill boreholes between 27 and 74 feet below ground level. A total of 20 boreholes were advanced throughout the property.

Subsurface lithology was recorded by Dennis Twerdoff, P.Geol. P.Ag.

The testhole number, location and general lithology for each test hole is illustrated in Figure 3, Appendix A.

A detailed description of each test hole lithology is outlined in Appendix B.

## 3 Results

### 3.1 2014 Drill Program

In general, testholes encountered a silty sand on the surface followed by a brown clay layer. In northwest portions of the property, a seam of sand and gravel was observed under the brown clay which seemed to pinch out into a gravelly clay south and east where topography dropped in elevation. Below this, another layer of clay was observed with a few holes drilled to bedrock.

The sand and gravel seam was encountered in the northwest portion of the property. The potentially mineable area is outlined in Figure 3, Appendix A.

The deposit was relatively sandy with an estimated $40 \%$ gravel content and $60 \%$ sand content. The sand and gravel seam along the edges of the deposit were mixed with clay while the majority of the seam was quite clean. Within the heart of the deposit, the sand and gravel seam was covered with 25-37 feet of overburden and the seam was approximately $10-20$ feet thick. The approximate overburden to gravel ratio was 3:1.

## 4 Discussions and Conclusions

There are active sand and gravel pits north, west and east of the property. Very old gravel pits are located approximately 2 miles south. The gravel pit west and northwest of the property are thought to be operated by Lafarge Canada. The gravel pit immediately north of the property is now operated by The Fath Group of Edmonton, AB (pers. comm. with pit operator).

The Alberta Geological survey identified this property as possessing sand and gravel. Water well logs on the SE 1-54-2-W5M did not reveal any sand and gravel layers within reported logs. There is an old excavation approximately 1.4 ha in size in the northwest corner of the property that appears to be largely depleted.

Test hole results indicated that sand and gravel was present in the northwest portion of the property. Typically, when sand and gravel is mixed with clay, as in the outer fringes of this deposit, reserves are not economic because it is difficult to separate the clay from the gravel using conventional processing.

The sand and gravel seam was present on the north side of the steep valley and creek that intersects the northern portion of the property. Typically, a 10 m buffer from the valley break would be required if mining operations encroached on the steep valley and creek.

The area where the sand and gravel seam was present was approximately 18 ha in size, less 1.4 ha that have been previously excavated and another approximately 4 ha encompassed by a steep valley and creek.

It is estimated that there is approximately 12.6 ha of area that is potentially mineable. It is estimated that there are approximately $400,000 \mathrm{~m}^{3}$ of sandy gravel reserves within the mineable area. There is an approximate $3: 1$ overburden to aggregate ratio.

Since Wes Erickson is not in the sand and gravel business, it is recommended that Mr. Erickson contact Lafarge Canada and The Fath Group to see if they would be interested in mining the area under a lease agreement. This should be done fairly soon as both pits operated by both operators appear to be nearly depleted and they may commence with reclamation soon. Getting an operator
in the future may be difficult because the area is relatively small and there are large amounts of overburden to move to access the gravel seam.

## 4 Closure

This report, entitled Aggregate Characterization Assessment, prepared for Wes Erickson.; prepared by Twerdoff \& Associates Inc.; June 2014, was produced by the following individual(s):


APEGA Permit to Practice
Permit No. 12187

This report was prepared by Twerdoff \& Associates Inc.. ("Twerdoff"), for Wes Erickson.. The information in it reflects the opinion of Twerdoff in consideration of the information available to it at the time of writing. This report is not to be used by anyone other Wes Erickson. without the prior written consent of Twerdoff. Whatever uses any third party makes of this report, or any reliance on or decision made based on it, are the responsibility of those third parties. Twerdoff accepts no liability for damages, if any, suffered by any third party as a result of decisions made or actions based on this report.

## 5 References

1. Abadata, 2014. Abacus Datagraphics. http://www.abacusdatagraphics.com/
2. Shetson, 1990. Quaternary Geology. Central Alberta. Alberta Research Council. http://www.ags.gov.ab.ca/publications/MAP/PDF/MAP 213.PDF
3. http://groundwater.alberta.ca/WaterWells/d/

## Appendix A

Figures




## Appendix B

## Test hole logs

TWERDOFF \& ASSOCLATES Inc.
Environment Geoscience

| Test Hole \#1 | Description | UTM Zone 11 |
| :---: | :---: | :---: |
| Depth (ft) |  | $5946400 \mathrm{~N}, 687750 \mathrm{E}$ |
| $0-10$ | Sand and silt |  |
| $10-15$ | Clay w/ small amount of gravel |  |
| $15-26$ | Dark brown clay |  |
| $26-37$ | Clay w/ small amount of gravel |  |
| $37-39$ | Sand w/ minor gravel |  |
| $39-42$ | Sandy gravel |  |
| $42-44$ | Sand |  |
| $44-50$ | Sandy gravel |  |
| $50-52$ | Gravel |  |
| $52-57$ | Clay |  |
| $57-68$ | Clay |  |
| $68-74$ | Hard clay |  |
| 74 | Bedrock |  |


| Test Hole\#2 | Description | UTM Zone 11 |
| :---: | :---: | :---: |
| Depth $(\mathrm{ft})$ |  | $5946400 \mathrm{~N}, 687950 \mathrm{E}$ |
| $0-5$ | Sandy clay |  |
| $5-6$ | Gravel lense |  |
| $6-12$ | Clay w/ minor gravel |  |
| $12-28$ | Clay |  |
| $28-37$ | Clay gravel |  |
| $37-39$ | Gravel |  |
| $39-42$ | Sandy gravel |  |
| $42-52$ | Clay |  |
| $52-62$ | Clay/sand |  |


| Test Hole \#3 | Description | UTM Zone 11 |
| :---: | :---: | :---: |
| Depth $(\mathrm{ft})$ |  | $5946400 \mathrm{~N}, 688150 \mathrm{E}$ |
| $0-4$ | Sand silt |  |
| $4-5$ | Sand/gravel |  |
| $5-8$ | Sand |  |
| $8-37$ | Clay |  |
| $37-52$ | Hard clay/sand |  |
| 52 | Bedrock |  |


| Test Hole \#4 | Description | UTM Zone 11 |
| :---: | :---: | :---: |
|  |  |  |
| 0-5.5 | Sand silty |  |
| $558$ | Gravelse3m, 2 \% 2 \% |  |
| 5.8-37 | Clay |  |
| $39 a 1$ | dolays |  |


| Test Hole \#5 | Description | UTMA Zone 11 |
| :---: | :---: | :---: |
|  |  |  |
| 0-3 | Sand silty |  |
|  |  |  |
| 8-25 | Brown clay/dark brown clay |  |
| $25-34$ |  |  |
| 34-37 | Clay |  |
| $37.38$ | Geavel |  |
| 38-42 | Clay/sit |  |
| $\stackrel{\mu}{n}$ |  |  |


| Test Hote $\ddagger 6$ | Description | LTM Zone 11 |
| :---: | :---: | :---: |
|  |  |  |
| 0-12 | Sandy gravel |  |
| $12-17$ <br> sandyctas |  |  |
| 17-27 | Clay/sitt |  |


| Test Hale \#\% | Description | UTM Zone 11 |
| :---: | :---: | :---: |
|  |  |  |
| 0-5 | Clay tila |  |
| 2-5 - $\quad$ Sand (nedimotourse Eraned) |  |  |
| $8-12$ | Sondy clay |  |
|  |  |  |
| 28 Clay w/ minor gravel |  |  |
| 2827 - Sadow/minoravel |  |  |
| 37-42 Clay w/ sand and gravel |  |  |
| $424 \%$, - $\quad$, $\quad$ ard sandy tay |  |  |
| 47-52 | Silty clay |  |


| Test Hole \#8 | Description | UTM Zone 11 |
| :---: | :---: | :---: |
| \%0¢th (f) \% \% \% \% |  |  |
| -25 Medium grained sand |  |  |
| $2525$ <br> Clay. |  |  |
| 35 Dark grey clay |  |  |
| 357. \% C Cay wravel |  |  |
| 37-39 Gravel* |  |  |
|  |  |  |
| 31-45 | Gravel* |  |
| 454\% , Eloww sondand bavet |  |  |
| 47-52 Clay w/minor sand and gravel |  |  |
| 52.5 <br> Lightbouno had sadyedzy |  |  |



| Test Hole \#10 | Destription | UTM Zone 11 |
| :---: | :---: | :---: |
|  |  |  |
| 0-2 | Sand silty |  |
| $21$ | Bround clav, \% , \%, \%, \% |  |
| 15-34 | Dark grey clay |  |
| -3437 | rownclay |  |


| Test Hole \#11 | Description | UTM Zone 11 |
| :---: | :---: | :---: |
|  |  |  |
| 0-2 | Sand sity |  |
| 418 | Brownclay, \%, \%- , |  |
| 18-29 | Dark grey clay |  |
| $29.4 \%$ | diy platy bac |  |


| Test Hole ${ }^{\text {\# }} 12$ | Description | UTM Zone 11 |
| :---: | :---: | :---: |
|  |  |  |
| . $0-3$ |  |  |
| 24-37 | Dark grev clay |  |


| Test Hole \#13 | Description | UTM Zone 11 |
| :---: | :---: | :---: |
|  |  |  |
| $0-2$ | Sand slizy |  |
| $217$ | Brownctay man |  |
| 17-42 | Dark grey clay |  |


| Test Hole \#1.4 | Description | UTM Zone 11 |
| :---: | :---: | :---: |
|  |  |  |
| 0.1 .5 | Sand slity |  |
| Bredriclay |  |  |
| 25-33 | Dark grey clay |  |


| Test Hoie ${ }^{\text {d }} 15$ | Description | UTM Zone 11 |
| :---: | :---: | :---: |
|  |  |  |
| --6 | Silty sand |  |
| Cavenad |  |  |
| 12-25 | Brown clay |  |
| $2530$ | akkirece | Wh |
| 30-33 | Gravel* |  |
| $33,40$ | Brown cley |  |
| 40-46 | t brown sand |  |




| Test Hole \#19 | Description | UTM Zone 11 |
| :---: | :---: | :---: |
|  |  |  |
| 0-2 | Silty sand |  |
| $28$ |  |  |
| 8-15 | Brown clay |  |
| $1532$ | Coy W/ mio |  |
| 32-39 | Sandy gravel |  |
|  | brownsand |  |


| Test Hole \#20 | Description | UTM Zone 11 |
| :---: | :---: | :---: |
|  |  |  |
| 0-2 | Silty sand |  |
| $216$ | Brownctay |  |
| 16-25 | Dark grey clay |  |
| $25-36$ | coarsenw | \% |
| 36 | Boulder |  |

# "TRAFFIC IMPACT ASSESSMENT" 

S.E. 1-54-2-5

PREPARED BY

D \& A PAULICHUK CONSULTANTS LTD.

## APPENDIX 'C’

## TRAFFIC IMPACT ASSESSMENT

## LAUNCH VENTURES RESIDENTIAL DEVELOPMENT SE 1-54-2-W5M Lac Ste. Anne County <br> Twp. Rd. 540, Hwy. 43, Rge. Rd. 20, Hwy. 16:14


$\mathrm{D}_{\&} \mathrm{~A}_{\text {consulting ltd. }}^{\text {PAULICHUK }}$

LOCATION PLAN L-1


## LOCATION PLAN L-2



## LOCATION PLAN L-3



LOCATION PLAN L-4


## DEVELOPMENT PLAN D-1



## TABLE OF CONTENTS

LOCATION PLAN L-1 ..... 2
LOCATION PLAN L-2 ..... 2
LOCATION PLAN L-3. .....  3
LOCATION PLAN L-4 ..... 3
DEVELOPMENT PLAN D-1 ..... 4

1. INTRODUCTION ..... 8
2. DEVELOPMENT DETAILS ..... 9
2.1 Development Site ..... 9
2.2 Development Trip Generation ..... 10
3. EXISTING CONDITIONS - Local Roads ..... 13
3.1 Development Routes of Travel ..... 13
3.2 Range Road 20 ..... 13
3.2.1 Site Observations. ..... 18
3.3 Township Road 540 (Heatherdown Road) ..... 23
3.3.1 Site Observations. ..... 27
4. EXISTING CONDITIONS - Highways ..... 28
4.1 Existing Intersection Treatment - Hwy. 16:14 \& RR 20 ..... 28
4.2 Existing Intersection Treatment - Hwy. 43:22 \& Twp. Rd. 540. ..... 30
4.3 Available Traffic Data ..... 32
4.4 Traffic Counts ..... 35
4.5 Traffic Growth - Hwy. 16:14 ..... 36
4.6 Traffic Growth - Hwy. 43:22 ..... 36
4.7 Traffic Growth - Rge. Rd. 20 \& Twp. Rd. 540 ..... 37
4.8 Back Ground Traffic Turning Movement Diagrams ..... 37
5. ANALYSIS - Development Access \& Rge. Rd. 20 ..... 40
5.1 Initial Determination based on Traffic Volume Warrant Chart ..... 41
5.2 Design Speed ..... 42
5.3 Detailed Analysis ..... 42
5.4 Intersectional Sight Distance ..... 44
6. ANALYSIS - Hwy. 16:14 \& Rge. Rd. 20 ..... 46
6.1 Design Speed ..... 46
6.2 Intersecting Road Classification ..... 46
6.3 Detailed Analysis ..... 47
6.3.1 Right Turn Lane ..... 47
6.3.2 Left Turn Lane. ..... 47
6.4 Intersectional Sight Distance ..... 49
6.5 Highway Capacity Analysis. ..... 52
6.6 Future Freeway Planning ..... 56
7. ANALYSIS - Hwy. 43:22 \& Twp. Rd. 540 ..... 57
7.1 Design Speed ..... 57
7.2 Intersecting Road Classification ..... 57
7.3 Detailed Analysis ..... 58
7.3.1 Right Turn Lane ..... 58
7.3.2 Left Turn Lane ..... 58
7.4 Intersectional Sight Distance ..... 60
7.5 Highway Capacity Analysis. ..... 61
7.6 Future Freeway Planning ..... 64
8. ILLUMINATION \& SIGNALIZATION WARRANTS ..... 65
9. CONCLUSIONS \& RECOMMENDATIONS ..... 66
9.1 Conclusions \& Recommendations ..... 66
9.2 Closure ..... 69

APPENDIX A - Alberta Highways Traffic Volume History<br>Traffic Data - Turning Movement Diagrams<br>Traffic Count Data

APPENDIX B - Development Access \& Rge. Rd. 20
Turning Movement Diagrams

APPENDIX C - Hwy. 16:14 \& Rge. Rd. 20
TURNing Movement Diagrams
Synchro Analysis

APPENDIX D - Hwy. 43:22 \& Twp. Rd. 540
Turning Movement Diagrams
Synchro Analysis
Illumination Warrant


## 1. INTRODUCTION

This report is a Traffic Impact Assessment (TIA) report for a Residential development located in the SE quarter section of Section 1, Township 54, Range 2, West of 5th Meridian. See Location Plans before this report. The residential subdivision is proposed to contain 13 lots.

This report has been prepared to determine the impact of the proposed development on traffic using Highway 43:22 \& Township Road 540 (Heatherdown Road) and Hwy. 16:14 \& Range Road 20 which are the anticipated access routes to the proposed development. The scope of work in this case, is to assess the existing roadways and any required intersection treatment as a result of the increased traffic volumes and turning movements attributed to the development. This document will address the following:

- Retrieve any available traffic volume data, including conducting traffic counts.
- Determine present traffic volumes for 2016.
- Determine projected traffic volumes to 2016, 2021, 2026 and 2036.
- Determine traffic volumes generated from the development.
- Determine combined traffic volumes for 2016, 2021, 2026 and 2036.
- Complete intersectional analysis of the roadways if required.
- Identifies geometric deficiencies based on current 3R/4R guidelines.
- Review sight distance requirements.
- Complete illumination and signalization warrant analysis, if required.
- Provides a preliminary engineering cost estimate for recommended improvements.

This report is based on information provided by the developers of the 13 lots in SE 1 -54-2-W5M, site observations from Mr. Darcy Paulichuk, P. Eng., traffic volume data from Alberta Transportation, intersectional analysis procedures and standards documented in Alberta Infrastructure and Transportation's "Highway Geometric Design Guide", 1999, and Alberta Infrastructure and Transportation's "Traffic Impact Assessment Guideline", 2005.

This report has been prepared for developers of the 13 lots in SE 1-54-2-W5M for the purposes of gaining approval from municipal and provincial governments for the development of this site.

## 2. DEVELOPMENT DETAILS

### 2.1 Development Site

The proposed development is for a 13 lot Residential rural subdivision within SE 1-54-2-W5M. The site will utilize Range Road 20 for access to the east and then south on Rge. Rd. 20 to the south to Hwy 16 or west on Twp. Rd. 540 (Heatherdown Road) to Hwy. 43. See below:

FIGURE 2.1
Proposed Development Site Plan


## SE 1-54-2-W5M

Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

### 2.2 Development Trip Generation

The proposed development will comprise of 13 lots.
For the Residential development lots, ITE 210 - "Single-Family Detached Housing" Land Use from the Institute of Transportation Engineers (ITE) Trip Generation Manual, $9^{\text {th }}$ Edition, can used. The following data is available from the manual:

| Time Period | Trips per Lot |
| :--- | :---: |
| Daily | 9.52 |
| Peak Hour of Adjacent Street Traffic <br> One Hour Between 7 and 9 a.m. | 0.75 |
| Peak Hour of Adjacent Street Traffic <br> One Hour Between 4 and 6 p.m. | 1.00 |

Single-Family Detached Housing (210)

Average Vehicle Trip Ends vs: Dwelling Units
Ona: Weakoby

| Number of Sludes: 755 <br> Avg. Number of Dweling Unis: 198 <br> Disctosisi Dostroution: $\$ 0$ he entoring $50 \%$ asiting |  |  |
| :---: | :---: | :---: |
| Trip Generation per Dwvelling Unit |  |  |
| Aumage Matn | Rarge of Rases | Standad Oevatos |
| PS | 471 - 71.85 | 1220 |

Data Plat and Equation


## IRAFFIC IMPACT ASSESSMENT

Therefore, the estimated trip generation of the proposed development with 13 new lots is as follows:

| Daily Traffic: | 13 lots $\times 9.52$ trips/day | $=$ | 124 trips per day |
| :--- | :--- | :--- | :--- |
| AM Peak Hour: | 13 lots $\times 0.75$ trips/hour | $=$ | 10 trips per hour |
| PM Peak Hour: | 13 lots $\times 1.00$ trips/hour | $=$ | 13 trips per hour |

These values above represent the traffic generation once all 13 lots are built, homes are constructed and then occupied. Since this number is fairly low, it is estimated that 3 lots will be occupied in 2016 and the remaining 10 lots will be occupied in 2021 (Year 5). The trip distribution is anticipated to be $40 \%$ to the west on Twp. Rd. 540 to Hwy. 43 and $60 \%$ to the south on Range Road 20 to Hwy. 16, with $25 \%$ entering/ $75 \%$ exiting in the AM Peak and $63 \%$ entering $/ 37 \%$ exiting in the PM Peak. The distribution of traffic is projected as follows:



## 3. EXISTING CONDITIONS - LOcAL Roads

The focus of this traffic impact assessment is mainly on Twp. Rd. 540 to Hwy. 43 and Rge. Rd. 20 to Hwy. 16 in which it is anticipated that the development will use.

### 3.1 Development Routes of Travel

Range 20 traverses between Highway 16 to the south and Highway 633 to the north. Twp. Rd. 540 traverses between Highway 43 and Hwy. 779 to the east.


### 3.2 Range Road 20

Range Road 20 can be classified as a minor to major collector roadway. The existing width is approximately 8.2 m from Hwy. 16 to Twp. Rd. 541 , and 9.2 m wide from Twp. Rd. 541 to Hwy. 633 (west leg)/Twp. Rd. 542. Range Road 20 is surfaced with a full surfacing structure comprised of granular base course and asphalt concrete pavement. The pavement thickness appears to be $100-150 \mathrm{~mm}$ in thickness. The roadway also contains roadway paint lines for the centreline and shoulders. Range Road 20 provides collection of traffic from the local area and provides connections
to the provincial highway system. It also is a route constructed for the haul of natural resources from the area such as aggregate.

The section of Range Road 20 from Hwy. 16 to Hwy. 633 is posted at $80 \mathrm{~km} / \mathrm{hr}$. This usually implies a $90 \mathrm{~km} / \mathrm{hr}$. design speed.

The section of Range Road 20 from Hwy. 16 to Twp. Rd. 541 was constructed several years ago and likely to a RLU-208 standard ( 8 m paved width standard) as shown below as Lac Ste. Anne County Standard Drawing G-07:


This standard is dated March 2008, however this section of Range Road 20 was constructed several years before this, likely with a lesser standard. This can be recognized with physical aspects such as a 3:1 fill slope in some locations.

This present County standard would be most representative for this section.

Below is an air photo view and ground photo of this section of Range Road 20.


The following table is from Lac Ste．Anne County＇s General Municipal Servicing Standards and represents the roadway design parameters for the various road standards．The two standards that are being used as comparable standards for Range Road 20 are noted below．

|  | Designation | Surface | AADT＊ | Truck Traffic | Min． ROW $^{* *}$ （m） | Preferred ROW＂ （m） | Design Speed （ $\mathrm{km} / \mathrm{h}$ ） | Posted Speed （ $\mathrm{k}=\mathrm{v} / \mathrm{h}$ ） | $\begin{gathered} \text { ssd } \\ (\mathrm{m}) \end{gathered}$ | Crest <br> k （m） | Sagk （m） | Min． <br> Horiz． <br> Radius <br> （m） |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | RLU－207G | Gravel | $<25$ | None | 20 | 30 | 40 | 30 | 65 | 7 | 11 |  |
|  | RLU－208G（a） | Gravel | ＜ 100 | Minimal | 20 | 30 | 60 | 50 | 85 | 15 | 20 | 90 |
|  | RLU－208G（b） | Gravel | $\leqslant 200$ | Minimal | 20 | 30 | 60 | 50 | 85 | 15 | 20 | 90 |
|  | RLU－209G | Qravel | $>200$ |  | 20 | 40 | 60 | 50 | 85 | 15 | 20 | 90 |
|  | RLU－2106 | Gravel | $>200$ | Significam | 20 | 40 | 70 | 60 | 140 | 35 | 30 | 190 |
|  | RLU－208（a） | Cold <br> Max | ＜ 200 | Minimal | 20 | 40 | 60 | 50 | 85 | 15 | 20 | 90 |
|  | RLU－208（b） | ACP | ＜ 200 |  | 20 | 40 | 60 | 50 | 85 | 15 | 20 | 90 |
| 要 | RLU－209（a） | Cold <br> Mix | ＜ 500 | Minimal | 20 | 40 | 60 | 50 | 85 | 15 | 20 | 90 |
| 古 | RLU－209（b） | ACP | ＜ 500 |  | 40 | 40 | 60 | 50 | 85 | 15 | 20 | 90 |
| $\frac{8}{2}$ | RLU－210 | ACP | ＊2000 |  | 40 | 40 | 70 | 60 | 140 | 35 | 30 | 190 |
| 家 | RLU－211 | ACP | ＞2000 | Significant | 40 | 40 | 70 | 60 | 140 | 35 | 30 | 190 |
| $\frac{\mathrm{A}}{5}$ | ULU－209 | ACP | $\leqslant 2000$ |  | 20 | 30 | 60 | 50 | 85 | 15 | 20 | 90 |
| $8$ | ULU－211．5 | ACP | $>2000$ | Significant | 30 | 30 | 70 | 60 | 140 | 35 | 30 | 190 |

For both road standards，the posted and design speeds appear to be lower than the existing posted speed on Range Road 20．The lower posted speed on the standards above mostly are reflected in the vertical and horizontal alignment．

Using the Transportation Association of Canada standards shown below，the following minimum vertical and horizontal parameters can be used for an $80 \mathrm{~km} / \mathrm{hr}$ ． posted speed（design speed $=90 \mathrm{~km} / \mathrm{hr}$ ．）：

| Minimum Horizontal Radius： | 340 m |
| :--- | :---: |
| Maximum Gradient： | $5-7 \%$ |
| Minimum K value for Crest Curves： | 53 |
| Minimum K value for Sag Curves： | 40 |

## LAUNCH VENTURES RESIDENTIAL

## SE 1－54－2－W5M

Lac Ste．Anne County TR 540，Hwy．43：22，RR 20，Hwy．16：14

eenircis ar considsarations influencing the forionial algoment ano sacemliovation

Uureower，now speas intan ercions，avinos ary accuatomed do a gantorievti of d ncomfors foction factors eisicg tien beswrive porelsmition Whes ornessecericcuation es at are porriseble．
 actora as defnech ty Trite 2.12 .2 ans aspor in cacuioting mikomit radil

To siow lor the fact sat in low speed uman Smbas yanus aniss for maknum perninsible auperelevation ruay exist the minimum raduin
 dispertevition
7eie 2.124 provides rousted dedign wainstor mirimurs rasil for low upaed urban deaiga
$30 \mathrm{kmh} 1060 \mathrm{um} / \mathrm{h}$ romraty raprssantalive et
 uperslevation，reverse crown 10.02 mim mpersievation）and mexirum s．peselevalion rotoc of 0.04 end 0.05 min．Table 2.12 .42 .25 pervices a sunumary of the nanimm ack for a arveo ditioh desian sooede， 70 kmhn to 100 ken Ny associajed with mavimum superesovation
 The samp as the high jpeoc sran wavio
 Curcts Design Donain
Overview
Thare ave a number of mothoos af distributing and I orer a manger of curves facer than the Poge2．12．8

Aligrment and Lane Confguration
in calculang $\mathbb{K}$ valuae for varieut night
 nod ing heighte ol ohject as as ousined flewing and docunsed is more getar in Cruptar 1.2.
－For osomoing aight distance the most convinon ofioct e wonot has to siop loc ） neition of tail lyt is used．The lecibseted minimum is if． 38 m ond is adopted to irsiga Oher hutpias of objects can be ated If nocesasiry．
－Fer declasion aight dislance the mote pormmon ne ant of obiect is 0.15 m menest merkings．ken not incsirem

For pasuing vight aldarea the halght of object is 1.30 m ，whikh represeris tie Norgtef the oppouing whictor
Crest Vertical Curves：Design Domain Quanstative Aud

Basod on the above most commenly used felgita sol ctjpet and on uight tlatascer hom tetien 1.2 .53 and 1.2 .55 ． $\mathbf{D e n} K$ valuns for
topping eight distance ate provided in able 2.13 .8 and for passing Night dissaricat k values are providad in Tabie 2．1．3．3 the gecisich aight ciatance $K$ valuss are no included bacguse the vertical carvature docendinyenwher te divir hae to met

The calcuinind K velums ase besedón Ete largy or cunco miceading the sigrtatatace and wre， an be ined witroul sigificat encr when the enget of curve io lose than the sight datarce ppprocabie ditsronces oocur ony whers A E Nast and lite cr no arobicind cost is liviter notbining longer vert cal curvas．

Onuminised roets rossmpmig siguidiance it used to determing when na－passing
 cossble bof nos atraing sight diatarce generraily atequate fot ssfe passim manoeures
 Fohl diatanix，al dech desion spesec．Fassip
 of oncaminy veticies．

Tabie 21．3．2 K Factore to Provide Stopping sight Distance on Crest Vertical Curves

| $\begin{gathered} \text { Design spees } \\ \operatorname{lnghi} \end{gathered}$ | Aetumed Operatieg speed （ $\mathrm{mm} / \mathrm{h}$ ） | stopgieg sight Distanger（in） | Mals of VerficasCunvaturs p6 |  |
| :---: | :---: | :---: | :---: | :---: |
| 35 | 30 | 20.6 | 15 | 2 |
| 45 | 40 | 44.4 | 3.7 | 4 |
| 50 | 42． 60 | 57.42 .18 | 6．1．1．2 | 6．7 |
| E1 | $550-60$ | 74．3－84．6 | 102.133 | 10－73 |
| 0 | $43 \cdot 10$ | 等．1－170．0 | 164．228 | 1423 |
| 80 | 70－20 | 1128－1324 | 236.96 .1 | 24．36 |
| m0 | 77－90 | ＊3129837 | 320．625 | 32．53 |
| 160 | 65． 100 | 152．0．2050 | 458.750 | 45.00 |
| 110 | 81－110 | 179．5－2684 | 38．0－12？ | 6）－110 |
| 120 | 68．120 | 2029．235．6 | 96．4．151／A | 73－150 |
| 130 | 1015－130 | 22703279 | $364+199.5$ | 95.200 |

Note：The above are meimuml vayos，ute hugher $K$ trccors wrerever posebie

P2902130

Alyrumt and Lane Conffyration
varym

Maximumarede Design Domen Qustitere Aids

Ahnougn tre reiaucnehip owtwen sesign speeds and maximum grode is reiative tifective，restionsdie guidas far meximum reje hove beeri devieioped
 n labie 2.13 .1 ．

Maximin Gass：Design Domion Applazon

1．The rarge of vaiveeshonm ia Tabie 21,3 for dosing vailes nety fopogacis siected gereral finoncial cepabily of the rois
authoty to lund Exe caplat wores For kover caganczion uthan roacs，iand lwe is an adations prssiteration and land use is local and viban uncivided coluccton
2．Tha values shaven may be adjeted to ant losir and aconomio conditoss，Masimum ofian a mefer co colicy，amt as a resit． varr trom junsecition to urisciostion． Normaby Jue kecal pulicy is establithed at a senior ergineerny and plenoesy hivel in any aveot，in adusiling these foures，
 ctensioder the mpand of such meflemailve roximurn grade yanes on satefy

Table 21．2．1 Maximum Gradier

| $\begin{aligned} & \text { Desige } \\ & \text { Bpeed omplo } \\ & \text { Tepostaphyy } \end{aligned}$ | 304050 |  |  | 60 |  | 70 |  | 80 |  | 30 |  | 100 |  | 110 |  | 320130 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | A |  | \＃ | F． | 4 | \＃ | M | A | u | H | 4 | \＃ | 析 | － | $u$ | R | m |
| Ful | $\frac{7}{7}$ |  | 11 | 7 | it | 6 | 9 | 6 | \％ | 5 | 7 | 5 | 7 | － | － | ． | $\cdots$ |
| ACU | $=$ |  | － | 6 | 80 | 6 | $\theta$ | S | 1 | 5 | 7 | 5 | 7 | － | $=$ | $\pm$ | － |
| ACD |  |  | － | $-$ | $\checkmark$ | 6 | 8 | 8 | E | 5 | 7 | 5 | $\pm$ | － | ， | － | － |
| Pais |  |  | ＊ | － | － | \％ | ＝ | 4 | 7 | 4 | 䓪 | 3 | 5 | 3 | 6 | 3 | 3 |
| Rad |  |  | 4 | － | － | $\square$ | ＊ | 4 | 7 | $\varepsilon$ | \＃ | 3 | 5 | 3 | 3 | 3 | 5 |
| FAFO |  |  | $+$ | － | ＋ | － | － | － | － | － | － | 0 | 3 | 3 | 6 | 3 | 5 |
| ULU－ Fivaidential ULi： Industria／ |  |  | 㭠 | － | － | － | ， | ＊ | ＊ | － | － | ， | ， | － | － | － | ＊ |
|  |  |  | 12 | － | ＊ | － | － | ＊ | － | － | － | $\rightarrow$ | － | － | － | ＊ | ${ }^{*}$ |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Commarcal UCU－ |  |  | 12 | 7 | 11 | 7 | 10 | － | ＊ | － | － | ， | － | ． | ． | ＋ | － |
| Pesiotentis |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| UCu－ kolutiak |  |  | 12 | E． | 11 | C | 3 | 0 | 0. | － | ＊ | ＋ | \％ | 4 | $\stackrel{1}{ }$ | \％ | ： |
| Commectal |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| UCD |  |  | 10 | 6 | 9 | 5 | E | B | 7 | \％ | － | － | － | $=$ | － | ＋ | $\%$ |
| WAL |  | 6 | 10 | 6. | 3 | 5 | 6 | 5 | 7 | ＊ | ， | $\bigcirc$ | ： | $\cdots$ | ＊ | $*$ | － |
| Und |  |  | \％ | 3 | $\square^{5}$ | \＄ | 6 | 3 | ${ }^{\text {® }}$ | 3 | \％ | 3 | 5 | － | \％ | $\stackrel{1}{2}$ | $\square$ |
| UED |  |  | ＋ | ， | － | ＋ | － | 3 | 6 | 4 | 5 | 4 | 5 | 4 | 5 | 3 | 3 |
| WFo |  |  | 8 | － | $\bigcirc$ | 3 | － | － | － | 4 | 5 | B | 5 | 3 | 5 | 3 | 5 |
| Notsie | Shoit ghadat tesa then 150 m in langut，ard one viy down graces may be $1 \%$ higher on uitian nouds and 2tis highar on low volurne rual raids． <br> A nitors bo roiling topograghy． <br> M refers to mountuinesa topograchy． |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Page 2．1．32 Sepumber 1999

Tery Geomerie Design Guide for Canadian Raada


Vehpe forsag curvervie hased on the contort Cthencon aw shemn in Tatien 2．1．3．4

Thesak umimitrkagomes amutatinuran obatons suchasunderjeoses where El a cheril recessary far property and acsems rosogna to thepattion ongral gaurd bienalios toras gron a Cisarin as posmibe Mhinum vatuen av mormaily oxcesed where leasioit，in
 mefunctions to the streer lykerg systerne Desidning aog vortical turvee aiona curved out feasble de to the interest far gries an mentitet artace oniragopmbike

## 2．1．3．4 <br> Sight Distance at

While not a fecuent dosign probiem，fhe slight ditance at unsorpasise ray se motrictod due the bothen of the overpass stucturn reaticioting the ine of eight．The oght deanset through a grache sepparation shesid be eqpal to or giralser than the drimum Mapping sight distance．

 an underpase clacmance of C．Case 4 is for a bight simanos croster than the langth of the whica curve（ $5 \times$ li）and case 2 is for a wight

Figure 2．1．3．3 Sight Distance at Underpass


The existing horizontal curvatures on Range Road 20 appear to be between 300 to 350 m presently, which is near the standard for an $80 \mathrm{~km} / \mathrm{hr}$. posted speed. These curves are within Parkland County. The horizontal alignment of Range Road 20 through Lac Ste. Anne County is straight and on tangent.

The existing vertical profile contains some crest and sag curves that appear to be within the $80 \mathrm{~km} / \mathrm{hr}$. posted speed standard within Parkland County. Range Road 20 is fairly flat within Lac Ste. Anne County.

All grades appear to be less than $8 \%$.

### 3.2.1 Site Observations

Site observations of Range Road 20 between September 10 to 14 are shown below:



## TRAFFIC IMPACT ASSESSMENT




km 6.8, Range Road 20:
Viewing east along Twp. Rd. 540.
Twp. Rd. 540 is gravelled and appears to have little use in this area. Likely local traffic only.

km 6.8, Range Road 20:
Viewing west along Twp. Rd. 540.
Twp. Rd. 540 is gravelled and appears to have little use in this area as well. Likely local traffic only.

## SUMMARY

Upon inspection, Range Road 20 appears to be consistent with a Rural Local Road County standard with widths from 8 to 9 m . The existing roadway exhibits cross sectional attributes near or within the standard that includes sideslopes, ditch widths and backslopes. The horizontal and vertical alignments also appear satisfactory and consistent with standard.

There are no concerns with the present geometric, sight distances and condition of Range Road 20.

### 3.3 Township Road 540 (Heatherdown Road)

Township Road 540 can be classified as a minor to major collector roadway. It presently collects traffic from the area and distributes it to other collectors or arterial roads such as highways. The existing width is approximately $7-9 \mathrm{~m}$ from Hwy. 43 to Hwy. 779. Range Road 20 is gravel surfaced between Hwy. 43 to RR 20. Township Road 540 will be an important internal local road route once the at-grade intersections close onto Hwy. 43 and Hwy. 16.

The section of Township Road 540 from Hwy. 43 to RR 20 is posted at $80 \mathrm{~km} / \mathrm{hr}$. This usually implies a $90 \mathrm{~km} / \mathrm{hr}$. design speed.

The section of Range Road 20 from Hwy. 16 to Twp. Rd. 541 was constructed several years ago and likely to a RLU-208a standard ( 8 m gravelled width standard) as shown below as Lac Ste. Anne County Standard Drawing G-02:


NOTL: Des stuNend is wroced ron
tow vouw lock poces beor



## IRAFFIC IMPACT ASSESSMENT

This standard is dated March 2008, however this section of Township Road 20 was constructed several years before this, likely with a lesser standard. This can be recognized with physical aspects such as a 3:1 fill slope in some locations. This present County standard would be most representative for this section. Below is an air photo view and ground photo of this section of Range Road 20.


The following table is from Lac Ste. Anne County's General Municipal Servicing Standards and represents the roadway design parameters for the various road standards. The two standards that are being used as comparable standards for Township Road 540 are noted below.

|  | Designation | Surface | AADT* | Truck Traffic | Min. ROW (m) | Preferred ROW" (m) | Design Speed ( $\mathrm{km} / \mathrm{h}$ ) | Posted Speed ( $\mathrm{km} / \mathrm{Vh}$ ) | $\begin{gathered} \text { sso } \\ (\mathrm{m}) \end{gathered}$ | Crest <br> k(m) | Sagk (m) | Min. <br> Horiz. <br> Radius <br> (m) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | RLU-207G | Gravel | $<25$ | None | 20 | 30 | 40 | 30 | 65 | 7 | 11 |  |
|  | RLU-208G(a) | Gravel | < 100 | Minimal | 20 | 30 | 60 | 50 | 85 | 15 | 20 | 90 |
|  | RLU-208G(b) | Gravel | $\leqslant 200$ | Minimal | 20 | 30 | 60 | 50 | 65 | 15 | 20 | 90 |
|  | RLU-209G | Oravel | $>200$ |  | 20 | 40 | 60 | 50 | 85 | 15 | 20 | 90 |
|  | RLU-210G | Oravel | $>200$ | Significamt | 20 | 40 | 70 | 60 | 140 | 35 | 30 | 190 |
|  | RLU-208(a) | Cold <br> Max | < 200 | Minimal | 20 | 40 | 60 | 50 | 85 | 15 | 20 | 90 |
|  | RLU-208(b) | ACP | < 200 |  | 20 | 40 | 60 | 50 | 85 | 15 | 20 | 90 |
|  | RLU-209(a) | Cold <br> Mix | < 500 | Minimal | 20 | 40 | 60 | 50 | 85 | 15 | 20 | 90 |
|  | RLU-209(b) | ACP | < 500 |  | 40 | 40 | 60 | 50 | 85 | 15 | 20 | 90 |
|  | RLU-210 | ACP | < 2000 |  | 40 | 40 | 70 | 60 | 140 | 35 | 30 | 190 |
|  | RLU-211 | ACP | >2000 | Significant | 40 | 40 | 70 | 60 | 140 | 35 | 30 | 190 |
|  | ULU-209 | ACP | $<2000$ |  | 20 | 30 | 60 | 50 | 85 | 15 | 20 | 90 |
|  | ULU-211.5 | ACP | $>2000$ | Significant | 30 | 30 | 70 | 60 | 140 | 35 | 30 | 190 |

For both road standards, the posted and design speeds appear to be lower than the existing posted speed on Range Road 20. The lower posted speed on the standards above mostly are reflected in the vertical and horizontal alignment.

Using the Transportation Association of Canada standards shown below, the following minimum vertical and horizontal parameters can be used for an $80 \mathrm{~km} / \mathrm{hr}$. posted speed (design speed $=90 \mathrm{~km} / \mathrm{hr}$.):

Minimum Horizontal Radius: 340m
Maximum Gradient:

$$
5-7 \%
$$

Minimum K value for Crest Curves:
Minimum K value for Sag Curves:

Angomener aroc Lane contauruion
7nsme
Table 2．1．23
Minimum Fladif for Limiting velues of e and 1 for Rural and High Spend Urtear Roadwaya＇

| Design Spany ［amin］ | (minim) | Dosion value for 1 | ＊ 1 | Minimum Ratiuy（m） ［eaiculated］ | Minimum Hadius for Desion［［m］ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 46 | 0.04 | 97 | 0.21 | $80^{\circ}$ | 60 |
| 50 | 0.04 | 018 | 0.20 | 98 | 100 |
| 60 | 0.04 | 0.15 | 0.19 | 1487 | 150 |
| 70 | 0.04 | 015 | 0.9 | 208 | 200 |
| 60 | 0.04 | 2.14 | 0.18 | 206 | 200 |
| 30 | 0.04 | a13 | $0 \cdot 1$ | 375 | 360 |
| 100 | 0.04 | 0.12 | 0.95 | 488 | 499 |
| 45 | 0.06 | $4 \%$ | 023 | 55 | 55 |
| 5 | 4，000 | 213 | 022 | 樶 | （6） |
| 180 | 0.06 | 0.15 | 021 | 135 | 196 |
| 70 | 0.06 | 0.15 | 021 | 184 | 190 |
| （0） | 6000 | 014 | 0.23 | 20\％ | 280 |
| 30 | 0.06 | 0.3 | 019 | 336 | 340 |
| 100 | 0.06 | 012 | 015 | 437 | 449 |
| 170 | 0.06 | 2.10 | 0.15 | 598 | 400 |
| 120 | 0.06 | 009 | 0.15 | 73 | 750 |
| 130 | 000 | 089 | 0.14 | 951 | 890 |
| 40 | 0.08 | 2.47 | 025 | 50 | 50 |
| 50 | 0.08 | 0.18 | 0.24 | H2 | \％ 0 |
| 80 | 0.08 | 0.15 | 023 | 123 | 120 |
| 0 | 0.09 | 915 | 923 | 188 | 170 |
| 90 | 0.09 | 014 | 0.23 | 238 | 230 |
| 0 | 0.00 | 013 | 021 | 304 | 900 |
| 100 | 0.00 | 2.12 | 020 | 394 | 300 |
| 110 | 1．09 | 210 | 0.90 | 523 | 539 |
| 120 | 004 | 309 | 087 | $66^{6}$ | 670 |
| 130 | 009 | 006 | 0.18 | 132 | ＊99 |

equircis ar considsarations influencing the ncrisonial algomentand sapenduvation
Moreover，n ow speasirtar cerclions，avires


 In trese caves，trein masmum taveral ficton
 edcuisting inikmurn ridic
To now for the fact sat in ion speed uman ans a y vaicus inves for makinumperriusible auperelevator may ewst me momum raduan dsperilevition
Treie 21．2．4 mavidas roukted devign waunstor ririmurs racil for bo upaed urban dowign

Fage2．1．28
Aligrment and Lane Confguration
$30 \mathrm{kmh} 2060 \mathrm{um} / \mathrm{h}$ rommaty raprespniallive of tefofl conditions．Mirmum rasilare staied trx nombal crown（ 0.02 min ，ar adyerbe suparelevation，noverse crown 10.02 mim ratec ol 0.04 end 0.05 mm ．Table $2.12 \mathrm{~A} \mathrm{z} / \mathrm{lo}$
 mreo dhigh design spoote， 70 kmh to 100 ker A，associaied with maselmum superesovation walkis of 0.04 mm and 0.06 mmm ．The velires ore the same as the ingh speod sran watise in Tabien 2．1．2．3

Curits：Design Domain
Overview
Thareace a number of mochoos af distributings and＇over a mange of coives facer than the

In balcyatung $K$ valuse for varieus night In calcu suncas，the felgh of aiver s wpe is 1.05 mp wid ine híght do dijpet is as ousined folowing and dscuabed is mors getar in Chaplar．
－For oforping sight distance the most cormon sofpct e vehide has to miop tor ） anneftion velicico aheod on the road，tw reigh of tai lift 3 used The regibeter trasion Oher hüptia of obiects can be ated if nocesasy．
－For decisian angin dialack the morn osermon no am of abiect is 0.15 m shougn ofer rongts，suct as zuro for pinemest makings．we not urcantron．
－For pasung sight mblarca the halght of object is 1.30 m ，which tepreserte the object is the m，which tepie

Crest Versical Curves：Design Demain Quanstativenas
Bavod on the above most commenly used
 Tatien 1.2 .53 and 1.2 .55 ，the $k$ vaiues for
stopping eight diatanse are provided in
Thele 2.13 .8 and for pessirg Night clavisarica tes $K$ values are provided in Tabie 2．13．3 The gocisich aight sotance K values are not incluces because the vertical carvature dependa an twe haight of clejet witich is varition （docending on whet te diviur has ve aee）．）
The calcuinind $K$ velums are basedon te largth ICune oxcoodigg the Eightactarce and trey longh ot curve a loas than the sight datarce． Aqpreciaben afforonces occur ony whers $A$ as that and Hie cr no aditicrad cost la linaited notbining langer vertsol turvan．
On untinisad roes ronsmpipg sigit disance is used to determine when na－passing pasment rax ge ano mqund is sourse posszlo bof noe．strising sight fimtarce is generalily attequate for ssfor pasaing manoeurss

Not－singang abje Cixancu s hes that passing Night diatarme，at sech desion speed．Fessk th manoeuvos can bo corppietod in iesp than the If passigy sidt ditance beciuns of the lining of oncoming vericies．

Tabie 21．3．2 K Factore to Provide Stopping Sight Distance on Crest Vertical Curvas

| $\begin{gathered} \text { Dosign spees } \\ (\mathrm{k}=\mathrm{ha} \mid \end{gathered}$ | Aetumes Operatineg \＄poee （ $\mathrm{mm} / \mathrm{h}$ ） | Stoppieg Sight Distancer（in） | Rate of Verficas Cumative Mo |  |
| :---: | :---: | :---: | :---: | :---: |
| 39 | 30 | 20.6 | 15 | 2 |
| 40 | 40 | 44.4 | 3.7 | 4 |
| 57 | 47.60 | 57 4．62．11 | 61.1 .3 | 6.7 |
| 6） | $55-80$ | 74．3－84．6 | 102.133 | 10－73 |
| 10 | 43， 70 | 40．1－140．E | 154.228 | 14.23 |
| 80 | $70-20$ | 1128－1324 | 23，80．96 | 24．36 |
| 90 | 72－90 | 19124837 | 320：528 | 38.53 |
| 160 | 65． 100 | 152．0．2350 | 458．750 | 45．80 |
| 110 | 81－110 | 179．5－2684 | $39.0-112.7$ | （0）－110 |
| 120 | 65． 120 | 2029.235 .6 | $764.151 / 4$ | P－350 |
| 590 | 195－130 | 22703279 | 384－199．5 | 96200 |

Algrement and Lane Conflyuralion
$\rightarrow$ min

Mexinumarak Petign Damen Cunturye Aids

Ahnougn se reiationthip swiwen sesign speeds and maximam grode is reiatively s．ejective，mestonstie puides tor mexamum grepe hige beari deveioped
The ouidires for macinkm gradenm oregiven nithere 2.13 .1

Makinum Onas：Deainn Domion Apploaton Hembiki

1．Thersige of vaiueo shomm ia Table 21,2 ． 1 ecugnurs war meerruir grice seecied lor doslon varies net topogapty and the geveral finspole：cepablity of the rood
atifoty to lund tee coplat wones For kowe caasficaion uthan roans，liand ine is an incorporatod inte Fre guisetines for iatian local aed vpan uncrivied soasctona

2．Tha values shown majy be nadjefod to sot losir and sconomic conditices．Maskimum grefients by chasstratioe incland use are offon a mater ce policy．ans as a cesult． vary from junsdialion to jurisditation． a serior ergineerny and plerners livepl in aer aveot in aflusing those foures
 crinstor the impura of such alliernative moximum grade vaives on satefy

Table 21．3．1 Maximum Gradients＊

| $\begin{aligned} & \text { Des ge } \\ & \text { Speed ammp } \\ & \text { Tapograpty } \end{aligned}$ | 304950 |  |  | 60 |  | 70 |  | 80 |  | 30 |  | 100 |  | 170 |  | 120430 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | A | \＃ | F | 4 | \＃ | M | 月 | $u$ | H | 4 | H | t | － | $u$ | R | n |
| fu |  | 7 | 11 | 7 | 11 | 6 | 9 | 6 | \＃ | 5 | 7 | 5 | 7 | － | － | ， | ＋ |
| ACU |  | － | － | 6 | 90 | 6 | $\theta$ | 5 | 1 | 8 | 7 | 5 | 7 | － | $=$ | $\pm$ | \＆ |
| ACD |  | － | $\checkmark$ | － | $\bigcirc$ | 6 | 9 | ${ }^{6}$ | d | 8 | 7 | 5 | 7 | － | ， | ＋ | － |
| maj |  | ＋ | $\stackrel{+}{4}$ | ． | ． | \％ | ． | 4 | 7 | 4 | 古 | 3 | 5 | 3 | \％ | 3 | 3 |
| RAD |  |  | $\pm$ | ， | － | － | － | 4 | 7 | s | \＃ | 3 | 5 | 3 | 3 | 3 | 5 |
| FFO |  |  | 4 | － | ＋ | － | － | － | － | － | － | $\square$ | 3 | 3 | 5 | 3 | 5 |
| UuJ－ |  | ＊ | 15 | － | － | － | － | ＊ | － | － | － | － | － | － | － | － | ＋ |
| Pevidential ULIJ－ Industrial |  | t | 12 | － | － | － | － | － | － | － | － | $\rightarrow$ | ＋ | － | － | ＊ | ＋ |
| Convmaroal UCU－ |  | $\theta$ | 12 | 7 | 11 | 7 | 10 | － | $\bullet$ | － | － | ＊ | － | － | ＊ | ＋ | ＊ |
| Resiounal |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | － |
| UCU－ kduetray |  | ， | 12 | E． | ＂1 | 6 | 9 | 6 | A | － | ＊ | ＋ | $\sim$ | － | － | $\sim$ | ＊ |
| Commectal |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| UCD |  | $E$ | 10 | ह | 9 | 5 | ${ }^{8}$ | 5 | 7 | $t$ | ， | \％ | － | $=$ | ＊ | \％ | $\%$ |
| Wat |  |  | 10 | 6. | 3 | 5 | 6 | 5 | 7 | － | ， | － | ， | $\because$ | ＊ | － | － |
| UND |  |  | ， | 3. | 6 | 3 | 6 | 3 | \＆ | 3 | a | 3 | 5 | $\cdots$ | \％ | ． | ＊ |
| UED |  |  | ＋ | ． | － | ， | － | 5 | 6 | 4 | 5 | 4 | 5 | 4 | 5 | 3 | 5 |
| 3 FO |  | － | \％ | － | $\bigcirc$ | 1 | － | $\checkmark$ | $\checkmark$ | 4 | 5 | 5 | 5 | 3 |  | 3 | 5 |
| Noter | 1．Beit sudat tesa than 150 m in tangth，arg one wiy down groces may be <br>  <br> 2．A retors to roing topognachy． <br> 3 M relors io moumuineis topography |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Pago 21．32 |  |  |  |  |  |  |  |  |  |  |  |  |  | Sepramber 799 |  |  |  |
| watyons |  | Geomenie Deaign Guide for Conadian Roads |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |


| Tabie 2．1．3．4 |  | K Factoes to Provide Mininum Stopping Sight Distance on Sag Vertical Curves＇ |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Desian Speed | Assimet！ Operating bmil | Steppling 8ight Unstane （mil |  |  |  |  |
|  |  |  | Heatliyt Centent |  | Combtan Cantral |  |
|  |  |  | Cuculisted | Rounded | Caicainted | Kownead |
| 30 | 30 | 23．3 | 39 | 4 | 2.7 | 2 |
| 40 | 40 | 64.4 | 71 | 7 | 4.1 | 4 |
| 50 | 47－50 | 574．42 | 40．2．11．5 | 11－12 | 5.563 | $5-6$ |
| 50 | $56-60$ | 7e．3－84．5 | \｛4．6．17．1 | 15.10 | 7.791 | 4.9 |
| 76 | 65－70 | 99． $1-1108$ | 12．8－74 1 | 20－25 | 100－12．4 | 10－12 |
| B0 | 70.60 | 1128．739．4 | 24．8．31．7 | 25：32 | 12．4－16．2 | 12.48 |
| 30 | $n 700$ | $13+2 \mathrm{ncos}$ | 29.6 .43 .1 | 30.40 | 250．200 | 15.20 |
| 109 | 45－100 | 151．0－2060 | 36.7501 | 3 P 30 | 供．7253 | 18.25 |
| 110 | 91.110 | 179.3 .2084 | 410．61 ${ }^{\text {\％}}$ | $43-62$ | 21.030 .6 | 2100 |
| 120 | 5a－120 |  | 45．6－72．7 | 50.73 | 24.3364 | 34， 3 |
| 130 | 105－130 | 2278.3279 | 36． $7-15.0$ | By－${ }^{\text {a }}$ | 27， 9 －428 | 20－4］ |

Wahke for sag curvatiee bisee on the comtion －thurion ase shomn is Tatier 2．1．3．4
Theak umimetreagoines annutadinurtan shatons suctusumberimese were tia chien necesalary lar popeny and scceas roasorelo a dsara as poasibe Minirum satuss ave hormaily exceeged where leasioie，in previderkiond pocsicie powar tainess sicither inalfunctions to the streat ligiting systime． Desidning asg vorfical turvet wiong curved rogowas lor soosioe siphe cistance is normaty tiot feasble ive to the intruror flat graces and renulart turtace ominoge probieme．

2．1．3．4 Sight Distance at Underpasses
While not a focuent dosign probiem，fhe slaht dietance atunsoppasspes ray be metrictod due Intwowr，ss shat ve ar wiy hangleg teion the botut dre overpass shucturn reaincling
 than the turimun mapping sight distanco．

 an Underpass cloasunce of C．Case i is for a eight siampor crestor than the langth of the woica curw $(8 \times$ ）and Case 2 is for $x$ eight

Figure 2．t．3．3 Sight Distance at Underpass


The horizontal alignment of Township Road 20 is straight and on tangent. The existing vertical profile contains some crest and sag curves that appear to be within the $80 \mathrm{~km} / \mathrm{hr}$. posted speed standard and is fairly flat. All grades appear to be less than 8\%.

### 3.3.1 Site Observations

Site observations of Township Road 540 between September 10 to 14 are shown below:


## SUMMARY

Upon inspection, Township Road 540 appears to be consistent with a Rural Local Road County standard with widths from 8 to 9 m . The existing roadway exhibits cross sectional attributes near or within the standard that includes sideslopes, ditch widths and backslopes. The horizontal and vertical alignments also appear satisfactory and consistent with standard.

There are no concerns with the present geometric, sight distances and condition of Township Road 540.

## 4. EXISTING CONDITIONS - HIgHWAYS

### 4.1 Existing Intersection Treatment - Hwy. 16:14 \& RR 20

The details of the Hwy. 16:14 \& RR 20 intersection site are as follows:

- Highway $16: 14$ is a four-laned roadway and is paved.
- The intersection has an existing treatment as follows:
- EBL's: Right Turn
- Deceleration: 80 m 25:1 Taper with 100 m parallel 3.5 m right turn lane;
- Acceleration: 90m parallel 3.5m right turn lane with a $110 \mathrm{~m} 30: 1$ Taper;
- EBL's Left Turn
- Deceleration: 70m 20:1Taper with 100 m parallel 3.5 m left turn lane;
- Acceleration: 120m 25:1 Taper with 120m parallel 3.5m left turn lane;
- WBL's Right Turn
- Deceleration: 140m 40:1 Taper with 100 m parallel 3.5 m right turn lane;
- Acceleration: 75m Taper
- WBL's Left Turn
- Deceleration: 120m 35:1 Taper with 90m parallel left turn lane.
- Acceleration: None
- There is existing full intersection illumination.
- The side slopes seem to be 5:1 or better throughout.




### 4.2 Existing Intersection Treatment - Hwy. 43:22 \& Twp. Rd. 540

The details of the Hwy. 43:22 \& Twp. Rd. 540 intersection site are as follows:

- Highway 43:22 is a four-laned roadway and is paved.
- The intersection has an existing treatment as follows:
- NBL's: Right Turn
- Deceleration: None;
- Acceleration: None;
- NBL's Left Turn
- Deceleration: 90m 30:1Taper with 120 m parallel 3.0m left turn lane;
- Acceleration: None
- SBL's Right Turn
- Deceleration: 140m 40:1 Taper with 50m parallel 3.0m right turn lane;
- Acceleration: None
- SBL's Left Turn
- Deceleration: 80m 25:1 Taper with 110 m parallel 3.0-3.5m left turn lane.
- Acceleration: None
- There is no existing illumination.
- The side slopes seem to be 5:1 or better throughout.




### 4.3 Available Traffic Data

Existing Alberta Transportation intersectional traffic count locations are shown in the map below:


AT website http://www.transportation.alberta.ca/3460.htm has traffic counts available that are relevant for comparison purposes for this assessment.

There are two specific traffic counts available of interest for this assessment as shown below:

Table-4.1a: 2014 AADT and ASDT from Alberta Highways Traffic Volume History

| Intersection Leg | 2014 AADT |  |
| :---: | :---: | :---: |
| 16 \& Lake Eden Rd (74523) | $23580-$ Hwy. 16 | $1940-$ RR 20 |
| $43 \& 633$ E of Alberta Beach (71530) | $12960-$ Hwy. 43 |  |
| $16 \& 43$ at Manly Corner (72510) | $13360-$ Hwy. 43 |  |

## TRAFFIC IMPACT ASSESSMENT

## LAUNCH VENTURES RESIDENTIAL

SE 1-54-2-W5M
Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

Refermen Ne: rasgis
hemersestion at
\$5 S LANR TOENRD $92-53-2500000150$


net hufle from Nom Tuning fige in. Tulk Tom Now Tranglat




E. Trific Pion Ime Timing lat

ET. Tuife Fome Ime hocembing Trewen
We Toke Fow Weat Tumeg lope


Mot Aowep Amer Daly Naste

selot of hevary I it Devenker 31 (Sel depo
asOn Aeewpe limerer Dabl Natic

serot of Moy 1 w Septereer 30 ISt devic


## LAUNCH VENTURES RESIDENTIAL

SE 1-54-2-W5M
Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

Reforence Na: 74530
liemsestion at
43563 E OF NLERTA BENOI
$204 \mathrm{MADT} / \mathrm{ASOT}$ ESTMAFL

## TRAFFIC IMPACT ASSESSMENT

LAUNCH VENTURES RESIDENTIAL
SE 1-54-2-W5M
Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

### 4.4 Traffic Counts

A traffic count was recently on Sept. 10, 2015, conducted by D\&A Paulichuk Consulting Ltd. on Rge. Rd. 20, just north of the proposed development. The results indicate that the 2015 AADT for RR 20 at the development access is approximately 995.
$\mathrm{D}_{\&} \mathrm{~A}$ covsulting LTD CONSULTING LTD.

Turning Movement Sumurry Diagram


2015
from Traffic Count Conversion


### 4.5 Traffic Growth - Hwy. 16:14

The following historical traffic data for Highway 16:14 is available from the Alberta Transportation's website, which indicates a growth of approximately $6.72 \%$ growth for the east leg of Hwy. 16 at RR 20, from 1989 to 2014 ( 25 years) and a $2.26 \%$ from 2007 to 2014 (7 years).

Historical Traffic Volumes - Hwy. 16:14

| Year | $\mathbf{2 0 0 4}$ | $\mathbf{2 0 0 5}$ | $\mathbf{2 0 0 6}$ | $\mathbf{2 0 0 7}$ | $\mathbf{2 0 0 8}$ | $\mathbf{2 0 0 9}$ | $\mathbf{2 0 1 0}$ | $\mathbf{2 0 1 1}$ | $\mathbf{2 0 1 2}$ | $\mathbf{2 0 1 3}$ | $\mathbf{2 0 1 4}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AADT | $\mathbf{1 5 6 3 0}$ | $\mathbf{1 6 3 9 0}$ | $\mathbf{1 8 7 6 0}$ | $\mathbf{2 0 3 6 0}$ | $\mathbf{2 0 4 8 0}$ | $\mathbf{2 1 3 0 0}$ | $\mathbf{2 1 6 6 0}$ | $\mathbf{2 0 6 9 0}$ | $\mathbf{2 2 4 0 0}$ | $\mathbf{2 3 0 4 0}$ | $\mathbf{2 3 5 8 0}$ |


| Year | $\mathbf{1 9 9 3}$ | $\mathbf{1 9 9 4}$ | $\mathbf{1 9 9 5}$ | $\mathbf{1 9 9 6}$ | $\mathbf{1 9 9 7}$ | $\mathbf{1 9 9 8}$ | $\mathbf{1 9 9 9}$ | $\mathbf{2 0 0 0}$ | $\mathbf{2 0 0 1}$ | $\mathbf{2 0 0 2}$ | $\mathbf{2 0 0 3}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AADT | $\mathbf{9 9 4 0}$ | $\mathbf{1 0 1 3 0}$ | $\mathbf{1 0 0 2 0}$ | $\mathbf{9 7 6 0}$ | $\mathbf{1 0 4 9 0}$ | $\mathbf{1 1 2 8 0}$ | $\mathbf{1 1 5 7 0}$ | $\mathbf{1 2 8 3 0}$ | $\mathbf{1 3 5 5 0}$ | $\mathbf{1 4 2 9 0}$ | $\mathbf{1 4 2 8 0}$ |


| Year | $\mathbf{1 9 8 9}$ | $\mathbf{1 9 9 0}$ | $\mathbf{1 9 9 1}$ | $\mathbf{1 9 9 2}$ |
| :--- | :---: | :---: | :---: | :---: |
| AADT | $\mathbf{8 8 0 0}$ | $\mathbf{9 0 6 0}$ | $\mathbf{9 0 4 0}$ | $\mathbf{9 2 5 0}$ |

Since there has been a noticeable growth rate change between the longer term and shorter term, $4.5 \%$ will be used for projecting the next 20 years.

### 4.6 Traffic Growth - Hwy. 43:22

The following historical traffic data for Highway 43:22 is available from the Alberta Transportation's website, which indicates a growth of $4.03 \%$ per annum noncompounded from 1986 to 2014 ( 28 years), $3.46 \%$ from 1994 to 2014 ( 20 years), $3.08 \%$ from 2004 to 2104 ( 10 years) and a $3.00 \%$ from 2009 to 2014 (5 years) for the south leg of Hwy. 43 from Hwy. 633.

## Historical Traffic Volumes - Hwy. 43:22

| Year | $\mathbf{2 0 0 4}$ | $\mathbf{2 0 0 5}$ | $\mathbf{2 0 0 6}$ | $\mathbf{2 0 0 7}$ | $\mathbf{2 0 0 8}$ | $\mathbf{2 0 0 9}$ | $\mathbf{2 0 1 0}$ | $\mathbf{2 0 1 1}$ | $\mathbf{2 0 1 2}$ | $\mathbf{2 0 1 3}$ | $\mathbf{2 0 1 4}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AADT | $\mathbf{9 9 1 0}$ | $\mathbf{1 0 2 3 0}$ | $\mathbf{1 1 0 3 0}$ | $\mathbf{1 1 7 7 0}$ | $\mathbf{1 1 3 2 0}$ | $\mathbf{1 1 2 8 0}$ | $\mathbf{1 1 5 3 0}$ | $\mathbf{1 1 5 1 0}$ | $\mathbf{1 2 0 5 0}$ | $\mathbf{1 2 2 7 0}$ | $\mathbf{1 2 9 6 0}$ |


| Year | $\mathbf{1 9 9 3}$ | $\mathbf{1 9 9 4}$ | $\mathbf{1 9 9 5}$ | $\mathbf{1 9 9 6}$ | $\mathbf{1 9 9 7}$ | $\mathbf{1 9 9 8}$ | $\mathbf{1 9 9 9}$ | $\mathbf{2 0 0 0}$ | $\mathbf{2 0 0 1}$ | $\mathbf{2 0 0 2}$ | $\mathbf{2 0 0 3}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| AADT | $\mathbf{7 3 9 0}$ | $\mathbf{7 6 6 0}$ | $\mathbf{7 7 0 0}$ | $\mathbf{7 6 5 0}$ | $\mathbf{7 9 1 0}$ | $\mathbf{8 2 9 0}$ | $\mathbf{8 2 9 0}$ | $\mathbf{8 5 3 0}$ | $\mathbf{8 9 2 0}$ | $\mathbf{9 2 5 0}$ | $\mathbf{9 6 0 0}$ |


| Year | 1984 | $\mathbf{1 9 8 5}$ | $\mathbf{1 9 8 6}$ | $\mathbf{1 9 8 7}$ | $\mathbf{1 9 8 8}$ | $\mathbf{1 9 8 9}$ | $\mathbf{1 9 9 0}$ | $\mathbf{1 9 9 1}$ | $\mathbf{1 9 9 2}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| AADT |  |  | $\mathbf{6 0 9 0}$ | $\mathbf{6 4 9 0}$ |  | $\mathbf{6 7 2 0}$ | $\mathbf{6 8 5 0}$ | $\mathbf{6 8 6 0}$ | $\mathbf{7 0 0 0}$ |

A growth rate of $3.39 \%$ will be used for the next 20 years, which is the average of the four rates shown above.

### 4.7 Traffic Growth - Rge. Rd. 20 \& Twp. Rd. 540

The following historical traffic data for Range Road 20 north of Hwy. 16 is available from the Alberta Transportation's website, which indicates a growth of approximately $0.80 \%$ growth for north leg off of Hwy. 16:14 intersection, from 2002 to 2014 (12 years).

Historical Traffic Volumes - Rge. Rd. 20

| Year | $\mathbf{2 0 0 4}$ | $\mathbf{2 0 0 5}$ | $\mathbf{2 0 0 6}$ | $\mathbf{2 0 0 7}$ | $\mathbf{2 0 0 8}$ | $\mathbf{2 0 0 9}$ | $\mathbf{2 0 1 0}$ | $\mathbf{2 0 1 1}$ | $\mathbf{2 0 1 2}$ | $\mathbf{2 0 1 3}$ | $\mathbf{2 0 1 4}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| AADT | $\mathbf{1 8 1 0}$ | $\mathbf{1 8 3 0}$ | $\mathbf{2 2 8 0}$ | $\mathbf{2 4 6 0}$ | $\mathbf{2 5 4 0}$ | $\mathbf{2 6 4 0}$ | $\mathbf{2 6 8 0}$ | $\mathbf{1 7 7 0}$ | $\mathbf{1 8 4 0}$ | $\mathbf{1 9 0 0}$ | $\mathbf{1 9 4 0}$ |


| Year | 2002 | 2003 |
| :--- | :--- | :--- |
| AADT | $\mathbf{1 7 7 0}$ | $\mathbf{1 7 4 0}$ |

A growth rate of $1.0 \%$ per year will be used for Range Road 20 for analysis purposes.
For Twp. Rd. 540 between Hwy. 43 and RR 20, there is no historical data available. A growth rate of $1.0 \%$ per year will be used for Twp. Rd. 540 for analysis purposes.

### 4.8 Back Ground Traffic Turning Movement Diagrams

The background traffic turning movement diagrams are shown below for 2016.
The 2014 AADT for Twp. Rd. 540 is estimated to be 600 to the west and 140 to the east based on existing development in the area.

LAUNCH VENTURES RESIDENTIAL
SE 1-54-2-W5M
Lac Ste. Anne County TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

## Turning Moremeet Sumenary Dlagran



Turning Mowement Sumenary Diagrank


## 5. ANALYSIS - Development Access \& Rge. Rd. 20

The proposed development is to access Range Road 20 to the east as shown below.


The projected traffic volumes for the next 20 years is as follows:
Existing Highway Traffic Forecast, Daily Volumes

| Year | Range Road 20 <br> South Leg | Dev. Ent. <br> West Leg |
| :---: | :---: | :---: |
| 2016 | 1043 | 29 |
| 2021 (5 Year) | 1187 | 124 |
| 2026 (10 Year) | 1239 | 124 |
| 2036 (20 Year) | 1336 | 124 |

### 5.1 Initial Determination based on Traffic Volume Warrant Chart

The 2016, 2021, 2026 and 2036 AADT values for the Rge. Rd. 20 and Development Access intersection indicate from referencing Figure D-7.4, "Traffic Volume Warrant Chart for At-Grade Intersection Treatment on Two-Lane Rural Highways (Design Speeds 100/110/120 km/h)", that a Type II intersection maybe warranted for 2021, 2026 and 2036.


Due to the $80 \mathrm{~km} / \mathrm{hr}$. posted speed limit, a detailed analysis is required to further determine the appropriate intersection treatment type. It is important to note, that the above chart is for design speeds of $110 \mathrm{~km} / \mathrm{h}$ and greater.

### 5.2 Design Speed

The posted speed limit on Range Road 20 is $80 \mathrm{~km} / \mathrm{hr}$. It is therefore reasonable to conclude that a design speed of $90 \mathrm{~km} / \mathrm{h}$ is suitable.

### 5.3 Detailed Analysis

## Right Turn

In accordance with Alberta Transportation's "Highway Geometric Design Guide" (Section D.7.7), an exclusive right turn lane is warranted on an undivided highway when all three of the following conditions are met:

- Main (or though) road AADT $\geq 1,800$
- Intersecting road AADT $\geq 900$
- Right turn daily traffic volume $\geq 360$ for the movement in question.

The following table indicates the status of these requirements for right turns from Range Road 20.

Table 5.3a - Right Turn Warrant

| Condition | Base Year (2016) | $\begin{aligned} & \hline 5 \text { Year } \\ & (2021) \end{aligned}$ | $\begin{gathered} 10 \text { Year } \\ (2026) \end{gathered}$ | $\begin{gathered} 20 \text { Year } \\ (2036) \\ \hline \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: |
|  | (Condition Met)(Condition Met) |  | (Condition Met) | (Condition Met) |
| Main Road (Rge. Rd. 20) | 1043 | 1187 | 1239 | 1336 |
| AADT $\geq 1800$ | (No) | (No) | (No) | (No) |
|  |  |  |  |  |
| Intersecting Road (Dev. Ent.) | 29 | 124 | 124 | 124 |
| AADT $\geq 900$ | (No) | (No) | (No) | (No) |
|  |  |  |  |  |
| Right turn daily traffic $\geq \mathbf{3 6 0}$ | 0 | 0 | 0 | 0 |
|  | (No) | (No) | (No) | (No) |
|  |  |  |  |  |
| For movement in question | (No) | (No) | (No) | (No) |

Based on the projected volumes, an exclusive right turn lane is not required for the next 20 years with the added development traffic.

## Left Turn

The Highway Geometric Design Guide Section D.7.6 gives graphical guidelines for determining left turn warrant. The graphs use peak (100 th highest) hour volumes and factor in percent turning and design speed to identify the required treatment for the intersection. The following table shows the treatments needed for current and

## TRAFFIC IMPACT ASSESSMENT

LAUNCH VENTURES RESIDENTIAL

## SE 1-54-2-W5M

Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14
projected traffic volumes.
Table 5.3b - Required Treatment Type PM PEAK

|  | Base Year <br> $\mathbf{( 2 0 1 6 )}$ | 5 Year <br> $(\mathbf{2 0 2 1})$ | 10 Year <br> $(\mathbf{2 0 2 6})$ | 20 Year <br> $(\mathbf{2 0 3 6})$ |
| :--- | :---: | :---: | :---: | :---: |
| Peak 100th Hour - p.m. |  |  |  |  |
| \% Left Turns | $5.9 \%$ | $18.6 \%$ | $18.2 \%$ | $17.0 \%$ |
| $\mathbf{V}_{\mathbf{a}}=$ Advancing Volume (VPH) | 34 | 43 | 44 | 47 |
| $\mathbf{V}_{\mathbf{0}}=$ Opposing Volume (VPH) | 34 | 36 | 37 | 41 |
| $\mathbf{V I =}$ Left turning Volume (VPH) | 2 | 8 | 8 | 8 |
|  |  |  |  |  |
|  |  |  |  |  |
| Design Speed | $90 \mathrm{~km} / \mathrm{hr}$ | $90 \mathrm{~km} / \mathrm{hr}$ | $90 \mathrm{~km} / \mathrm{hr}$ | $90 \mathrm{~km} / \mathrm{hr}$ |
| Required Treatment Type | Type I | Type I | Type I | Type I |



Main Rd: Range Road 20
Minor Rd: Devolopment Engance

Direation: NB Count date or year: 2021

The Alberta Transportation hourly analysis charts are both indicating a result to the far bottom, against the x-axis. This is indication that an intersection treatment above a Type I is not required since the opposing traffic and advancing traffic are minimal and provide significant opportunity to make a left turn. Based on this, a Type I intersection treatment is warranted between 2016-2036.

### 5.4 Intersectional Sight Distance

In accordance with section D.4, "Sight Distances at Intersections", the sight distance for left turning vehicles from the approach, without interfering with vehicles nearing the intersection, is used for determination of minimum sight distance requirements. Using Fig. D-4.2.2.2 below, the required sight distances for various vehicle types with a $110 \mathrm{~km} / \mathrm{hr}$ design speed are as follows:

## Vehicle Type

Passenger Vehicle (P)
Required Sight Distance - 90 kph

Single Unit or Bus (SU)
Semi-Trailer Combination (WB15)
175 m

Semi-Trailer Combination (WB2),
265 m

WB23, WB28, WB33)

## LAUNCH VENTURES RESIDENTIAL

SE 1-54-2-W5M
Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14


It is anticipated that the largest vehicle type to use the intersection will be WB-15 which requires 350 m of sight distance. Presently there is more than 350 m of sight distance available to the south and north of this intersection.

## TRAFFIC IMPACT ASSESSMENT

LAUNCH VENTURES RESIDENTIAL

## 6. ANALYSIS - HwY, 16:14 \& Rge. Rd, 20

The following tables show the estimated combined traffic volumes at the intersection.

| Combined Traffic Forecast, Daily Volumes |  |  |
| :---: | :---: | :---: |
| Year | Hwy. 16:14 <br> East Leg <br> Combined | Range Road 20 <br> North Leg <br> Combined |
| 2016 | 25580 | 1994 |
| 2021 | 30590 | 2149 |
| 2026 | 35556 | 2246 |
| 2036 | 45668 | 2618 |

Projected peak hour traffic loading (100th highest hour) is shown below for am/pm:

| Combined Traffic Forecast, Peak Hour Volumes |  |  |
| :---: | :---: | :---: |
|  | Hwy. 16:14 | Range Road 20 |
|  | East Leg | North Leg <br> Combined |
| Year | Combined | am/pm |
| 2016 | $2590 / 2601$ | $229 / 135$ |
| 2021 | $3080 / 3120$ | $245 / 147$ |
| 2026 | $3565 / 3638$ | $256 / 153$ |
| 2036 | $4542 / 4675$ | $281 / 166$ |

### 6.1 Design Speed

The posted speed on Highway 16:14 at this location is $110 \mathrm{~km} / \mathrm{hr}$. It is therefore reasonable to conclude that a design speed of $120 \mathrm{~km} / \mathrm{h}$ is suitable.

### 6.2 Intersecting Road Classification

Intersection treatment on divided highways are characterized into three general formats:

- No Treatment; very low intersecting volumes (approx. less than 200 AADT)
- MINOR Roadway (i.e. Road Allowances)
- MAJOR Roadway (i.e. Highways, Town Access Roads, Park Access Roads)

However it is best to verify the proper treatment by completing a more detail analysis of each separate movement using the projected turning movement traffic volumes as per the following sections.

### 6.3 Detailed Analysis

### 6.3.1 Right Turn Lane

In accordance with Section D.8.7, "Warrants for Right Turn Lanes on Four-Lane Divided Highways", the following is apparent:

- $\quad$ The Right Turn Lane Volume must be at least 360 vehicles per day to warrant a Right Turn Lane. This is summarized in the following Table:

Table: Right Turn Lane Warrant

| Condition | Base Year <br> $\mathbf{( 2 0 1 6 )}$ | 5 Year <br> $(\mathbf{2 0 2 1})$ | 10 Year <br> $\mathbf{( 2 0 2 6 )}$ | 20 Year <br> $(\mathbf{2 0 3 6})$ |
| :---: | :---: | :---: | :---: | :---: |
| Range Road 20 | (Condition Met) | (Condition Met) | Condition Met) | (Condition Met) |
| Right turn daily traffic $\geq \mathbf{3 6 0}$ | 598 | 651 | 679 | 915 |
| For movement in question | (Yes) | (Yes) | (Yes) | (Yes) |

Based on the projected volumes, a Right Turn Lane is presently warranted. A 100 m right turn lane and 140 m taper presently exists which satisfies this warrant.

### 6.3.2 Left Turn Lane

The Highway Geometric Design Guide Section D.8.6, "Warrants for Left Turn Lanes on Four-Lane Divided Highways" gives graphical guidelines for determining left turn warrant. The graphs use peak ( $100^{\text {th }}$ highest) hour volumes and factor in percent turning and design speed to identify the required treatment for the intersection. The following table shows the treatments needed for current and projected traffic volumes.

Table: Required Treatment Type

|  | Base Year <br> $(\mathbf{2 0 1 6})$ | 5 Year <br> $(\mathbf{2 0 2 1})$ | 10 Year <br> $(\mathbf{2 0 2 6})$ | 20 Year <br> $(\mathbf{2 0 3 6})$ |
| :--- | :---: | :---: | :---: | :---: |
| Range Road 20 |  |  |  |  |
| Peak am 100th Hour |  |  |  |  |
| $\mathbf{V}_{\mathbf{I}}=$ Turning Volume (VPH) | 7 | 8 | 8 | 10 |
| $\mathbf{V}_{\mathbf{0}}=$ Opposing Volume (VPH) | 1474 | 1768 | 2061 | 2649 |
| Treatment Required | No | Yes | Yes | Yes |

The above values are then used on Figure D-8.6c, "Warrants for Left Turn Lanes and Storage Requirements for Four-Lane Divided Highways", shown below.

## LAUNCH VENTURES RESIDENTIAL

## SE 1-54-2-W5M

Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

Based on the projected volumes, a Left Turn Lane is not warranted until 2020 for the EB to NB left turning movement. A full left turn lane and taper already exists at this location and meets the requirements.

APRIL $1995 \quad$| Alborta infrastructure |
| :--- | HIOHWAY QEOMETRIC DESION QUIDE

FIGURE D-8.6c WARRANTS FOR LEFT TURN LANES AND STORAGE REOUIREMENTS FOR FOUR-LANE DIVIDED HIGHWAYS


## SUMMARY:

Presently, the intersection already contains a "Major Road Intersection on Four-Lane Divided Highway" intersection treatment and therefore no improvements are required for the next 20 years.

### 6.4 Intersectional Sight Distance

In accordance with section D.4, "Sight Distances at Intersections", the sight distance for left turning vehicles from the approach, without interfering with vehicles nearing the intersection, is used for determination of minimum sight distance requirements. Using Fig. D-4.2.2.2, shown in Appendix C, the required sight distances for various vehicle types with a $120 \mathrm{~km} / \mathrm{hr}$ design speed are as follows:

Vehicle Type
Passenger Vehicle (P)
Single Unit or Bus (SU)
Semi-Trailer Combination
(WB15)
Semi-Trailer Combination (WB21, WB23, WB28, WB33)

Required Sight Distance - 120 km/hr. 233 m
355 m
470 m
613 m

$80100120 \quad 140160180200220240260280300320340360380400420440460480500520540560580600620640660680700$ 0 = REOUIRED SIGHT DISTANCE ALONG MAJOR TWO-LANE HIGHWAY IN METRES

Hwy. 16:14 EBL's

The intersection of Hwy. 16:14 and Range Road 20 is approximately at km 4.8.

The intersection is on a horizontal curve of 2500 m radius alignment in an east west direction.

The intersection is on a grade of $+0.2 \%$.

The sight distance from RR 20 to the west is $>800 \mathrm{~m}$ to the east.

# LAUNCH VENTURES RESIDENTIAL 

Hwy. 16:14 WBL's



The site distance is greater than 650 metres in both directions on Hwy. 16 at the intersection of Range Road 20.

## LAUNCH VENTURES RESIDENTIAL

## SE 1-54-2-W5M

TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

### 6.5 Highway Capacity Analysis

The capacity analysis was performed for the intersection for the 100th highest hour for the AM Peak for Years 2016, 2021, 2026 and 2036. The traffic analysis was completed using Synchro 9 software based on HCM 2010 methodology. A saturation flow of 1700 vpl was used in this analysis. See the tables below for the results.


It is important to note that Synchro does not take into account acceleration lanes. As seen in the above schematic, deceleration lanes are shown but not acceleration lanes. Acceleration lanes exist for right turns coming out of Range Road 20 as well as a left turn acceleration for the SB to EB movement onto Hwy. 16.

For this reason, some of the results below will be incorrect.

## TRAFFIC IMPACT ASSESSMENT

| YEAR |  |  | Highway 16:14 |  |  |  |  |  | Rge. Rd. 20 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| TIME PERIOD |  | PARAMETERS | $\begin{gathered} \text { EB } \\ \text { Left } \end{gathered}$ | $\begin{gathered} \text { EB } \\ \text { Thru } \end{gathered}$ | $\begin{gathered} \text { EB } \\ \text { Right } \end{gathered}$ | WB <br> Left | $\begin{aligned} & \text { WB } \\ & \text { Thru } \end{aligned}$ | $\begin{aligned} & \hline \text { WB } \\ & \text { Right } \end{aligned}$ |  | $\begin{gathered} \text { SB } \\ \text { All } \\ \text { Turns } \end{gathered}$ |
| $\begin{aligned} & \text { Year } \\ & 2016 \end{aligned}$ | AM <br> Peak | LOS | B | A | A | C | A | A | D | F* |
|  |  | Delay (s) | 11.7 | 0.0 | 0.0 | 15.6 | 0.0 | 0.0 | 26.3 | 165.3 |
|  |  | v/c Ratio | 0.01 | 0.44 | 0.00 | 0.06 | 0.29 | 0.04 | 0.45 | 1.10 |
|  |  | 95 ${ }^{\text {th }}$ Que | 0.4 | 0.0 | 0.0 | 1.4 | 0.0 | 0.0 | 17.9 | 69.7 |
|  | PM Peak | LOS | C | A | A | B | A | A | E | E* |
|  |  | Delay (s) | 16.1 | 0.0 | 0.0 | 12.9 | 0.0 | 0.0 | 42.6 | 47.1 |
|  |  | v/c Ratio | 0.02 | 0.35 | 0.01 | 0.06 | 0.45 | 0.03 | 0.48 | 0.35 |
|  |  | 95 ${ }^{\text {th }}$ Que | 0.6 | 0.0 | 0.0 | 1.5 | 0.0 | 0.0 | 18.2 | 11.5 |
| $\begin{aligned} & \text { Year } \\ & 2021 \end{aligned}$ | AM <br> Peak | LOS | B | A | A | C | A | A | E | F* |
|  |  | Delay (s) | 13.2 | 0.0 | 0.0 | 19.8 | 0.0 | 0.0 | 42.7 | 548.6 |
|  |  | v/c Ratio | 0.02 | 0.53 | 0.00 | 0.08 | 0.35 | 0.04 | 0.62 | 1.96 |
|  |  | 95 ${ }^{\text {th }}$ Que | 0.4 | 0.0 | 0.0 | 2.1 | 0.0 | 0.0 | 29.1 | 119.4 |
|  |  | LOS | C | A | A | C | A | A | F | F* |
|  | PM <br> Peak | Delay (s) | 20.4 | 0.0 | 0.0 | 15.8 | 0.0 | 0.0 | 93.5 | 98.6 |
|  |  | v/c Ratio | 0.04 | 0.43 | 0.01 | 0.08 | 0.54 | 0.03 | 0.74 | 0.60 |
|  |  | 95 ${ }^{\text {th }}$ Que | 0.9 | 0.0 | 0.0 | 2.1 | 0.0 | 0.0 | 32.8 | 21.8 |
| $\begin{aligned} & \text { Year } \\ & 2026 \end{aligned}$ | AM <br> Peak | LOS | C | A | A | D | A | A | F | F* |
|  |  | Delay (s) | 15.1 | 0.0 | 0.0 | 25.8 | 0.0 | 0.0 | 84.0 | >500 |
|  |  | v/c Ratio | 0.02 | 0.62 | 0.00 | 0.11 | 0.41 | 0.04 | 0.84 | 4.71 |
|  |  | $95^{\text {th }}$ Que | 0.6 | 0.0 | 0.0 | 3.0 | 0.0 | 0.0 | 47.5 | >250 |
|  | PM <br> Peak | LOS | D | A | A | C | A | A | F | F* |
|  |  | Delay (s) | 26.5 | 0.0 | 0.0 | 18.4 | 0.0 | 0.0 | 241.2 | 247.1 |
|  |  | v/c Ratio | 0.05 | 0.49 | 0.01 | 0.11 | 0.63 | 0.15 | 1.15 | 1.00 |
|  |  | $95^{\text {th }}$ Que | 1.0 | 0.0 | 0.0 | 2.6 | 0.0 | 0.0 | 47.8 | 30.2 |
| $\begin{aligned} & \text { Year } \\ & 2036 \end{aligned}$ | AM Peak | LOS | C | A | A | E | A | A | F | F* |
|  |  | Delay (s) | 20.5 | 0.0 | 0.0 | 49.4 | 0.0 | 0.0 | 409.2 | >500 |
|  |  | v/c Ratio | 0.04 | 0.80 | 0.00 | 0.23 | 0.53 | 0.05 | 1.65 |  |
|  |  | 95 ${ }^{\text {th }}$ Que | 1.0 | 0.0 | 0.0 | 6.6 | 0.0 | 0.0 | 103.1 | >250 |


|  | LOS | E | A | A | D | A | A | F | $\mathrm{F}^{*}$ |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PM | Delay (s) | 49.2 | 0.0 | 0.0 | 28.8 | 0.0 | 0.0 | $>500$ | $>500$ |
| Peak | V/c Ratio | 0.12 | 0.63 | 0.01 | 0.19 | 0.82 | 0.03 | 4.31 |  |
|  | $95^{\text {th }}$ Que | 3.1 | 0.0 | 0.0 | 5.4 | 0.0 | 0.0 | $>250$ | $>250$ |

*Note: The SB turns are showing a LOS of F and high delay times which is inaccurate since Synchro does provide calculation for efficiencies due to acceleration lanes.

As noted above, the above results indicate that the southbound turns are all at a Level of Service (LOS) of F. This does not represent the actual operation on site since a 120 m left turn acceleration and 120 m taper exists for the southbound to eastbound movement. In order to validate this, a site inspection was conducted on Sept. 8 to 10, 2015, and the wait times were measured. The results indicated an average wait time of 20 seconds to 55 seconds during the AM Peak (average approximately 35 seconds). This is significantly lower than the Synchro result of 126.7 seconds shown above. Again, this likely due to the fact that Synchro does not use calculations that account for acceleration lanes. Therefore in extrapolating this analysis, the NB and SB turns will both likely be at a LOS of E in 2020. In 2025, the NB and SB turns will both likely be at a LOS of $F$. The proposed development fractionally increased the results above, since it only adds 4 left turns in the AM Peak Hour and 2 left turns in the PM Peak Hour.

Therefore, in summary, the intersection is performing near capacity presently. Full capacity maybe reached within the next $10-15$ years, mostly depending on the growth of Highway 16. The intersection should be monitored in about $5-10$ years to further evaluate the operation and safety.

LEVEL OF SERVICE (LOS) CRITERIA

| Control Delay Per Vehicle (s) | LOS by Volume to Capacity Ratio |  |
| :---: | :---: | :---: |
|  | $\leq 1$ | $>1$ |
| $\leq 10$ | A | F |
| $>10$ and $\leq 15$ | B | F |
| $>15$ and $\leq 25$ | C | F |
| $>25$ and $\leq 35$ | D | F |
| $>35$ and $\leq 50$ | E | F |
| $>50$ | F | F |



Level of Service "A"


Level of Service "C"


Level of Service "E"


Level of Service "B"


Level of Service "D"


Level of Service "F"

## IRAFFIC IMPACT ASSESSMENT

## LAUNCH VENTURES RESIDENTIAL

## SE 1-54-2-W5M

Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

### 6.6 Future Freeway Planning

It is important to complete a review on how the traffic in the area will be integrated into the future freeway system. Highway 16 is designated as a future freeway in Alberta. Access to this future freeway will only be made via a grade separation or interchange. Presently, Alberta Transportation's long range plan is to construct an interchange at the Highway 16 and Range Road 20 intersection location. See below:


Therefore, the present Range Road 20 will be directly connected to the future interchange. No re-routing of Range Road 20 will be required. The timing of this improvement is not known at this time. Traffic volume on Range Road 20 is likely to increase due to the consolidation of traffic in the area occurs with the implementation of the interchange.

## TRAFFIC IMPACT ASSESSMENT

LAUNCH VENTURES RESIDENTIAL

## 7. ANALYSIS - HwY. 43:22 \& TWP. RD. 540

The following tables show the estimated combined traffic volumes at the intersection.

| Combined Traffic Forecast, Daily Volumes |  |  |
| :---: | :---: | :---: |
| Year | Hwy. 43:22 <br> South Leg <br> Combined | Twp. Rd. 540 <br> East Leg <br> Combined |
| 2016 | 14147 | 156 |
| 2021 | 16364 | 200 |
| 2026 | 18554 | 208 |
| 2036 | 22935 | 222 |

Projected peak hour traffic loading (100th highest hour) is shown below for am/pm:

| Combined Traffic Forecast, Peak Hour Volumes |  |  |
| :---: | :---: | :---: |
|  | Hwy. 43:22 | Twp. Rd. 540 |
|  | South Leg | East Leg |
|  | Combined | Combined |
| Year | am/pm | am/pm |
| 2016 | $1437 / 1442$ | $15 / 15$ |
| 2021 | $1661 / 1667$ | $18 / 19$ |
| 2026 | $1882 / 1890$ | $18 / 19$ |
| 2036 | $2329 / 2338$ | $22 / 23$ |

### 7.1 Design Speed

The posted speed on Highway 43:22 at this location is $110 \mathrm{~km} / \mathrm{hr}$. It is therefore reasonable to conclude that a design speed of $120 \mathrm{~km} / \mathrm{h}$ is suitable.

### 7.2 Intersecting Road Classification

Intersection treatment on divided highways are characterized into three general formats:

- No Treatment; very low intersecting volumes (approx. less than 200 AADT)
- MINOR Roadway (i.e. Road Allowances)
- MAJOR Roadway (i.e. Highways, Town Access Roads, Park Access Roads)

However it is best to verify the proper treatment by completing a more detail analysis of each separate movement using the projected turning movement traffic volumes as per the following sections.

### 7.3 Detailed Analysis

### 7.3.1 Right Turn Lane

In accordance with Section D.8.7, "Warrants for Right Turn Lanes on Four-Lane Divided Highways", the following is apparent:

- $\quad$ The Right Turn Lane Volume must be at least 360 vehicles per day to warrant a Right Turn Lane. This is summarized in the following Table:

Table: Right Turn Lane Warrant

| Condition | Base Year <br> (2016) | 5 Year <br> (2021) | 10 Year <br> (2026) | 20 Year <br> (2036) |
| :---: | :---: | :---: | :---: | :---: |
| Twp. Rd. 540 | (Condition Met) | (Condition Met) | (Condition Met) | (Condition Met) |
| Right turn daily traffic $\geq \mathbf{3 6 0}$ | 45 | 60 | 62 | 66 |
| For movement in question | (No) | (No) | (No) | (No) |

Based on the projected volumes, a Right Turn Lane is not warranted for the next 20 years.

### 7.3.2 Left Turn Lane

The Highway Geometric Design Guide Section D.8.6, "Warrants for Left Turn Lanes on Four-Lane Divided Highways" gives graphical guidelines for determining left turn warrant. The graphs use peak ( $100^{\text {th }}$ highest) hour volumes and factor in percent turning and design speed to identify the required treatment for the intersection. The following table shows the treatments needed for current and projected traffic volumes.

Table: Required Treatment Type

|  | Base Year <br> $(\mathbf{2 0 1 6})$ | 5 Year <br> $(\mathbf{2 0 2 1})$ | 10 Year <br> $(\mathbf{2 0 2 6})$ | 20 Year <br> $(\mathbf{2 0 3 6})$ |
| :--- | :---: | :---: | :---: | :---: |
| Twp. Rd. 540 |  |  |  |  |
| Peak PM 100th Hour |  |  |  |  |
| $\mathbf{V}_{\mathbf{1}}=$ Turning Volume (VPH) | 3 | 4 | 4 | 5 |
| $\mathbf{V}_{\mathbf{0}}=$ Opposing Volume (VPH) | 718 | 830 | 941 | 1165 |
| Treatment Required | No | No | No | Yes |

The above values are then used on Figure D-8.6c, "Warrants for Left Turn Lanes and Storage Requirements for Four-Lane Divided Highways", shown below.

## LAUNCH VENTURES RESIDENTIAL

## SE 1-54-2-W5M

Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

Based on the projected volumes, a Left Turn Lane is not warranted for the next 20 years for the BB to EB left turning movement. A full left turn lane and taper already exists at this location and exceeds this requirement.
APRIL $1895 \quad$ HIGHWAY GEOMETRIC DESION OUIDE

FIGURE D-8.6c WARRANTS FOR LEFT TURN LANES AND STORAGE REOUIREMENTS FOR FOUR-LANE DIVIDED HIGHWAYS


## SUMMARY:

Presently, the intersection already contains a left turn lane for SB to EB left turns. There is no right turn lane and tapers for NB to EB , however this not warranted due to the low turning volumes. Therefore no improvements are required for the next 20 years.

### 7.4 Intersectional Sight Distance

In accordance with section D.4, "Sight Distances at Intersections", the sight distance for left turning vehicles from the approach, without interfering with vehicles nearing the intersection, is used for determination of minimum sight distance requirements. Using Fig. D-4.2.2.2, shown in Appendix C, the required sight distances for various vehicle types with a $120 \mathrm{~km} / \mathrm{hr}$ design speed are as follows:

Vehicle Type
Passenger Vehicle (P)
Single Unit or Bus (SU)
Semi-Trailer Combination
(WB15)
Semi-Trailer Combination (WB21, WB23, WB28, WB33

Required Sight Distance - $120 \mathrm{~km} / \mathrm{hr}$. 233 m
355 m
470 m
613 m

$80 \quad 100 \quad 120 \quad 140 \quad 160180200220240260280300320340360380400420440460480500520540560580600620640 \quad 660680700$ D = REOUIRED SIGHT DISTANCE ALONG MAJOR TWO-LANE HIGHWAY IN METRES

The sight distance from Twp. Rd. 540 to the south and north is $>650 \mathrm{~m}$.

### 7.5 Highway Capacity Analysis

The capacity analysis was performed for the intersection for the 100th highest hour for the AM Peak for Years 2016, 2021, 2026 and 2036. The traffic analysis was completed using Synchro 9 software based on HCM 2010 methodology. A saturation flow of 1700 vpl was used in this analysis. See the tables below for the results.


## TRAFFIC IMPACT ASSESSMENT

| YEAR |  | Highway 43:22 |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |

## LAUNCH VENTURES RESIDENTIAL

SE 1-54-2-W5M
Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

| LOS <br> PM <br> Peak | Delay (s) | 12.1 | 0.0 | 0.0 | 11.7 | 0.0 | 0.0 | 20.5 | 24.1 |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | V/c Ratio | 0.05 | 0.48 | 0.25 | 0.01 | 0.37 | 0.01 | 0.14 | 0.06 |
|  | $95^{\text {th }}$ Que | 1.2 | 0.0 | 0.0 | 0.2 | 0.0 | 0.0 | 4.0 | 1.5 |

The above results indicate that the intersection will operate with sufficient capacity for the next 20 years.

## TRAFFIC IMPACT ASSESSMENT

## LAUNCH VENTURES RESIDENTIAL

## SE 1-54-2-W5M

Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

### 7.6 Future Freeway Planning

It is important to complete a review on how the traffic in the area will be integrated into the future freeway system. Highway 43 is designated as a future freeway in Alberta. Access to this future freeway will only be made via a grade separation or interchange. Presently, Alberta Transportation's long range plan is to construct an interchange at the Highway 43 and Hwy. 633 intersection location. The at-grade Hwy. 43:22 and Twp. Rd. 540 intersection will be closed. This will require the redirection of traffic via service roads and local roads in order to access the nearest interchange. Twp. Rd. 540 will likely be one of these collector roads that will increase in traffic due to these future changes.


## TRAFFIC IMPACT ASSESSMENT

## 8. ILLUMINATION \& SIGNALIZATION WARRANTS

Street lighting presently exists at the intersection of Hwy. 16:14 and Rge. Rd. 20. Signalization is not allowed on Hwy. 16 as it is designated as a future freeway.

Street lighting does not exist at the intersection of Hwy. 43:22 and Twp. Rd. 540. illumination warrants were reviewed. The results are summarized in the Table below:

| Location | Year | Illumination Warrant Score | Illumination Warrant Met? (Min. 120) | Signalization Warrant Score | Signalization Warrant Met? |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{gathered} \text { Hwy. 43:22 } \\ \& \\ \text { Twp. Rd. } 540 \end{gathered}$ | 2016 | 96 | No | Not Allowed | N/A |
| $\begin{gathered} \text { Hwy. 43:22 } \\ \& \\ \text { Twp. Rd. } 540 \end{gathered}$ | 2021 | 96 | No | Not Allowed | N/A |
| $\begin{gathered} \text { Hwy. 43:22 } \\ \& \\ \text { Twp. Rd. } 540 \\ \hline \end{gathered}$ | 2026 | 96 | No | Not Allowed | N/A |
| $\begin{gathered} \text { Hwy. 43:22 } \\ \& \\ \text { Twp. Rd. } 540 \end{gathered}$ | 2036 | 96 | No | Not Allowed | N/A |

Illumination is not warranted at the Hwy. 43:22 \& Twp. Rd. 540 intersection. Signalization is not allowed on Hwy. 43 as it is designated as a future freeway.

For the other intersections along Range Road 20 and Township Road 540, there is no requirement for illumination or signalization since the traffic volumes are too low, as demonstrated by the warranted intersection treatments discussed earlier.

## 9. CONCLUSIONS \& RECOMMENDATIONS

### 9.1 Conclusions \& Recommendations

The proposed development is for a 13 lot Residential rural subdivision within SE 1-542 -W5M. The site will utilize Range Road 20 for access to the east and then south on Rge. Rd. 20 to the south to Hwy 16 or west on Twp. Rd. 540 (Heatherdown Road) to Hwy. 43. See below:


For the Residential development Iots, ITE 210 - "Single-Family Detached Housing" Land Use from the Institute of Transportation Engineers (ITE) Trip Generation Manual, $9^{\text {th }}$ Edition, can used. The following data is available from the manual:

| Time Period | Trips per Lot |
| :--- | :---: |
| Daily | 9.52 |
| Peak Hour of Adjacent Street Traffic <br> One Hour Between 7 and 9 a.m. | 0.75 |
| Peak Hour of Adjacent Street Traffic <br> One Hour Between 4 and 6 p.m. | 1.00 |

## IRAFFIC IMPACT ASSESSMENT

Therefore, the estimated trip generation of the proposed development with 13 new lots is as follows:

| Daily Traffic: | 13 lots $\times 9.52$ trips/day | $=$ | 124 trips per day |
| :--- | :--- | :--- | :--- |
| AM Peak Hour: | 13 lots $\times 0.75$ trips/hour | $=$ | 10 trips per hour |
| PM Peak Hour: | 13 lots $\times 1.00$ trips/hour | $=13$ trips per hour |  |

It is estimated that 3 lots will be occupied in 2016 and the remaining 10 lots will be occupied in 2021 (Year 5). The trip distribution is anticipated to be $40 \%$ to the west on Twp. Rd. 540 to Hwy. 43 and $60 \%$ to the south on Range Road 20 to Hwy. 16, with $25 \%$ entering $/ 75 \%$ exiting in the AM Peak and $63 \%$ entering $/ 37 \%$ exiting in the PM Peak. The distribution of traffic for 2012 is projected as follows:


A detailed intersection analysis was completed on three potentially impacted intersections. The analysis concluded the following:

## Development Access and Range Road 20intersection

- A Type I intersection treatment is warranted between 2016-2036.
- It is anticipated that the largest vehicle type to use the intersection will be WB15 which requires 350 m of sight distance. Presently there is more than 350 m of sight distance available to the south and north of this intersection.
- A highway capacity analysis was not performed at this location as the traffic volumes are low and the intersection should have sufficient capacity above a LOS of $D$ for the next 20 years.
- Illumination or signalization is not required for this location as the traffic volumes are too low.


## Range Road 20 and Highway 16:14 intersection

- Presently, the intersection already contains a "Major Road Intersection on FourLane Divided Highway" intersection treatment with left turn acceleration lane for SB to EB left turns. No improvements are required for the next 20 years.
- The site distance is greater than 650 metres in both directions on Hwy. 16 at the intersection of Range Road 20.
- Upon completing a highway capacity analysis, the intersection is performing near capacity presently. Full capacity maybe reached within the next $10-15$ years, mostly depending on the growth of Highway 16. The intersection should be monitored in about 5-10 years to further evaluate the operation and safety.
- Street lighting presently exists at the intersection of Hwy. 16:14 and Rge. Rd. 20. Signalization is not allowed on Hwy. 16 as it is designated as a future freeway.


## Township Road 540 and Highway 43:22 intersection

- No improvements are required for the next 20 years.
- The sight distance from Twp. Rd. 540 to the south and north is $>650 \mathrm{~m}$.
- Upon completing a highway capacity analysis, the intersection will operate with sufficient capacity for the next 20 years.


## LAUNCH VENTURES RESIDENTIAL

## SE 1-54-2-W5M

- Illumination is not warranted at the Hwy. 43:22 \& Twp. Rd. 540 intersection. Signalization is not allowed on Hwy. 43 as it is designated as a future freeway.

The internal locals roads, Range Road 20 and Township Road 540 appear to be acceptable for transporting the proposed development traffic and are within acceptable parameters of the road standards for rural collector roadways.

### 9.2 Closure

We trust the information provided meets your present requirements. Should any questions arise, please contact our office at your convenience.


Darcy O. Paulichuk, P. Eng.


Dec. 9, 2015
APEGGA Permit to Practice Number: P12132

## APPENDIX A

## ALBERTA HIGHWAYS TRAFFIC VOLUME HISTORY TRAFFIC DATA - TURNING MOVEMENT DIAGRAMS

## Turning Movement Summary Diagram

Reference No.: 72510

## Intersection of:

16 \& 43 AT MANLY CORNER

| North On 43 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Vehicle Type |  |  | Vol | \% |
| A: Pasaenger Vehicie |  |  | 10725 | 80.3 |
| A: Recreational Veticte |  |  | 204 | 15 |
| C: Bua |  |  | 41 | 0.3 |
| D: Single Unit Truck |  |  | 605 | 4.5 |
| E: Tractor Treiler Unit |  |  | 1785 | 13.4 |
| ASDT | 15000 | AADT | 13360 |  |



## Turning Movement Summary Diagram

Reference No.: 72510

## Intersection of:

16 \& 43 AT MANLY CORNER

2014 a.m. 100th Highest Hour ESTIMATES

| North On 43 |  |  |  |
| :---: | :---: | :---: | :---: |
| Vehicle Type |  | Voi | \% |
| A: Paskenger Vehicle |  | 1039 | 84.4 |
| A: Recreational Venicle |  | $?$ | 06 |
| C: Bus |  | 9 | 07 |
| D: Single Unit Truck |  | 45 | 37 |
| E: Tractor Treiler Unit. |  | 130 | 10.6 |
|  | Total | 1231 |  |



TURNING MOVEMENT ABBREVIATIONS
NR: Traffic From North Turning Right
NL.: Tratfic From North Tuming Lefi
NI: Traffic From North Proceecing Through
SR: Traffo From South Turning Rignt
SL. Tratic From South Tuming Lett
ST: Traffic From Sounh Proceeding Through
ER: Traffe From East Turning Righ
EL - Traffic From East Turring Left
ET: Traffic From Eas! Proceeding Throsgh
WR: Tratfic From West Turning Right
WL. Traflic From West Tuming Left
WT: Traffic Fram West Proceeding Through
C

## Turning Movement Summary Diagram

Reference No.: 72510

## Intersection of:

16 \& 43 AT MANLY CORNER

| North On 43 |  |  |  |
| :---: | :---: | :---: | :---: |
| Vehicle Type |  | Voi | \% |
| A: Paskenger Vehicle |  | 1099 | 84.3 |
| A: Recreational venicle |  | 19 | 15 |
| C: Bus |  | 1 | 0.1 |
| D: Single Unit Truck |  | 37 | 28 |
| E: Tractor Treiler Unit. |  | 148 | 11.3 |
|  | Total | 1304 |  |



## Turning Movement Summary Diagram

Reference No.: 71530

## Intersection of:

$43 \& 633$ E OF AL BERTA BEACH

| North On 43 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Vehicle Type |  |  | Voi | \% |
| A: Pasaenger Vehilele |  |  | 8734 | 79.1 |
| A: Recreational vehicle |  |  | 531 | 48 |
| C: Bus |  |  | 9 | 0.1 |
| 0: : Single Unit Truck |  |  | 361 | 3.3 |
| E:Tractor Treiler Unit |  |  | 1405 | 12.7 |
| ASDT | 12400 | AADT | 11040 |  |



## Turning Movement Summary Diagram

Reference No.: 71530

## Intersection of:

$43 \& 633$ E OF ALBERTA BEACH


## Turning Movement Summary Diagram

Reference No.: 71530

## Intersection of:

$43 \& 633$ E OF ALBERTA BEACH


# APPENDIX B <br> Development Access \& Rge. Rd. 20 

TURNING MOVEMENT DIAGRAMS

## Turning Movement Summary Diagram

Intersection of: Rge. Rd. 20 \& Development Access

2014 AADT



## Turning Movement Summary Diagram









## Turning Movement Summary Diagram



CONSULTING LTD.

## Turning Movement Summary Diagram







## Turning Movement Summary Diagram

















# APPENDIX C - Hwy. 16 \& Rge. Rd. 20 

## TURNING MOVEMENT DIAGRAMS SYNCHRO ANALYSIS

## Turning Movement Summary Diagram

Intersection of: Highway 16:14 \& Rge. Rd. 20


NR: Traffic from North Turning Righ
NL: Traffic from North Turning Left
Traffic from North Proceeding Through
$\begin{array}{ll}\text { SR: } & \text { Traffic from South Turning Right } \\ \text { SL: } & \text { Traffic from South Turning Left }\end{array}$
Traffic from South Proceeding Through
R: Traffic from East Turning Right
Traffic from East Turning Left
Traffic from East Proceeding Through
TR: Taffic from West Turning Right
WT: Traffic from West Proceeding Through
TURNING MOVEMENT ABBREVIATIO
DAT: Average Annual Daily Traffic
Average daily traffic expressed as vehicles per day for period of
January 1 to December 31 ( 365 days)
ASDT: Average Summer Daily Traffic Average daily traffic expressed as vehilces per day for period of May 1 to September 30 ( 153 days)





## Turning Movement Summary Diagram









## Turning Movement Summary Diagram














## Turning Movement Summary Diagram







## Turning Movement Summary Diagram

2036 DEVELOPMENT PM PEAK HOURLY VOLUME

Traffic from South Turning Right
Traffic from South Turning Left
ST: Traffic from South Proceeding Through
ER: Traffic from East Turning Right
EL: Traffic from East Turning Left
ET: Traffic from East Proceeding Through
$\begin{array}{ll}\text { WR: } & \text { Taffic from West Turning Right } \\ \text { WL: } & \text { Traffic from West Turning Left }\end{array}$
WT: Traffic from West Proceeding Through
tURNING MOVEMENT ABBREVIATION
ADT: Average Annual Daily Traffic
Average Annual Daily Traftic
Average daily traffic expressed as vehicles per day for period of January 1 to December 31 (365 days)
ASDT: Average Summer Daily Traffic May 1 to September 30 (153 days)


## Turning Movement Summary Diagram





|  | 4 | $\rightarrow$ | $\cdots$ | 7 |  | 4 | 4 | 9 | 7 | ( | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 中4 | F' | ${ }^{*}$ | 44 | 「 |  | * |  |  | * |  |
| Traffic Volume (veh/h) | 7 | 1371 | 2 | 18 | 914 | 59 | 0 | 16 | 110 | 118 | 21 | 8 |
| Future Volume (Veh/h) | 7 | 1371 | 2 | 18 | 914 | 59 | 0 | 16 | 110 | 118 | 21 | 8 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 8 | 1490 | 2 | 20 | 993 | 64 | 0 | 17 | 120 | 128 | 23 | 9 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed ( $\mathrm{m} / \mathrm{s}$ ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | Raised |  |  | Raised |  |  |  |  |  |  |  |
| Median storage veh) |  | 3 |  |  | 3 |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1057 |  |  | 1492 |  |  | 2063 | 2603 | 745 | 1922 | 2541 | 496 |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  | 1506 | 1506 |  | 1033 | 1033 |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  | 557 | 1097 |  | 890 | 1508 |  |
| vCu , unblocked vol | 1057 |  |  | 1492 |  |  | 2063 | 2603 | 745 | 1922 | 2541 | 496 |
| tC, single (s) | 4.5 |  |  | 4.5 |  |  | 7.6 | 6.6 | 7.0 | 7.6 | 6.6 | 7.0 |
| tC, 2 stage (s) |  |  |  |  |  |  | 6.6 | 5.6 |  | 6.6 | 5.6 |  |
| tF (s) | 2.4 |  |  | 2.4 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 99 |  |  | 94 |  |  | 100 | 89 | 66 | 8 | 84 | 98 |
| cM capacity (veh/h) | 548 |  |  | 359 |  |  | 119 | 156 | 350 | 139 | 145 | 511 |
| Direction, Lane \# | EB 1 | EB 2 | EB 3 | EB 4 | WB 1 | WB 2 | WB 3 | WB 4 | NB 1 | SB 1 |  |  |
| Volume Total | 8 | 745 | 745 | 2 | 20 | 496 | 496 | 64 | 137 | 160 |  |  |
| Volume Left | 8 | 0 | 0 | 0 | 20 | 0 | 0 | 0 | 0 | 128 |  |  |
| Volume Right | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 64 | 120 | 9 |  |  |
| cSH | 548 | 1700 | 1700 | 1700 | 359 | 1700 | 1700 | 1700 | 303 | 145 |  |  |
| Volume to Capacity | 0.01 | 0.44 | 0.44 | 0.00 | 0.06 | 0.29 | 0.29 | 0.04 | 0.45 | 1.10 |  |  |
| Queue Length 95th (m) | 0.4 | 0.0 | 0.0 | 0.0 | 1.4 | 0.0 | 0.0 | 0.0 | 17.9 | 69.7 |  |  |
| Control Delay (s) | 11.7 | 0.0 | 0.0 | 0.0 | 15.6 | 0.0 | 0.0 | 0.0 | 26.3 | 165.3 |  |  |
| Lane LOS | B |  |  |  | C |  |  |  | D | F |  |  |
| Approach Delay (s) | 0.1 |  |  |  | 0.3 |  |  |  | 26.3 | 165.3 |  |  |
| Approach LOS |  |  |  |  |  |  |  |  | D | F |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 10.6 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 76.6\% |  | CU Level | Service |  |  | D |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | 4 | $\rightarrow$ | $\cdots$ | 7 |  | 4 | 4 | 9 | 7 | $\checkmark$ | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 44 | F' | * | 44 | 「 |  | * |  |  | \& |  |
| Traffic Volume (veh/h) | 7 | 1080 | 10 | 27 | 1405 | 42 | 4 | 44 | 29 | 18 | 16 | 8 |
| Future Volume (Veh/h) | 7 | 1080 | 10 | 27 | 1405 | 42 | 4 | 44 | 29 | 18 | 16 | 8 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 8 | 1174 | 11 | 29 | 1527 | 46 | 4 | 48 | 32 | 20 | 17 | 9 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed ( $\mathrm{m} / \mathrm{s}$ ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | Raised |  |  | Raised |  |  |  |  |  |  |  |
| Median storage veh) |  | 3 |  |  | 3 |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1573 |  |  | 1185 |  |  | 2029 | 2821 | 587 | 2244 | 2786 | 764 |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  | 1190 | 1190 |  | 1585 | 1585 |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  | 839 | 1631 |  | 659 | 1201 |  |
| vCu , unblocked vol | 1573 |  |  | 1185 |  |  | 2029 | 2821 | 587 | 2244 | 2786 | 764 |
| tC, single (s) | 4.5 |  |  | 4.5 |  |  | 7.6 | 6.6 | 7.0 | 7.6 | 6.6 | 7.0 |
| tC, 2 stage (s) |  |  |  |  |  |  | 6.6 | 5.6 |  | 6.6 | 5.6 |  |
| tF (s) | 2.4 |  |  | 2.4 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 98 |  |  | 94 |  |  | 98 | 62 | 93 | 80 | 87 | 97 |
| cM capacity (veh/h) | 332 |  |  | 484 |  |  | 165 | 127 | 445 | 100 | 134 | 340 |
| Direction, Lane \# | EB 1 | EB 2 | EB 3 | EB 4 | WB 1 | WB 2 | WB 3 | WB 4 | NB 1 | SB 1 |  |  |
| Volume Total | 8 | 587 | 587 | 11 | 29 | 764 | 764 | 46 | 84 | 46 |  |  |
| Volume Left | 8 | 0 | 0 | 0 | 29 | 0 | 0 | 0 | 4 | 20 |  |  |
| Volume Right | 0 | 0 | 0 | 11 | 0 | 0 | 0 | 46 | 32 | 9 |  |  |
| cSH | 332 | 1700 | 1700 | 1700 | 484 | 1700 | 1700 | 1700 | 177 | 130 |  |  |
| Volume to Capacity | 0.02 | 0.35 | 0.35 | 0.01 | 0.06 | 0.45 | 0.45 | 0.03 | 0.48 | 0.35 |  |  |
| Queue Length 95th (m) | 0.6 | 0.0 | 0.0 | 0.0 | 1.5 | 0.0 | 0.0 | 0.0 | 18.2 | 11.5 |  |  |
| Control Delay (s) | 16.1 | 0.0 | 0.0 | 0.0 | 12.9 | 0.0 | 0.0 | 0.0 | 42.6 | 47.1 |  |  |
| Lane LOS | C |  |  |  | B |  |  |  | E | E |  |  |
| Approach Delay (s) | 0.1 |  |  |  | 0.2 |  |  |  | 42.6 | 47.1 |  |  |
| Approach LOS |  |  |  |  |  |  |  |  | E | E |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 2.1 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 64.5\% |  | CU Level | Service |  |  | C |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | 4 | $\rightarrow$ | $\cdots$ | 7 |  | 4 | 4 | 9 | 7 | （ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 中4 | F＇ | ${ }^{*}$ | 中4 | 「 |  | ＊ |  |  | ＊ |  |
| Traffic Volume（veh／h） | 7 | 1655 | 2 | 19 | 1102 | 62 | 0 | 17 | 115 | 127 | 22 | 10 |
| Future Volume（Veh／h） | 7 | 1655 | 2 | 19 | 1102 | 62 | 0 | 17 | 115 | 127 | 22 | 10 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\％ |  |  | 0\％ |  |  | 0\％ |  |  | 0\％ |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate（vph） | 8 | 1799 | 2 | 21 | 1198 | 67 | 0 | 18 | 125 | 138 | 24 | 11 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width（m） |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed（ $\mathrm{m} / \mathrm{s}$ ） |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare（veh） |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | Raised |  |  | Raised |  |  |  |  |  |  |  |
| Median storage veh） |  | 3 |  |  | 3 |  |  |  |  |  |  |  |
| Upstream signal（m） |  |  |  |  |  |  |  |  |  |  |  |  |
| pX，platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC ，conflicting volume | 1265 |  |  | 1801 |  |  | 2479 | 3122 | 900 | 2290 | 3057 | 599 |
| $\mathrm{vC1}$ ，stage 1 conf vol |  |  |  |  |  |  | 1815 | 1815 |  | 1240 | 1240 |  |
| vC 2 ，stage 2 conf vol |  |  |  |  |  |  | 664 | 1307 |  | 1050 | 1817 |  |
| vCu ，unblocked vol | 1265 |  |  | 1801 |  |  | 2479 | 3122 | 900 | 2290 | 3057 | 599 |
| tC，single（s） | 4.5 |  |  | 4.5 |  |  | 7.6 | 6.6 | 7.0 | 7.6 | 6.6 | 7.0 |
| tC， 2 stage（s） |  |  |  |  |  |  | 6.6 | 5.6 |  | 6.6 | 5.6 |  |
| tF（s） | 2.4 |  |  | 2.4 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \％ | 98 |  |  | 92 |  |  | 100 | 84 | 55 | 0 | 75 | 97 |
| cM capacity（veh／h） | 448 |  |  | 265 |  |  | 76 | 110 | 276 | 82 | 96 | 437 |
| Direction，Lane \＃ | EB 1 | EB 2 | EB 3 | EB 4 | WB 1 | WB 2 | WB 3 | WB 4 | NB 1 | SB 1 |  |  |
| Volume Total | 8 | 900 | 900 | 2 | 21 | 599 | 599 | 67 | 143 | 173 |  |  |
| Volume Left | 8 | 0 | 0 | 0 | 21 | 0 | 0 | 0 | 0 | 138 |  |  |
| Volume Right | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 67 | 125 | 11 |  |  |
| cSH | 448 | 1700 | 1700 | 1700 | 265 | 1700 | 1700 | 1700 | 232 | 88 |  |  |
| Volume to Capacity | 0.02 | 0.53 | 0.53 | 0.00 | 0.08 | 0.35 | 0.35 | 0.04 | 0.62 | 1.96 |  |  |
| Queue Length 95th（m） | 0.4 | 0.0 | 0.0 | 0.0 | 2.1 | 0.0 | 0.0 | 0.0 | 29.1 | 119.4 |  |  |
| Control Delay（s） | 13.2 | 0.0 | 0.0 | 0.0 | 19.8 | 0.0 | 0.0 | 0.0 | 42.7 | 548.6 |  |  |
| Lane LOS | B |  |  |  | C |  |  |  | E | F |  |  |
| Approach Delay（s） | 0.1 |  |  |  | 0.3 |  |  |  | 42.7 | 548.6 |  |  |
| Approach LOS |  |  |  |  |  |  |  |  | E | F |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 29.8 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 87．7\％ |  | CU Level | Service |  |  | E |  |  |  |
| Analysis Period（min） |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | 4 | $\rightarrow$ | $\cdots$ | 7 |  | 4 | 4 | 9 | 7 | （ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 中4 | F＇ | ${ }^{*}$ | 中4 | 「 |  | ＊ |  |  | ＊ |  |
| Traffic Volume（veh／h） | 8 | 1352 | 10 | 28 | 1693 | 47 | 4 | 46 | 30 | 20 | 17 | 9 |
| Future Volume（Veh／h） | 8 | 1352 | 10 | 28 | 1693 | 47 | 4 | 46 | 30 | 20 | 17 | 9 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\％ |  |  | 0\％ |  |  | 0\％ |  |  | 0\％ |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate（vph） | 9 | 1470 | 11 | 30 | 1840 | 51 | 4 | 50 | 33 | 22 | 18 | 10 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width（m） |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed（ $\mathrm{m} / \mathrm{s}$ ） |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare（veh） |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | Raised |  |  | Raised |  |  |  |  |  |  |  |
| Median storage veh） |  | 3 |  |  | 3 |  |  |  |  |  |  |  |
| Upstream signal（m） |  |  |  |  |  |  |  |  |  |  |  |  |
| pX，platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC ，conflicting volume | 1891 |  |  | 1481 |  |  | 2487 | 3439 | 735 | 2711 | 3399 | 920 |
| $\mathrm{vC1}$ ，stage 1 conf vol |  |  |  |  |  |  | 1488 | 1488 |  | 1900 | 1900 |  |
| vC 2 ，stage 2 conf vol |  |  |  |  |  |  | 999 | 1951 |  | 811 | 1499 |  |
| vCu ，unblocked vol | 1891 |  |  | 1481 |  |  | 2487 | 3439 | 735 | 2711 | 3399 | 920 |
| tC，single（s） | 4.5 |  |  | 4.5 |  |  | 7.6 | 6.6 | 7.0 | 7.6 | 6.6 | 7.0 |
| tC， 2 stage（s） |  |  |  |  |  |  | 6.6 | 5.6 |  | 6.6 | 5.6 |  |
| tF（s） | 2.4 |  |  | 2.4 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \％ | 96 |  |  | 92 |  |  | 96 | 39 | 91 | 64 | 80 | 96 |
| cM capacity（veh／h） | 242 |  |  | 363 |  |  | 107 | 82 | 355 | 61 | 89 | 267 |
| Direction，Lane \＃ | EB 1 | EB 2 | EB 3 | EB 4 | WB 1 | WB 2 | WB 3 | WB 4 | NB 1 | SB 1 |  |  |
| Volume Total | 9 | 735 | 735 | 11 | 30 | 920 | 920 | 51 | 87 | 50 |  |  |
| Volume Left | 9 | 0 | 0 | 0 | 30 | 0 | 0 | 0 | 4 | 22 |  |  |
| Volume Right | 0 | 0 | 0 | 11 | 0 | 0 | 0 | 51 | 33 | 10 |  |  |
| cSH | 242 | 1700 | 1700 | 1700 | 363 | 1700 | 1700 | 1700 | 118 | 83 |  |  |
| Volume to Capacity | 0.04 | 0.43 | 0.43 | 0.01 | 0.08 | 0.54 | 0.54 | 0.03 | 0.74 | 0.60 |  |  |
| Queue Length 95th（m） | 0.9 | 0.0 | 0.0 | 0.0 | 2.1 | 0.0 | 0.0 | 0.0 | 32.8 | 21.8 |  |  |
| Control Delay（s） | 20.4 | 0.0 | 0.0 | 0.0 | 15.8 | 0.0 | 0.0 | 0.0 | 93.5 | 98.6 |  |  |
| Lane LOS | C |  |  |  | C |  |  |  | F | F |  |  |
| Approach Delay（s） | 0.1 |  |  |  | 0.2 |  |  |  | 93.5 | 98.6 |  |  |
| Approach LOS |  |  |  |  |  |  |  |  | F | F |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 3.9 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 74．6\％ |  | CU Level | Service |  |  | D |  |  |  |
| Analysis Period（min） |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | 4 |  | $\checkmark$ | 7 |  | 4 | 4 | $\dagger$ | \% | ( | $\downarrow$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{1}$ | 44 | 「 | ${ }^{1}$ | 44 | 「 |  | \& |  |  | $\dagger$ |  |
| Traffic Volume (veh/h) | 8 | 1937 | 2 | 20 | 1290 | 64 | 0 | 17 | 121 | 133 | 24 | 10 |
| Future Volume (Veh/h) | 8 | 1937 | 2 | 20 | 1290 | 64 | 0 | 17 | 121 | 133 | 24 | 10 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 9 | 2105 | 2 | 22 | 1402 | 70 | 0 | 18 | 132 | 145 | 26 | 11 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed ( $\mathrm{m} / \mathrm{s}$ ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | Raised |  |  | Raised |  |  |  |  |  |  |  |
| Median storage veh) |  | 3 |  |  | 3 |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1472 |  |  | 2107 |  |  | 2892 | 3639 | 1052 | 2658 | 3571 | 701 |
| vC 1 , stage 1 conf vol |  |  |  |  |  |  | 2123 | 2123 |  | 1446 | 1446 |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  | 769 | 1516 |  | 1212 | 2125 |  |
| vCu , unblocked vol | 1472 |  |  | 2107 |  |  | 2892 | 3639 | 1052 | 2658 | 3571 | 701 |
| tC, single (s) | 4.5 |  |  | 4.5 |  |  | 7.6 | 6.6 | 7.0 | 7.6 | 6.6 | 7.0 |
| tC, 2 stage (s) |  |  |  |  |  |  | 6.6 | 5.6 |  | 6.6 | 5.6 |  |
| tF (s) | 2.4 |  |  | 2.4 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 98 |  |  | 89 |  |  | 100 | 76 | 39 | 0 | 56 | 97 |
| cM capacity (veh/h) | 366 |  |  | 195 |  |  | 48 | 77 | 218 | 34 | 60 | 374 |
| Direction, Lane \# | EB 1 | EB 2 | EB 3 | EB 4 | WB 1 | WB 2 | WB 3 | WB 4 | NB 1 | SB 1 |  |  |
| Volume Total | 9 | 1052 | 1052 | 2 | 22 | 701 | 701 | 70 | 150 | 182 |  |  |
| Volume Left | 9 | 0 | 0 | 0 | 22 | 0 | 0 | 0 | 0 | 145 |  |  |
| Volume Right | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 70 | 132 | 11 |  |  |
| cSH | 366 | 1700 | 1700 | 1700 | 195 | 1700 | 1700 | 1700 | 178 | 39 |  |  |
| Volume to Capacity | 0.02 | 0.62 | 0.62 | 0.00 | 0.11 | 0.41 | 0.41 | 0.04 | 0.84 | 4.71 |  |  |
| Queue Length 95th (m) | 0.6 | 0.0 | 0.0 | 0.0 | 3.0 | 0.0 | 0.0 | 0.0 | 47.5 | Err |  |  |
| Control Delay (s) | 15.1 | 0.0 | 0.0 | 0.0 | 25.8 | 0.0 | 0.0 | 0.0 | 84.0 | Err |  |  |
| Lane LOS | C |  |  |  | D |  |  |  | F | F |  |  |
| Approach Delay (s) | 0.1 |  |  |  | 0.4 |  |  |  | 84.0 | Err |  |  |
| Approach LOS |  |  |  |  |  |  |  |  | F | F |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 465.0 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 98.3\% |  | CU Level | Service |  |  | F |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



|  | 4 |  | $\cdots$ | 7 |  | 4 | 4 | 9 | $p$ | $t$ | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 44 | 7 | ${ }^{1}$ | 中4 | 「 |  | * |  |  | \& |  |
| Traffic Volume (veh/h) | 9 | 2504 | 2 | 22 | 1668 | 71 | 0 | 19 | 132 | 145 | 26 | 11 |
| Future Volume (Veh/h) | 9 | 2504 | 2 | 22 | 1668 | 71 | 0 | 19 | 132 | 145 | 26 | 11 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 10 | 2722 | 2 | 24 | 1813 | 77 | 0 | 21 | 143 | 158 | 28 | 12 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed ( $\mathrm{m} / \mathrm{s}$ ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | Raised |  |  | Raised |  |  |  |  |  |  |  |
| Median storage veh) |  | 3 |  |  | 3 |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 1890 |  |  | 2724 |  |  | 3722 | 4680 | 1361 | 3396 | 4605 | 906 |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  | 2742 | 2742 |  | 1861 | 1861 |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  | 980 | 1938 |  | 1534 | 2744 |  |
| vCu , unblocked vol | 1890 |  |  | 2724 |  |  | 3722 | 4680 | 1361 | 3396 | 4605 | 906 |
| tC, single (s) | 4.5 |  |  | 4.5 |  |  | 7.6 | 6.6 | 7.0 | 7.6 | 6.6 | 7.0 |
| tC, 2 stage (s) |  |  |  |  |  |  | 6.6 | 5.6 |  | 6.6 | 5.6 |  |
| tF (s) | 2.4 |  |  | 2.4 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 96 |  |  | 77 |  |  | 100 | 41 | 0 | 0 | 0 | 96 |
| cM capacity (veh/h) | 242 |  |  | 105 |  |  | 18 | 36 | 134 | 0 | 14 | 273 |
| Direction, Lane \# | EB 1 | EB 2 | EB 3 | EB 4 | WB 1 | WB 2 | WB 3 | WB 4 | NB 1 | SB 1 |  |  |
| Volume Total | 10 | 1361 | 1361 | 2 | 24 | 906 | 906 | 77 | 164 | 198 |  |  |
| Volume Left | 10 | 0 | 0 | 0 | 24 | 0 | 0 | 0 | 0 | 158 |  |  |
| Volume Right | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 77 | 143 | 12 |  |  |
| cSH | 242 | 1700 | 1700 | 1700 | 105 | 1700 | 1700 | 1700 | 99 | 0 |  |  |
| Volume to Capacity | 0.04 | 0.80 | 0.80 | 0.00 | 0.23 | 0.53 | 0.53 | 0.05 | 1.65 | Err |  |  |
| Queue Length 95th (m) | 1.0 | 0.0 | 0.0 | 0.0 | 6.6 | 0.0 | 0.0 | 0.0 | 103.1 | Err |  |  |
| Control Delay (s) | 20.5 | 0.0 | 0.0 | 0.0 | 49.4 | 0.0 | 0.0 | 0.0 | 409.2 | Err |  |  |
| Lane LOS | C |  |  |  | E |  |  |  | F | F |  |  |
| Approach Delay (s) | 0.1 |  |  |  | 0.6 |  |  |  | 409.2 | Err |  |  |
| Approach LOS |  |  |  |  |  |  |  |  | F | F |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | Err |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 119.8\% |  | CU Level | Service |  |  | H |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  | $\rangle$ | $\rightarrow$ | 7 | 7 |  | 4 | 4 | $\dagger$ | $p$ |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{4}$ | 个4 | ${ }^{7}$ | ${ }^{7}$ | 个4 | 「 |  | ¢ |  |  | $\dagger$ |  |
| Traffic Volume（veh／h） | 10 | 1970 | 12 | 32 | 2564 | 53 | 5 | 52 | 34 | 22 | 19 | 10 |
| Future Volume（Veh／h） | 10 | 1970 | 12 | 32 | 2564 | 53 | 5 | 52 | 34 | 22 | 19 | 10 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\％ |  |  | 0\％ |  |  | 0\％ |  |  | 0\％ |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate（vph） | 11 | 2141 | 13 | 35 | 2787 | 58 | 5 | 57 | 37 | 24 | 21 | 11 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width（m） |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed（ $\mathrm{m} / \mathrm{s}$ ） |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare（veh） |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | Raised |  |  | Raised |  |  |  |  |  |  |  |
| Median storage veh） |  | 3 |  |  | 3 |  |  |  |  |  |  |  |
| Upstream signal（ $m$ ） |  |  |  |  |  |  |  |  |  |  |  |  |
| pX，platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC ，conflicting volume | 2845 |  |  | 2154 |  |  | 3648 | 5078 | 1070 | 4015 | 5033 | 1394 |
| $\mathrm{vC1}$ ，stage 1 conf vol |  |  |  |  |  |  | 2163 | 2163 |  | 2857 | 2857 |  |
| $\mathrm{vC2}$ ，stage 2 conf vol |  |  |  |  |  |  | 1485 | 2915 |  | 1158 | 2176 |  |
| vCu, unblocked vol | 2845 |  |  | 2154 |  |  | 3648 | 5078 | 1070 | 4015 | 5033 | 1394 |
| tC ，single（s） | 4.5 |  |  | 4.5 |  |  | 7.6 | 6.6 | 7.0 | 7.6 | 6.6 | 7.0 |
| $\mathrm{tC}, 2$ stage（s） |  |  |  |  |  |  | 6.6 | 5.6 |  | 6.6 | 5.6 |  |
| tF（s） | 2.4 |  |  | 2.4 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \％ | 88 |  |  | 81 |  |  | 62 | 0 | 83 | 0 | 11 | 91 |
| cM capacity（veh／h） | 92 |  |  | 186 |  |  | 13 | 15 | 212 | 0 | 24 | 128 |
| Direction，Lane \＃ | EB 1 | EB 2 | EB 3 | EB 4 | WB 1 | WB 2 | WB 3 | WB 4 | NB 1 | SB 1 |  |  |
| Volume Total | 11 | 1070 | 1070 | 13 | 35 | 1394 | 1394 | 58 | 99 | 56 |  |  |
| Volume Left | 11 | 0 | 0 | 0 | 35 | 0 | 0 | 0 | 5 | 24 |  |  |
| Volume Right | 0 | 0 | 0 | 13 | 0 | 0 | 0 | 58 | 37 | 11 |  |  |
| CSH | 92 | 1700 | 1700 | 1700 | 186 | 1700 | 1700 | 1700 | 23 | 0 |  |  |
| Volume to Capacity | 0.12 | 0.63 | 0.63 | 0.01 | 0.19 | 0.82 | 0.82 | 0.03 | 4.31 | Err |  |  |
| Queue Length 95th（m） | 3.1 | 0.0 | 0.0 | 0.0 | 5.4 | 0.0 | 0.0 | 0.0 | Err | Err |  |  |
| Control Delay（s） | 49.2 | 0.0 | 0.0 | 0.0 | 28.8 | 0.0 | 0.0 | 0.0 | Err | Err |  |  |
| Lane LOS | E |  |  |  | D |  |  |  | F | F |  |  |
| Approach Delay（s） | 0.2 |  |  |  | 0.3 |  |  |  | Err | Err |  |  |
| Approach LOS |  |  |  |  |  |  |  |  | F | F |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | Err |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 104．9\％ |  | CU Level | f Service |  |  | G |  |  |  |
| Analysis Period（min） |  |  | 15 |  |  |  |  |  |  |  |  |  |

## APPENDIX D - Hwy. 43 \& Twp. Rd. 540

TURNING MOVEMENT DIAGRAMS<br>SYNCHRO ANALYSIS<br>ILLUMINATION WARRANT

## Turning Movement Summary Diagram

Intersection of: Highway 43:22 \& Twp. Rd. 540

2014 AADT
Estimated from Adjacent Data



## Turning Movement Summary Diagram





CONSULTING LTD.





## Turning Movement Summary Diagram








Turning Movement Summary Diagram




## Turning Movement Summary Diagram




## Turning Movement Summary Diagram


















|  | 4 |  |  | $\downarrow$ |  |  | 4 | 4 | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | \＄ |  | ${ }^{*}$ | 性 |  | ${ }_{1}$ | 个个 | F |
| Traffic Volume（veh／h） | 12 | 0 | 24 | 7 | 0 | 5 | 24 | 1128 | 6 | 4 | 1140 | 12 |
| Future Volume（Veh／h） | 12 | 0 | 24 | 7 | 0 | 5 | 24 | 1128 | 6 | 4 | 1140 | 12 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\％ |  |  | 0\％ |  |  | 0\％ |  |  | 0\％ |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate（vph） | 13 | O | 26 | 8 | 0 | 5 | 26 | 1226 | 7 |  | 1239 | 13 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width（m） |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed（ $\mathrm{m} / \mathrm{s}$ ） |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare（veh） |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | Raised |  |  | Raised |  |
| Median storage veh） |  |  |  |  |  |  |  | 2 |  |  | 2 |  |
| Upstream signal（m） |  |  |  |  |  |  |  |  |  |  |  |  |
| pX，platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC ，conflicting volume | 1912 | 2532 | 620 | 1909 | 2542 | 616 | 1252 |  |  | 1233 |  |  |
| VC 1 ，stage 1 conf vol | 1247 | 1247 |  | 1282 | 1282 |  |  |  |  |  |  |  |
| $\mathrm{vC2}$ ，stage 2 conf vol | 665 | 1285 |  | 628 | 1260 |  |  |  |  |  |  |  |
| vCu ，unblocked vol | 1912 | 2532 | 620 | 1909 | 2542 | 616 | 1252 |  |  | 1233 |  |  |
| tC，single（s） | 7.6 | 6.6 | 7.0 | 7.6 | 6.6 | 7.0 | 4.2 |  |  | 4.2 |  |  |
| $\mathrm{tC}, 2$ stage（s） | 6.6 | 5.6 |  | 6.6 | 5.6 |  |  |  |  |  |  |  |
| tF（s） | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| po queue free \％ | 92 | 100 | 94 | 95 | 100 | 99 | 95 |  |  | 99 |  |  |
| cM capacity（veh／h） | 159 | 153 | 424 | 147 | 146 | 426 | 535 |  |  | 545 |  |  |
| Direction，Lane \＃ | EB 1 | WB 1 | NB 1 | NB 2 | NB 3 | SB 1 | SB 2 | SB 3 | SB 4 |  |  |  |
| Volume Total | 39 | 13 | 26 | 817 | 416 | 4 | 620 | 620 | 13 |  |  |  |
| Volume Left | 13 | 8 | 26 | 0 | 0 | 4 | 0 | 0 | 0 |  |  |  |
| Volume Right | 26 | 5 | 0 | 0 | 7 | 0 | 0 | 0 | 13 |  |  |  |
| CSH | 273 | 197 | 535 | 1700 | 1700 | 545 | 1700 | 1700 | 1700 |  |  |  |
| Volume to Capacity | 0.14 | 0.07 | 0.05 | 0.48 | 0.24 | 0.01 | 0.36 | 0.36 | 0.01 |  |  |  |
| Queue Length 95th（m） | 3.9 | 1.7 | 1.2 | 0.0 | 0.0 | 0.2 | 0.0 | 0.0 | 0.0 |  |  |  |
| Control Delay（s） | 20.4 | 24.6 | 12.1 | 0.0 | 0.0 | 11.7 | 0.0 | 0.0 | 0.0 |  |  |  |
| Lane LOS | C | C | B |  |  | B |  |  |  |  |  |  |
| Approach Delay（s） | 20.4 | 24.6 | 0.2 |  |  | 0.0 |  |  |  |  |  |  |
| Approach LOS | C | C |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 0.6 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 49．1\％ | ICU Level of Service |  |  |  |  | A |  |  |  |
| Analysis Period（min） |  |  | 15 |  |  |  |  |  |  |  |  |  |



## Illumination of Isolated Rural Intersections <br> LIGHTING WARRANT SPREADSHEET

This spreadsheet is to be used in conjunction with IIlumination of Isolated Rural Intersections, Transportation Association of Canada, February 2001.
Please enter information in the cells with yellow background

| INTERSECTION CHARACTERISTICS | Date <br> Hwy. 43:22  <br> Twp. Rd. 540 Main Road <br> Minor Road <br> City/TownOther | December 9, 2015 <br> YEAR 2036 |
| :--- | :--- | :--- | :--- |


| GEOMETRIC FACTORS |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Value | Rating | Weight | Comments | Check | Score |
| Channelization Rating | Descriptive | , |  | Refer to Table 1(A) to determine rating value | OK |  |
| Presence of raised channelization? ( $\mathrm{Y} / \mathrm{N}$ ) | n |  |  |  | OK |  |
| Highest operating speed on raised, channelized approach (km/h) | 50 | 5 |  |  | OK |  |
| Channelization Factor |  |  |  |  | OK | 0 |
| Approach Sight Distance on most constrained approach (\%) | 100 | 0 | 10 | Relative to the recommended minimum sight distance | OK | 0 |
| Posted Speed limit (in 10's of km/h) | 110 |  |  | Enter "T" for tangent (no horizontal curve at the intersection) | OK |  |
| Radius of Horizontal Curve (m) | T |  |  |  | OK |  |
| Posted Speed Category = | A | 0 |  |  |  |  |
| Posted Speed Category = |  | 0 |  |  |  |  |
| Posted Speed Category = |  | 0 |  |  |  |  |
| Posted Speed Category = |  | 0 |  |  |  |  |
| Horizontal Curvature Factor |  | 0 | 5 |  | OK | 0 |
| Angle of Intersection (10's of Degrees) | 90 | 0 | 5 |  | OK | 0 |
| Downhill Approach Grade (x.x\%) | 1.0 | 0 | 3 | Rounded to nearest tenth of a percent | OK | 0 |
| Number of Intersection Legs | 4 | 2 | 3 | Number of legs $=3$ or more | OK | 6 |
|  |  |  |  | Geometric Factors Subtotal |  | 6 |

## OPERATIONAL FACTORS

| Is the intersection signalized? ( $\mathrm{Y} / \mathrm{N}$ ) | n |  |  | Calculate the Signalization Warrant Factor |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AADT on Major Road (2-way) | 22935 | 4 | 10 | Either Use the two AADT inputs OR the Descriptive Signalization | OK | 40 |
| AADT on Minor Road (2-way) | 222 | 0 | 20 |  | OK | 0 |
| Signalization Warrant | Descriptive | 0 | 30 | 1(B) for description and rating values for signalization warrant. | OK |  |
| Night-Time Hourly Pedestrian Volume | 0 | 0 | 10 | Refer to Table 1(B), note \#2, to account for children and seniors | OK | 0 |
| Intersecting Roadway Classification | Descriptive | 0 | 5 | Refer to Table 1(B) for ratings. | OK | 0 |
| Operating Speed or Posted Speed on Major Road (km/h) | 110 | 4 | 5 | Refer to Table 1(B), note \#3 | OK | 20 |
| Operating Speed on Minor Road (km/h) | 80 | 3 | 5 | Refer to Table 1(B), note \#3 | OK | 15 |
|  |  |  |  | Operational Factors Subtotal |  | 75 |

## ENVIRONMENTAL FACTOR

| Lighted Developments within 150 m radius of intersection | 0 | 0 | 5 | Maximum of 4 quadrants | OK | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Environmental Factor Subtotal | 0 |

## COLLISION HISTORY

| Average Annual night-time collision frequency due to inadequate lighting (collisions/yr, rounded to nearest whole \# ) OR | 1.0 | 1 | 15 | Enter either the annual frequency (See Table 1(C), note \#4) OR the number of collisions / MEV | OK | 15 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Collision Rate over last 3 years, due to inadequate lighting (/MEV) | 0 | 0 | 0 | (Unused values should be set to Zero) | OK | 0 |
| Is the average ratio of all night to day collisions >=1.5 (Y/N) | n | 0 |  |  | OK |  |


| Check Intersection Signalization: <br> Intersection is not Signalized |
| :---: |
| LIGHTING IS NOT WARRANTED |


| SUMMARY |  |
| ---: | :---: |
| Geometric Factors Subtotal | 6 |
| Operational Factor Subtotal | 75 |
| Environmental Factor Subtotal | 0 |
| Collision History Subtotal | 15 |
| TOTAL POINTS | $\mathbf{9 6}$ |

# "HYDROLOGICAL ASSESSMENT" 

S.E. 1-54-2-5

PREPARED BY

GROUNDWATER INFORMATION TECHNOLOGIES LTD.

## APPENDIX ‘D’

# Groundwater Information Technologies Ltd. 

Phase I Aquifer Analysis<br>SE-01-54-2W5<br>Lac Ste. Anne County

Prepared For:
Westcott Consulting Group

Prepared By:
Groundwater Information Technologies Ltd.


APEGA P12077
August 13, 2015
File: 15-1217

## Executive Summary

A review of available data was undertaken for a proposed 13 lot country residential subdivision in SE-01 $-54-2 W 5$ to determine expected water well yield and aquifer zones. Previously geological and hydrogeological studies were examined along with an examination of water well drillers' reports and groundwater quality analysis reports for the area.

According to the Water Act, if a well is installed on each lot, each lot is required to be supplied with water at a rate of $1250 \mathrm{~m}^{\prime} /$ year and water wells should be capable of providing water at a minimum rate of 1 imperial gallon per minute. If a community well is installed water needs are less, approximately 400 $\mathrm{m}^{3} /$ year for each lot.

Groundwater use in the area is primarily from domestic demands, with some wells installed for stock or industrial use.

The surficial deposits which immediately underlay the site are unconsolidated glacial deposits, which are underlain by the sandstones, siltstones and mudstones of the Horseshoe Canyon Formation. The sands and gravels of the surficial aquifer may be silty or dry underlying the site, and the primary aquifer present beneath the site are bedrock sandstones and siltstones of the Horseshoe Canyon Formation. This formation is generally found at depths greater than 20 meters and is up to 100 metres in thickness.

Analysis of available pumping test data shows that the Horseshoe Canyon Aquifer is capable of supplying water at a rate of approximately $15,940 \mathrm{~m}^{3} /$ year ( 7.1 imperial gallons per minute). This value is sufficient to supply individual lots by a well on each lot. Aquifer supplies for a single community well may not be available.

A review of water quality records for the area show that the groundwater is a sodium bicarbonate type water of Total Dissolved Solids concentration of around less than $500 \mathrm{mg} / \mathrm{h}$ and should be acceptable to most users. It is recommended that water samples be collected and analyzed for routine dissolved salts and bacterial parameters from each well prior to use as a potable supply in the house.

## Contents

Executive Summary .....
introduction ..... 2
Site Description ..... 3
Geological Description ..... 3
Water Wells and Groundwater Usage in Area ..... 4
Welts on Subject Site ..... 4
Aquifer Parameters ..... 5
Water Quality ..... A
List of Tables and Figures
Figure 1 - Site plan of proposed subdivision ..... 2
Figure 2 - Well Pumping Test Interpretation ..... 5
Figure 3 - Well pumping test interpretation ..... 6
Table 1 - Water Quality Analysis ..... 8Appendix 1 - Water Well Reconnaissance ReportAppendix 2 - Water Well Driller's Reports
Appendix 3-Water Quality Analyses Reports

## Introduction

At the request of Robert Westcott of Westcott Consulting Group, an aquifer analysis was conducted for a proposed 13- lot residential subdivision located within SEK-01-54-2W5. Lots range in size from 0.89 to 2.48 hectares (ha). A site plan outlining the proposed subdivision is shown below:

Figure 1 - Site plan of proposed subdivision


Water is to be supplied by a groundwater source. According to the Water Act, If individual wells are located on each lot then water requirements are to be supplied at a rate of 1250 cubic metres per year ( $\mathrm{m}^{1 /} /$ year) and wells should be capable of supplying water at a minimum rate of 1 imperial gallon per minute (igpm). If water needs are supplied by a community well supply, water needs are based on house size and number of bedrooms, and are generally around 250 gallons per day per lot.

This review consists of an examination of currently available information and should provide an estimate into depths, vields and quality of water from the aquifer(s) underlying the site. As no drilling or pumping tests were performed as part of the investigation, the review provides a representative analysis; however,
no guarantee is provided as to actual water quantity or quality that may be obtained by a well on any particular parcel on the site.

## Site Description

The site is located approximately 55 kilometers ( km ) east of the City of Edmonton within a mixed cropland. forested and country residential area of Lac Ste. Anne County. A portion of the 1:50,000 topographic map showing the topography and surface water features of the area is as follows:

Figure 2 - Topographic map showing subject location


The site is located in an area with gently undulating terrain at an elevation of approximately 730 meters above sea level (masl). Locally, topography dips slightly towards the east, towards the Kilini River which flows at an elevation of approximately 710 masl.

## Geological Description

The nature of strata underlying the site was reviewed with the aid of the Regionol Groundworer Assessment of Lac Ste. Anne County (Hydrogeological Consultants Ltd. 1998), Surficial Geology of Wabamum Lake Alberta, NTS 836 (Alberta Research Council, 1979). A review of the nearby water well records was also conducted to correlate the lithology record from the well with the published geological information. According to this assessment regional well capabilities from bedrock wells are on the order of $1.5-4.5$ imperial galions per minute.

The area is immediately underlain by approximately up to 20 m of quaternary age glaciofluvial deposits consisting of fine to coarse-grained sand and gravels. Also present are hummocky tills consisting of mixed sand, silt and clay with pebbles, cobbles and boulders. The presence of this formation may influence the migration of surface contaminants, such as septic field effluent, into underlying aquifers. Generally the gravels are dry or too poorly developed to serve as groundwater supply aquifers.

The Upper Horseshoe Canyon Formation of the Edmonton Group underlies the surficial deposits at the Site. This formation consists of interbedded sandstones, siltstones and mudstones and is classified as an aquifer. The Upper Horseshoe Canyon formation is approximately 100 m in thickness and is underlain by the middle and lower sections of the formation.

In the area, the Onoway Valley Channel represents a significant surficial aquifer, however this channel is located approximately 8 km north of the subject site and therefore cannot be expected to be used for water supply for the proposed subdivision.

## Water Wells and Groundwater Usage in Area

A search of the Alberta Environment water well database indicates that 59 wells have been installed within 1.6 km of the site indicating a moderate amount of groundwater usage. Domestic Use well depths range from 2-129 m, with most wells obtaining water from the Upper Horseshoe Canyon aquifer from depths below 12 m to 129 m . Most wells are used for domestic purposes with some used for a combination of domestic and stock uses. Three oilfield monitoring wells were installed in NW. 6-54-1 W5.

Initial well vields range from 2-20 igpm, with most well yields ranging around 5 - 10 igpm, indicating that moderate to good aquifers are generally encountered.

A review of Alberta Environment's water license and registration database was undertaken to determine large users of groundwater. A search of license and registrations of the subject section and adjoining eight sections was completed. One groundwater diversion licence was found within the area. Surface water diversion licenses are present in sections 11-54-2 W5, and 688-54-1 W5. Registrations are present in Sections 7-54-1 W5M. Overall groundwater usage in the area can be described as to moderate.

## Wells on Subject Site

There are six domestic use wells listed within the same quarter section as the proposed subdivision, however their exact locations are unknown. All six wells are completed in the bedrock aquifer at depths of approximately $28-32 \mathrm{~m}$. Static water levels in these wells ranged between 11.58 m and 20.73 m . Records indicate that water was removed from the wells at rates between 3.5 to 9 imperial gallons per minute (igpm), or 16 to 40 Litres per minute ( $L / \mathrm{min}$ ) indicating a productive aquifer.

## Aquifer Parameters

The closest well with available data to allow calculation of aquifer parameters is 0495878 , located within SE-01-54-2 W5. This well was installed in 1999 at a depth of 33 m , within the sandstones and shales of the Upper Horseshoe Canyon Formation. The well was pumped for 2 hours at a rate of 41 litres per minute (10 igpm), and data was collected during the recovery.

The pumping test data was interpreted with the aid of the AQTESOLV program developed by HydroSoft Inc. A confined radial flow model with full penetration and the Dougherty-Babu solution was applied. The water well drillers report for this well is artached in the appendix and a graph showing water displacement with time and the calculated aquifer parameters is as follows:

Figure 2-Well 495878 Pumping Test Interpretation


A 120 minute pumping test was undertaken on a well 01-04-54-02 W5 west of the site in 2007 (Well ID 1165293). This well also obtains water from the Horseshoe Canyon Aquifer from $56.08-79.25 \mathrm{~m}$. The solution from the pumping test is as follows:

Figure 3 - Well 1165293 pumping test interpretation


A 120 minute pumping test followed by a 120 minute recovery test was undertaken on a well at $5 W-5-54$ 1 WSM in 2012 (Well ID 1300394). This well also obtains water from the Horseshoe Canyon Aquifer from $58.83 \sim 73.15 \mathrm{~m}$. A confined radial flow model with full penetration and the Dougherty-Babu solution was applied. The water well drillers report for this well is attached in the appendix and a graph showing water displacement with time and the calculated aquifer parameters is as follows:


Transmissivity values range from 2 to $18 \mathrm{~m}^{1} /$ day from these tests. The average available head of the three wells is 12.7 m . The long term yield of a well, assuming no well bore losses, can be calculated by utilizing the Cooper and Jacob non-equilibrium equation:

0.183

Where

| Q | $=$ | Pumping rate |
| :--- | :--- | :--- |
| H | $=$ | Available Head $(12.7 \mathrm{~m})$ |
| T | $=$ | Transmissivity (Average of $10 \mathrm{~m}^{2} /$ day $)$ |
| S | $=$ | Aquifer Storativity $\left(5 \times 10^{2}-\right.$ representative of the Horseshoe Canyon |
|  |  | Formation) |
| $t$ | $=$ | Time $(20$ years or 7305 days $)$ |
| $r_{w}$ | $=$ | Well bore radius $(0.07 \mathrm{~m})$ |

A 20 year yield of $66 \mathrm{~m}^{3} /$ day $\left(24,200 \mathrm{~m}^{3} /\right.$ year $)$ is calculated. Alberta Environment and Sustainable Resource Development recommends a safety factor of 0.7 be applied, such that an average safe vield of $16,940 \mathrm{~m}^{2} /$ year ( 7.1 imperial gallons per minute) is calculated. These values show that the average well should be able to supply sufficient water for individual lots.

The values indicated that the average well in the area may have sufficient capability to provide enough water to be used as a community well, however a fair risk exists that there may be insufficient capability for a community supply well.

## Water Quality

Water quality analyses reports from the Alberta Environment and Parks water well were examined to illustrate water quality in Upper Horseshoe Canyon Formation. Chemical analysis reports from a well in SW-01-054-02 W5M (well iD 446859) and NW-36-053-02 W5M (well 10 458677), which are screened through the Upper Horseshoe River aquifer, were collected. The laboratory reports are attached in Appendix 3 with a summary of the results is as follows:

Table 1-Water Quality Analysis

| Parameter | 446859 | $\mathbf{4 5 8 6 7 7}$ | Drinking Water Quality |
| :---: | :---: | :---: | :---: |
| Lab pH | $\mathrm{N} / \mathrm{A}$ | $\mathrm{N} / \mathrm{A}$ |  |
| Lab Electrical | 581 | 779 | $6.5-8.5$ |
| Conductivity |  |  |  |
| Caicium | 56 | 80 | 200 |
| Magnesium | 10 | 16 | 0.3 |
| Sodium | 63 | 78 | 0.05 |
| Potassium | 1 | 2 | 250 |
| Iron | $<1$ | 2 | 1.5 |
| Manganese | $\mathrm{N} / \mathrm{A}$ | $\mathrm{N} / \mathrm{A}$ | 10 |
| Chloride | 3 | $<1$ | 500 |
| Fluorlde | $<1$ | $\mathrm{~N} / \mathrm{A}$ |  |
| Nitrate | $\mathrm{N} / \mathrm{A}$ | 22 | 500 |
| Sulfate | $<1$ | 496 | 443 |
| Bicarbonate | 346 |  |  |
| Total Dissolved Solids | 315 |  |  |

All results in me/L except sonductivity in $\mu \mathrm{s} / \mathrm{cm}$ and pH in $p h$ units. $N / A=$ Not Analyzed

The results show that the groundwater from both wells within the aquifer are a sodium bicarbonate type water with total dissolved solids concentration of less than $500 \mathrm{mg} / \mathrm{L}$ and should generally be acceptable for drinking water use.

It is recommended that water samples be collected and analyzed for routine dissolved salts and bacterial parameters from each well prior to use as a potable supply.

## Alberta．

Reconnaissance Report
View in Imperial
Export to Excel

| Wellid | LCD | Ste | TWP | BCI | M | DRHLHya COMPANY | contle | $0$ | Thtor Worx | U5t | GiM | L7 | ¢F | Wric owntr | 5Tatic Lever （m） | $\left\lvert\, \begin{gathered} \text { TSI } \\ \text { BATE } \\ \text { (Limin) } \end{gathered}\right.$ | SC DRAY （m） |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| teasab | Nw | 41 | （6）${ }^{\text {1 }}$ | b1 | 5 | nrind motinn gillite． Im | 1984－1025 | 40.67 | New whll | Dometic |  | 2 |  | Htust，SORDON | 3.15 | 29.55 | 11.41 |
| － 4154 | Mw | 31 | 0） | （1） | 5 |  10. | 19006\％ | 12180 | New whil | Domene |  | 5 |  | Milf，crotow | 5.4 | 45.4 | 0.00 |
| 50202 | NW | 21 | 051 | 01 | 5 | GEMLD NCOSNE DRLLING 17. | 2904－6－15 | 46.94 | New Wel | Soweibc |  | 15 |  | Muse（cumot | 4.2 | 34.10 | 11.41 |
| 43506 | NE | II | 51 | 1 | 5 | EGGEON DRUUNE LTD． | 195－05－05 | 57.91 | New Well | Dunetk |  | 8 |  | STEFFER CLAYTOA | 1372 | 22.73 | 1412 |
| 40203 | MC． | 31 | 53 | 1 | 5 | URavowtoraurccompl |  | 3291 | rea wal <br> Aburdicel | triesomer |  | 1 |  | SITHER，MOECK |  |  |  |
| 420\％ | 相 | II | 053 | is | 5 | Unasewhy praith |  | 1219 | Onmoy | Doneitc | 1 |  |  | Whacoussi， 2 | 9.45 |  | 80 |
| Peoth | M | 31 | （0at | bi | 5 |  110. | 1928 1024 | 5） 40 | New wel | Donesic | 1 | 8 |  | STEFFIEM IOUG | 0.50 | 11.57 | 11．46 |
| ALP11 | 12 | 31 | （es） | 14 | 5 | MaNewn medith |  | $6 \times 18$ | Chmier | Hicivat | 1 |  |  | simbra 0 | S6in |  | ace |
| 48213 | ME： | 31 | 081 | \％ | 5 | GNUD NCENK LOLILEG LTD． | 19／9－04－12 | 128．24 | Nesw whil | Dominac |  | 24 |  | Stiftek，poug | 45.72 | 6．82 | 11.43 |
| 458514 | 3F | 31 |  | 01 | 5 | CHOUD MCCNW DMHLINE： （70）： | 190\％ 10.13 | mass | Naw Whil | Donetic |  | 10 |  | gacruary Lou | 20.38 | 1599 | 11．4 |
| 450512 | Dit | 4 | 031 | （1） | 5 |  113． | feecobli | Ntoi | Now wira | Doveux |  | 5 |  | SHIFHECMANON | 000 | 15.91 | it 58 |
| 45026 | ME | 31 | 053 | 11 | 5 |  | 1971－12－29 | 96.10 | New Wed | bomeric |  | 11 |  |  |  |  | 11.28 |
| 458287 | m | 31 | （835 | （1） | 5 | Masew | 19月年－12 | स2\％ | gemeary | Doneatic | 1 |  |  | crexelin． |  |  | 0.00 |
| 403904 | SF． | 11 | 053 | 04 | 5 | DAD WiATER WFL ORALEG B SSEVICING LTD | 1005 07． 19 | 87.75 | New Whl | Domeatic |  | 15 | 16 | GCSCL LECNUAD | 4359 | 02.92 | 15.24 |
| 425021 | 85 | 04 | 084 | 12 | 5 | nouco drallang | 1200－20－14 | 12.52 | New whal | Donesuc |  | 11 | 7 | cule leo | 15.45 | 20.31 | 15.24 |
| 498028 | NW | 31 | 089 | 81 | 5 | DAD WATER WLL DAELINGA इovictaciti． | 2000－09－25 | 23.16 | Neew Whas | Domedic |  | 3 |  | STHILE，FOAERT | 12.19 | 68． 19 | 15.24 |
| 1715115 | ME | 31 | $053:$ | 01 | 5 |  | $2000-1023$ | 62.48 | New Wrell | Demestic |  | 6 | 25 | Eewncer emona | 1500 | 18.18 | 45.24 |
| 1715141 | Aw | 6 | 54 | 1 | 5 | SJMFERS DHLIINELTE． | 2015－27－11 | 45.24 | Meaoneter | Moritove |  | 3 |  | Latarce Camesh itic： |  |  |  |
| 12 ELH | M W | 6 | \＄4 | 1 | 5 |  | 2015．0211 | S．45 | Prometier | Monitnery |  | 3 |  | LAFNİE CNMOA ITC |  |  |  |
| 1715145 | NW | 6 | 54. | 1 | 5 | SEPERS OASLITICITD． | 2015－02－11 | 792 | Prexameler | Monitory |  | 3 |  | LAFAGE CMACA TIC |  |  |  |

## Aberta：

Reconnaissance Report
View in Imperial
Export to Excel

| Weil 10 | 150 | 5 Sc | TWP | सदE | M | Brriung company |  | $\begin{aligned} & \text { orph } \\ & (\mathrm{m}) \end{aligned}$ | $\begin{gathered} \text { wee or } \\ \text { woink } \end{gathered}$ | USE | CHM | 17 | W | WELI OWME | STatic IEVEI （m） | $\begin{aligned} & \text { TEST } \\ & \text { GATE } \\ & \text { (Mimig) } \end{aligned}$ | $\begin{aligned} & \text { scorng } \\ & \hline(\mathrm{m}) \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 42905 | Nw | 11 | 058 | ai | 1 | GEMD HCOEW DRLLDG tob． | 18741028 | 10.6 | Nem Well | Donnatic |  | 1 |  | MEER CORDON | 335 | 28.53 | ［12．4］ |
| tmest |  | 3 | （5） | tit | 1 |  tro． | 1m006es | 12．50 | Niw wit | Danest |  | 5 |  | MUEX，corcow | $5 *$ | 45.48 | 0.50 |
| sereat |  | 3 | ess | 01 | 3 |  t70． | 194日年 15 | 6.94 | new wiel | foretr |  | 15 |  | relat cmation | 4.27 | 34.10 | tids |
| Hemast | ＊ | 3 | 53 | ： | 5 |  | 1980\％ | 97815 | New yet | bonetic |  | － |  | Stutite catios | 132 | 220 | thtiz |
| swemit | N\％ | 3 | 89 | $\pm$ | 5 | Lawowncril Mccompli |  | 87.81 | Oid wis Abxiderel | Unemien |  | 1 |  | Steplea mokest |  |  |  |
| 48003 |  | 4 | 985 | 1 t | 5 | （saremin critit |  | 12 al 19 | Oheraty | tineve | 1 |  |  | winceoweo， 2 | 205 |  | 0.05 |
| 4sesic | Ne | H | 051 | 01 | 5 | GRND MCOEW DRLLINO ITD． | 1076－10．24 | 40．10 | Nim Wel | Donnetk | 1 | 1 |  | Steruk boua | 0.00 | 117 | 12．4） |
| 97845 | ne | 3 | （6） | ii | 5 | Inosonnicelitar |  | 4． 15 | Cuentry | Bratt | 1 |  |  | STHER， | S6081 |  | 0.00 |
| 438513 | ne | 31 | 688 | 01 | 5 | GOND MCGUN De Lillas 1 To． | 19m＋04 12 | 129.74 | Nen well | Doresk |  | 24 |  | Stime 00us | 4572 | $5 \times 2$ | 11.4 |
| $4 \times 9514$ | nt | д | 058 | 61 | 5 | GERNO MCLEN DKHLDM tro． | 190\％10－12 | 40.60 | now welt | OOnestic |  | to |  | gacuare iow | 2430 | 1593 | H2at |
| 4exis | ne | 3 | （0） | 9 | 5 | GELUD HCCDN DALLID： ITD． | 15460834 | 28.04 | Niw Wel | Donmeik |  | $\pm$ |  | STEPR Qavtow | 0.00 | 15.91 | 11.51 |
| \＄4xisis | ＊ | 3 | （6） | tr | 5 | mcuary priung co ith | $1973-224$ | ＊ 6.4 | Now welt | Dometa |  | t |  |  |  |  | 11.58 |
| 489512 | （a） | 24 | （9） | 01 | 3 | GPancwitwluek | 147e9s－17 | 71．25 | Centur | Corest | 1 |  |  | conese |  |  | 0.00 |
| tanas | Ne | 31 | 60 | 01 | 5 | DSD WATE WAL ORULING S Tavickitin． | 1995－0\％－19 | 17．28 | New Wel | Domeste |  | 10 |  | 16 cosk，［䒑олar | 4139 | segs | 15.24 |
| H2sen |  | \％f | （est | d） | 4 | nocop prawn | $150000 \cdot 14$ | 72，00 | How wert | Doninit |  | 11 |  | 7 diec， 40 | 1504 | entit | 1534 |
| fexas | NW | 31 | （6） | ai | 5 |  sevacing ITD． | 20000\％ 5 | 23.16 | New wel | Donets |  | 3 |  | 10 Stuflre noviet | 12.9 | 6.619 | 15.24 |
| 22isus |  | 31 | 2038 | 01 | 5 | Sunmers oraume lto． | 2007027 | 62．at | New well | Domet |  | 6 |  |  | 150 | 118.18 | 15．74 |
| 1／2614］ | N4 | 6 | St | 1 | 5 | Sumer orilimitis | E015 00.11 | 15.24 | nemmener | Montomey |  | 3 |  |  |  |  |  |
| 1716145 | Nw | 5 | 54 | 1 | 5 | SMMEAS ORLINE Itr． | 2015－02－11 |  | Anonetrer | Mertorny |  | 3 |  | InAmge CIMADA SC |  |  |  |
| 274645 | Nw | 6 | 3 | 1 | 5 | Sumber craumg ith | 2015－02－15 |  | Netometer： | Montorics |  | 3 |  | INANCS Catich inc |  |  |  |

## Alberta:

Reconnaissance Report
View in Imperial
Export to Excel

| Wellig | LSD | StC | Tw | mat |  | Deritung compant | $\begin{aligned} & \text { sant } \\ & \text { cositime } \end{aligned}$ | bedm | $\begin{aligned} & \text { met or } \\ & \text { woos } \end{aligned}$ | US1 | Cim | H | Ft | wrus ownth | STATE cevir (m) | $\begin{gathered} \text { TIST } \\ \text { nite } \\ \text { nimin) } \end{gathered}$ | $\begin{aligned} & \text { CCing blat } \\ & (\mathrm{m}) \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| S0802 | nw | 21 | 053 | 08 | 5 | Geshe magnes braysa L० | 1974-10.75 | 1067 | hew well | Domeste |  | 2 |  | MUEQ CORCOH | 3.35 | 28.5s | H.43 |
| Stuos | nw | 3 | cas | a | 3 | Hetraid hotan prouivo 110 | 1900m | 1200 | New Wet | Domest |  | 5 |  | Muts, CORDOH | 5.99 | $45 \%$ | a0m |
| stastr | *m | 31 | का | $\pi$ | 5 | GEND MCGEN DRAMEG บ०. | 19900015 | 469 | Now wat | Domete |  | 15 |  | Mailer, condor | 477 | 54.10 | Had |
| +3905 | M | 31 | 53 | 1 | 5 | Hig How oriling iti | 195-030 | 5797 | now wel | Domeice |  | B |  | Stiter Camot | 137 | 22.7 | Wil |
| 430508 | , ${ }^{\text {a }}$ | 34 | 53 | 1 | 5 | uncroweraumicompl |  | 57.31 | cad wes Nuandored | Inenom |  | 1 |  | Sterter noker |  |  |  |
| 46950 | M | 17 | का | \%f | 5 |  |  | 1315 | Cumblir | Diventx | 1 |  |  | Weiciensic, 2 | 2.45 |  | Oto |
| teps10 | n | 31 | 068 | 0 | 5 |  10. | 1978-5024 | 16.60 | now wed | oomesic | 1 | E |  | STFure coik. | 0.00 | 11.3 | 11.4) |
|  | \% | 31 | (3) | \%1. | 5 | Caxemer Deciles |  | (0x) | Oferviy | burcte | 1 |  |  | Stumee 0 | Sta |  | 008 |
| 450511 | N2 | 1 | © 61 | 04 | 5 | Gemo mCENN OLILIEG: ITO | 19990-12 | 120.24 | naw wel | Dosete |  | 24 |  | Stemiel Dovic | 45.72 | 68. | 14.4 |
| sashit |  | 31 | (0) | 9 | 5 |  GD: | 1999-10-13 | 6066 | now well | Doseare |  | 10 |  | Guandel lot | 2030 | 1551 | It.ts |
| Eans |  | 3 T | (4) | 0 | 5 | CEMD ACGON DRIUIN: 110. | 190e 0 Ont | 2304 | tow wed | Dowine |  | * |  | stemer carror | 000 | 15.\% | 11.58 |
| trinde | M | 31 | (6) | 01 | 5 | MCuki druink cr. LTD. | 2877 4278 | tris 10 | 7wims | Doseric. |  | 11 |  |  |  |  | 11: |
| seam | al | 34 | los | bi | 15 | Herown cealife | 89940-17 | 1023 | Oemiary | Deneine | 1 |  |  | Cowers, L. $\mathrm{D}^{\text {a }}$ |  |  | acom |
| +23404 |  | 31 | 65 | 01 | 3 | DED WATE WEL DRELMO A Shnctit | 19\%5-018.5 | B7.78 | Mew Wel | Domenk |  | 10 |  | 6 COSCL LECNAP | 4359 | w. 68 | 15.24 |
| 49883 |  | 01 | Cs | 02 | 3 | ncoco draung | $109000 \cdot 14$ | 92.nt | Sisw What | Dormetic |  | 11 |  | 7 cuee to | 准䉼 | 4051 | 15.24 |
| sexpa |  | 3 S | 000 | Q1 | 5 | os water wel offlumas sexicicizim | 2000.0405 | 23.16 | Now Wed | Donestic |  | 3 |  | \% stemer, mower | 1219 | 68.19 | 1524 |
| 17513 |  | 31 | cost | 01 | 5 | sumers dinuma lto. | 2000-10-21 | 62, 41 | Naw wel | Donemic: |  | 6 |  | Is neuncer, fowno | 1600 | 18, 18 | 15.84 |
| 127516 | sw | 6 | 54 | 1 | 5 | SmPEES DALINKi | 2015 -12-11 | 1524 | Mertmeter | Mentorvo |  | 3 |  | lonnce CNINSA DC. |  |  |  |
| 176541 |  | 6 | 54 | 1 | 5 | Sumpers drlung ito. | 2015-02-11 | 9.45 | Nameter | Montiong |  | 3 |  | WFARCE CNUOA MC |  |  |  |
| 1215145 |  | 6 | 54 | 1 | 5 | Symers onalikg iti. | 2015-02-11 | 752 | Reumider | Morituring |  | 3 |  | LAFABCE CMADA INC |  |  |  |








| Conimitar Cenimation |  |  |
| :---: | :---: | :---: |
|  UNEKNOWN NA ORLLER | Ceriflation $/$ in <br> $\uparrow$ |  |
| fivchety Name RODCO CFHLINS | Civir ar whet mpotamenfect bo dether | Deis anpowat netrer igied |




| Formation Loc |  | Mendinemeet in Metric. |
| :---: | :---: | :---: |
| Degot from ground inve (m) | Wury Eetring | Utekrey Desoriocem |
| 7.32 |  | Brwneh Yatiow TET |
| 1786 |  | Bue Goy CHy |
| 317 |  | Sun Jiny Tes mick |
| 3.1.2 |  | Gemel |
| $40 . \mathrm{M}$ |  | Grom Solt Srut |
| 44.as |  | Gray Medlum Grained Sinsurse |
| 56.08 |  | Trien STeir |
| 54.31 |  | Gray Meduan Grained Sandura |
| 60.45 |  | Green five |
| 71.93 |  | Grur Medion Sorned 5mionver |
| 15.98 |  | Green tase |
| 28.30 |  | Gay Mefian Granet Smaituce |
| 29.75 |  | Civen Stue |



Contractor Certificalion

MNDY AIELNGEA
Cameany Nivery
CACINR DALLINELTO

## Cemfration Ne

## 45840



# Water Well Drilling Report 




View in Imperial Export to Excel GKC wal in

116200 OoA Whal Tmotio Dubing Compary Wall io thate Roport Recenived




Waiar Divatiod far Driling
Water Baime
Anhuit Trave
Civeracta Bate 4 Nive




Watar Dhwhod 9 Driling
Watw Save
Aneurt trake
Duersion Duts 4 Time

Contractor Centificsson

|  RANOV REDUNGES | Certikaitan /at Abse |  |
| :---: | :---: | :---: |
| Comwary Nare CAMper dALLi*N LTD | Coby ef Whill most provideof to owier |  |

## $45 / 40$



## Albertar CHEMICAL ANALYSIS REPORT

| WELL NAME LOCATION | WADOELL, PAT |  | TWp | $053$ | RG | 62 | M 5 |  | GIC WELL IO SAMPLE NO. | $\begin{aligned} & 458077 \\ & 52418 \end{aligned}$ | H |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Lso NW | SEC 36 |  |  |  |  |  |  |  |  |  |  |
| WELL DEPTH | 124.00 | A |  |  |  |  |  |  | WATER LEVEL |  |  |  |
| AQUFER |  |  |  |  |  |  |  |  | LABORATORY | $A E$ |  |  |
| SAMPLING DATE | 1985-05-72 |  |  |  |  |  |  |  |  |  |  |  |
| PIELD |  |  |  |  | MGIL |  |  |  | FHELD |  |  | mal |
| BICARACNIATE |  |  |  |  |  |  |  |  | CAABCNLATE |  |  |  |
| GmLORIDE |  |  |  |  |  |  |  |  | CONOUCTIVITY |  |  |  |
| DISSCLVED OXYGEN |  |  |  |  |  |  |  |  | EH |  |  |  |
| reow |  |  |  |  |  |  |  |  | MANGANLSE |  |  |  |
| PH |  |  |  |  |  |  |  |  | SURPHATE |  |  |  |
| S2 |  |  |  |  |  |  |  |  | TEMPERATURE(C) |  |  | 0 |
| TOYAL ackalinity |  |  |  |  |  |  |  |  | TOTAL HATONESS |  |  |  |
| LABORATORY |  |  |  |  |  |  |  |  | Anslysis Date 1 | 1985.08.04 |  |  |
| COO |  |  |  |  |  |  |  |  | CONDUCTIVIT |  |  | 729 |
| Dic |  |  |  |  |  |  |  |  | PLUORDE |  |  | 0.1000 |
| ION BALANCE |  |  |  |  | 1.0100 |  |  |  | PH |  |  | 790 |
| 548 |  |  |  |  |  |  |  |  | 3102 |  |  | 139000 |
| TOTAL Alkalinuty |  |  |  |  | 7.0000 |  |  |  | TC |  |  |  |
| TDS |  |  |  |  | 443 |  |  |  | TN |  |  |  |
| DOG <br> AMMONULM.N |  |  |  |  |  |  |  |  | BiCARBONATE |  |  | 456.0498 |
| CALCIUM |  |  |  |  | Pvie? |  |  |  | CARBONATE |  |  |  |
| CHMOMIE |  |  |  |  | 10051 |  |  |  | MACNESTUM |  |  | 160135 |
| NITRATEN |  |  |  |  |  |  |  |  | NITRITEN |  |  | -00504 |
| PHOSPHATE |  |  |  |  |  |  |  |  | POTASSIUM |  |  | +,9440 |
| 500ILM |  |  |  |  | 71999 |  |  |  | FULPHATE |  |  | 720095 |
| $\mathrm{NO} 2+\mathrm{NO} 3$ |  |  |  |  | 00604 |  |  |  | TOTAL HARONESS |  |  | 266.0000 |
| Aluminum |  |  |  |  |  |  |  |  | ARSENIC |  |  |  |
| GAFUMM |  |  |  |  |  |  |  |  | BERTLLIM |  |  |  |
| CADMIUM |  |  |  |  |  |  |  |  | CHROMIUA |  |  |  |
| cosalt |  |  |  |  |  |  |  |  | COPPER |  |  |  |
| IRON |  |  |  |  | 2.1400 |  |  |  | LEAD |  |  |  |
| MMVGANESE |  |  |  |  |  |  |  |  | MERCURY |  |  |  |
| Moivboenum |  |  |  |  |  |  |  |  | NICNEL |  |  |  |
| SElentum |  |  |  |  |  |  |  |  | Strontium |  |  |  |
| VANACILIM |  |  |  |  |  |  |  |  | ZINC |  |  |  |
| IWMAIDCARBSNIS |  |  |  |  |  |  |  |  | Pesticanes |  |  |  |
| Premolics |  |  |  |  |  |  |  |  |  |  |  |  |

## Remasks:

## MATLIRF OF ALK AND MARONFSS IS CACOA



Note: this data may nol be fully shecked. The Province disclaims all responsibility for its acsuracy

## Albertar CHEMICAL ANALYSIS REPORT

| WELL NAME LOCATION | FOSSUM. PAIGE |  | TWP | $054$ |  | 02 |  |  | GIC WELL ID SAMPLE NO. | $\begin{aligned} & 440159 \\ & 7956 \end{aligned}$ | \# |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LSD SW | SEC of |  |  |  |  |  |  |  |  |  |  |
| WELL DEPTH | 13000 | $\pi$ |  |  |  |  |  |  | Water level. |  |  |  |
| AQUIFER |  |  |  |  |  |  |  |  | LABORATORY | AE |  |  |
| SAMPLING DATE | 1978-c7-26 |  |  |  |  |  |  |  |  |  |  |  |
| Fielo |  |  |  |  | MGIL |  |  |  | FIELO |  |  | Man |
| BICARHORATE |  |  |  |  |  |  |  |  | CARBONATE |  |  |  |
| Cheorice |  |  |  |  |  |  |  |  | CONDUCTIVITY |  |  |  |
| DISSOLVED OXYGEN |  |  |  |  |  |  |  |  | EH |  |  |  |
| HRON |  |  |  |  |  |  |  |  | MANGANESE |  |  |  |
| FH |  |  |  |  |  |  |  |  | SULPHATE |  |  |  |
| 52 |  |  |  |  |  |  |  |  | TEMPERATURE(C) |  |  | 0 |
| TOTALALKALEITIT |  |  |  |  |  |  |  |  | TOTAL HARONESS |  |  |  |
| LABORATORY |  |  |  |  |  |  |  |  | Anaysk Dite 1 | 1778-06-10 |  |  |
| coo |  |  |  |  |  |  |  |  | CONDUCTNTY |  |  | 581 |
| DIC |  |  |  |  |  |  |  |  | Fluoride |  |  | 0.8800 |
| IONANANCE |  |  |  |  | 1.0700 |  |  |  | PH |  |  | B 40 |
| SAR |  |  |  |  |  |  |  |  | \$102 |  |  | 139000 |
| TOTAL ALKALINITY |  |  |  |  | 4.0000 |  |  |  | TC |  |  |  |
| T05 |  |  |  |  | 315 |  |  |  | TN |  |  |  |
| D00 AUMOTiUMAN |  |  |  |  |  |  |  |  | BICARBONATE |  |  | 3660357 |
| CALCIUM |  |  |  |  | 55.9998 |  |  |  | CARBONATE |  |  |  |
| CHLORIDE |  |  |  |  | 30033 |  |  |  | MAGNESIUM |  |  | 100077 |
| NITRATEN |  |  |  |  |  |  |  |  | NITRTE-N |  |  | -0.0594 |
| PHOSPHATE |  |  |  |  |  |  |  |  | POTAssium |  |  | 12289 |
| SODium |  |  |  |  | 12.999 |  |  |  | SULPFATE |  |  | 210.014 |
| NO2 + NO3 |  |  |  |  | 0.2702 |  |  |  | TOTAL HARDNESS |  |  | 18.0000 |
| Alumpsum |  |  |  |  |  |  |  |  | Arsenic |  |  |  |
| BARILIM |  |  |  |  |  |  |  |  | BERYLuM |  |  |  |
| CADMILM |  |  |  |  |  |  |  |  | CHROMEM |  |  |  |
| cosalt |  |  |  |  |  |  |  |  | COPPER |  |  |  |
| iron |  |  |  |  | 00800 |  |  |  | LEAC |  |  |  |
| MANGANESE |  |  |  |  |  |  |  |  | Mercury |  |  |  |
| MOLyboenum |  |  |  |  |  |  |  |  | NICKEL |  |  |  |
| selenmum |  |  |  |  |  |  |  |  | STROONTLUA |  |  |  |
| VANADIUM |  |  |  |  |  |  |  |  | zinc |  |  |  |
| HVOROCARACOSS |  |  |  |  |  |  |  |  | pesticioes |  |  |  |
| Prevorics |  |  |  |  |  |  |  |  |  |  |  |  |

## Remarks:

| FH | - Oridmtion-fledtuction Proternal | SAR | - Sodium Adsorption Hatia | Dic. | " Dissmived tnorgante Carbon |
| :---: | :---: | :---: | :---: | :---: | :---: |
| cod | *Chemical Oxyjen Demand | $\infty \times 0$ | - Dissolves Organic Carbon | Tsis | - Torai Particulate Nerogen |
| TDS | - Total Dissolved Solids | TC | - Total Particulate Carben |  |  |

Note: this data may met be fully shecked. The Province disclaims all responsibrity for its accuracy

# "PIT REGISTRATION APPLICATION -YEOMAN/PARKER PIT" 

S.E. 1-54-2-5

PREPARED BY

ASPEN LAND GROUP INC..

## APPENDIX 'E’

## Pit Registration Application Yeoman/Parker Pit

October 2013
(Revised December 2013)


Prepared For:

## Westrock Aggregates Ltd.

Prepared by:

## Executive Summary

For many years sand and gravel mining operations have been ongoing within the E 1-54-2W5M. Based on a review of historical aerial photographs, mining operations began within the northern portion of the NE 1-54-2-W5M in the 70's. Mining operations then continued southward across the majority of the quarter and eventually into the northwest corner of the SE 1-54-2-W5M until the early 90's.

Although the history is somewhat unclear, it would appear that the pit was initially opened up and operated by Yeoman Aggregates Ltd. (Yeoman). In the late 80's the pit was then taken over by Westrock Aggregates Ltd. (Westrock). Approvals for the pit had been acquired under the former Land Surface Conservation and Reclamation Act (LSCRA) in 1986 and again in 1990. In 1990 a revised application was submitted under LSCRA and in 1997 an application was submitted under the Environmental Protection and Enhancement Act (EPEA). As neither of these applications received approval, mining operations were discontinued in the early to mid 90 's and the pit has been inactive since that time.

Given that Westrock now plan to reclaim the pit, Aspen Land Group Inc. was retained to prepare a focused registration application that will allow Alberta Environment and Sustainable Resource Development (ESRD) to register the pit under the Code of Practice for Pits (the Code) for reclamation purposes.

## Table of Contents

## Executive Summary

1.0 Introduction. ..... 1
1.1 Overview ..... 1
1.2 Land Ownership ..... 1
2.0 Existing Conditions ..... 1
2.1 History ..... 1
2.2 Current and Adjacent Land Use Activities ..... 2
2.3 Topography and Surface Drainage ..... 3
2.4 Soils ..... 3
2.5 Land Capability for Agriculture ..... 4
2.6 Surficial Geology ..... 4
2.7 Stratigraphy ..... 5
2.8 Groundwater ..... 5
2.9 Vegetation and Wildlife ..... 5
3.0 Reclamation Plan ..... 5
3.1 Current Development ..... 5
3.2 Pit Access ..... 6
3.3 Property Line Buffers, Pit Faces and Extraction Setbacks ..... 6
3.4 Measures to Control Dust, Wind and Water Erosion ..... 6
3.5 Contouring and Soil Replacement .....  6
3.6 Revegetation. ..... 7
3.7 Importation of Reclamation Materials Handing Plan. ..... 7
3.8 Environmental Management Plan .....  8
4.0 Financial Security .....  8
5.0 Limitations ..... 10

## List of Appendices

Appendix A: Certificate of Titles
Appendix B: Schedules under the Code of Practice for Pits
Appendix C: Conservation and Reclamation Plans

### 1.0 Introduction

### 1.1 Overview

Westrock Aggregates Ltd. (Westrock) is proposing to finish reclaiming the 56.29 ha disturbance associated with the sand and gravel pit located in the E 1-54-2-W5M. The pit is located along the west side of Range Road 20 approximately 5 km east of Hwy. 43 as shown on Drawing No. 1. Pit operations will include the removal of all remaining stockpiled product, the importation of topsoil and overburden material, and reclamation of the current disturbance.

The purpose of this report is to provide information required by Alberta Environment and Sustainable Resource Development (ESRD) to register this pit under the Code of Practice for Pits (the Code) for the sole purpose of reclaiming the pit. The schedules required as part of the registration process are included in Appendix B.

### 1.2 Land Ownership

Westrock currently owns the NE 1-54-2-W5M while Charles and Melanie Parker currently own the SE 1-54-2-W5M. Copies of the Certificate of Titles are included in Appendix A.

### 2.0 Existing Conditions

### 2.1 History

Based on a review of historical aerial photographs, mining within the NE 1-54-2-W5M commenced sometime between the 1973 and 1977 as shown on Drawing No. 4. On the 1977 aerial photograph it is evident that mining activities had begun to take place within the northern portion of the quarter and that an area of approximately 8.5 ha had been disturbed. As shown on the 1981 aerial photograph included on Drawing No. 4, this active mining area had expanded southward and that approximately 24.67 ha had been disturbed.

A Sand and Gravel Pit Inventory Report that was conducted by ESRD (formerly Alberta Environment) in 1985 indicated that 70 to 80 acres had been disturbed with approximately 40 acres of reserves remaining to be mined. Some reclamation was ongoing in terms of overburden placement while the older area within the eastern portion of the pit was beginning to re-vegetate itself naturally. Topsoil thickness was reported to be 6 to 8 inches with overburden varying from 10 to 25 feet.

In 1986, Yeoman Construction Ltd. (Yeoman) submitted an application for a Development and Reclamation Approval under the Land Surface Conservation and Reclamation Act (LSCRA). A five year approval was issued in March of 1986 (Approval No. SG-39-86) that sanctioned a 10 acre parcel out of a 120 acre development. Although it is not entirely clear why only 10 acres were sanctioned; it is assumed that the balance of the disturbed lands may have been considered as a pre-78 disturbance. Financial security was obtained in the amount of $\$ 2,500.00$. It is important to note that in the application submitted in support of
this approval that the topsoil depth was reported at 3 to 5 inches with the overburden depth ranging from 4 to 40 feet.

In the summer of 1989, a site inspection conducted by ESRD revealed a number of compliance issues associated with the pit. When Westrock had taken over the pit, they provided updated applications on October 24, 1989 and on February 9, 1990 in an attempt to address the compliance issues. On December 5, 1990 an approval was issued under LSCRA for a sanctioned area of 20 acres. This sanctioned area included future mining areas to the north of an intermittent drainage course within the SE 1-54-2-W5M as shown on Drawing No. 3. At that time financial security was assessed at $\$ 5,000.00$ but it was never submitted.

On January 24, 1991 Westrock submitted an application under LSCRA to amend their approval to allow mining operations to proceed along the western side of the SE 1-54-2W5M south of the intermittent drainage course as shown on Drawing No. 3. This amendment was never finalized; however, a 1500 mm concrete culvert was installed across the creek and mining commenced within an area south of the creek.

An application for approval was submitted under EPEA on May 26, 1997 that was subsequently revised on September 26, 1997. Although the application was deemed sufficiently complete to initiate the public review process, an approval was never issued.

Pit sizes for the different years/milestones are outlined below:
Extraction area pre-1978 8.5 ha
Extraction area post-1978
24.67 ha

Sanctioned area 1986 LSCRA approval
4.1 ha (10 acres)

Sanctioned area 1990 LSCRA approval
Current/overall disturbed area
8.1 ha (20 acres)
56.29 ha

### 2.2 Current and Adjacent Land Use Activities

The total disturbance associated with pit activities to date is approximately 56.29 ha . As the pit has been relatively inactive for a number of years, some of the disturbed areas are beginning to revegetate themselves. This is especially evident in areas where the overburden material had been re-contoured along the eastern side of the pit within the NE 1-$54-2-W 5 M$. The majority of the disturbed areas where are vegetated with trees and typical understory species along with well established grasses.

Lafarge Canada Inc. (Lafarge) maintains a haul road that extends across the north central part of the NE 1-54-2-W5M in an east/west direction. This road is used to haul product from their pits to the west to their wash plant located on the eastside of Rge. Rd. 20 within the W $6-54-1-W 5 M$. A large maintenance shop and associated buildings are located to the north of the Lafarge haul road. Although located in an area that once formed part of the pit, this shop was previously rented out and will remain in place as an improvement to the end land use.

The majority of the area located to the north of the Lafarge haul road represents a pre-78 disturbance while the area to the south of the Lafarge haul road within the NE 1-54-2-W5M for the most part forms the un-reclaimed pit area.

Apart from the treed area associated with the creek that extends across the northern portion of the SE 1-54-2-W5M and the area that was disturbed as a result of mining activities, the balance of the area within this quarter is under cultivation and is used for agricultural purposes.

Lafarge is currently mining a pit in the NW 1-54-2-W5M. Over the past couple years, Lafarge has mined the aggregate and reclaimed a portion of the common property line between the NE 1 and the NW 1-54-2-W5M as shown on Drawing No. 3. Yellowhead Aggregates (Yellowhead) is currently mining a pit within the SW 1-54-2-W5M. At this time Westrock and Yellowhead are in discussions about mining out the common property boundary between these two quarters, however, an agreement has not been made at this time. In the event that an agreement is reached to mine out this common property line, Westrock will submit an updated activities plan.

As depicted on Drawing No. 2 and 3, there are existing excavations associated with past mining activity currently located to the south within the NE 1-54-2-W5M and to the northwest in the SE 1-54-2-W5M. The remaining disturbed area has been stripped or had overburden material directly placed in low areas over undisturbed land to level out the area and to increase the arable land.

### 2.3 Topography and Surface Drainage

The topography of the area consists of a high relief landform or ridge sloping from west to east. Elevations range from 755 masl to 730 masl. Contours and existing surface drainage patterns are shown on Drawing No. 2.

The undisturbed land and partially reclaimed lands within the NE1-54-2-W5M generally slope from the west to the east or to the northeast. There is an intermittent drainage course crossing the northern part of the SE 1-54-2-W5M that conveys water in an easterly direction to a culvert under Rge. Rd. 20 and eventually to Kilini Creek. Although this intermittent drainage course also conveyed runoff from the lands to the west of the subject property, surrounding aggregate operations to the west have removed the majority of the upstream catchment basin, significantly reducing its natural flow. The majority of the surface runoff from within the SE 1-54-2-W5M drains directly to the east or to the northeast towards the intermittent drainage course located within the northern part of the quarter.

### 2.4 Soils

Based on the Alberta Soils Information Viewer, the soils within the general area of the E 1-54-2-W5M are comprised of three different soil polygons.

Polygon 18958 includes the west half of the SE 1-54-2-W5M and is comprised of well drained soils of the Dark Gray Luvisol and Orthic Gray Luvisol subgroups of the Uncas and

Cooking Lake soil series. Polygon 19049 includes the southwest corner of the NE 1-54-2W5M and is comprised $80 \%$ of very poorly drained soils of the Rego Humic Gleysol subgroup of the Kerensky series and 20\% of well drained soils of the Dark Grey Luvisol subgroup of the Elk Point Series. Polygon 19130 comprises the remaining area of the NE $1-54-2-\mathrm{W} 5 \mathrm{M}$ and is comprised of well drained miscellaneous coarse-textured soils in the black soil zone. This polygon also includes soils with Rego profiles that have highly variable landscape characteristics and soils series distribution that may be the result of past mining activities.

Based on a site assessment conducted on August 30, 2013 no subsoil stockpiles were found within the NE 1-54-2-W5M nor was there any evidence of replaced subsoil in the reclaimed areas. Considering when this part of the pit was operated, salvaging of topsoil and subsoil may have not been stripped to meet current criteria. Therefore, subsoil will not be available for reclamation in the NE 1-54-2-W5M.

As shown Drawing No. 3, topsoil and subsoil was salvaged and stockpiled separately for pit operations within the SE 1-54-02-W5M. The texture of the stockpiled topsoil and subsoil was primarily textured as a sandy loam; however, there were some sample locations that reflected more of a sandy clay loam texture.

### 2.5 Land Capability for Agriculture

The CLI capability for agriculture is rated as class $4 \mathrm{M}(60 \%)$ and O ( $40 \%$ ) within the E 1-54-2-W5M. Soils that are rated as class 4 have severe limitations and have restrictions on the range of crops or require special conservation practices due to moisture limitations. The areas that are rated as O are considered organic soils.

### 2.6 Surficial Geology

Based on the Quarternary Geology, Central Alberta map (I. Shetsen 1990) the subject property is located in an area described as a stagnation moraine which typically exhibits till of uneven thickness with local water sorted material up to 30 m thick. This stagnation moraine also exhibits undulating to hummocky topography reflecting variations in till thickness. In this area, the topography is described as undulating with local relief generally less than 3 m .

Bedrock at the base of the deposit consists of grey feldspathic, clayey sandstone, grey bentonitic mudstone and carbonaceous shale of the Horseshoe Canyon Formation of the Cretaceous age. The bedrock contact is non marine and is often characterized by a concretionary ironstone beds, scattered coal and bentonite beds of variable thickness, minor limestone beds.

### 2.7 Stratigraphy

Drawing No. 6 through 9 depict the current surface conditions. Based on a review of ESRD's file material, the average undisturbed soil depths within the pit area would have been as follows:

| Topsoil | -6 to 8 in $(0.15$ to 0.20 m$)$ |
| :--- | :--- |
| Subsoil | - not available |
| Overburden | -10 to $25 \mathrm{ft}(3.0$ to 7.60 m$)$ |
| Aggregate | - not available |

As outlined in Section 2.4, subsoil was not appropriately salvaged within the NE 1-54-2W5M. Subsoil material that was salvaged in the SE 1-54-2-W5M will be evenly distributed in that part of the pit prior to topsoil placement.

There is limited information available as to where and how much aggregate was removed by the previous Yeoman or Westrock operations. Although no formal aggregate test hole information is available, Westrock has concluded that no economical gravel remains to be mined within the pit. Consequently, as no further aggregate extraction is proposed, the provision of aggregate testing information is not considered required for the proposed reclamation activities.

### 2.8 Groundwater

The previous operations did not appear to have intercepted groundwater. As reclamation is the only activity proposed, groundwater is not expected to be encountered.

### 2.9 Vegetation and Wildlife

The proposed reclamation activities will only impact those areas that have already been previously disturbed and will not result in the loss of any naturally vegetated areas or negatively impact wildlife in the area. Common throughout the Onoway area, considerable tansy was noted onsite along with other noxious weeds such as Canada thistle and yellow toadflax.

### 3.0 Reclamation Plan

### 3.1 Current Development

Although at one time relatively active, as the aggregate resource within the pit has now been depleted Westrock would like to register the current pit disturbance of 56.29 ha for reclamation. All areas south of the Lafarge haul road will be reclaimed while the processing/shop area will be maintained. A portion of the Lafarge haul road from Rge. Rd. 20 to the processing/shop will also remain in order to provide secondary access to the shop. That portion of the haul road that extends from the shop area to the western boundary of the NE 1-54-2-W5M will be reclaimed and integrated with the surrounding landscape. In addition to the portion of the Lafarge haul road that will remain in place, the southern access road developed between 1977 and 1981 off of Rge. Rd. 20 will also remain in place.

In the unlikely event that aggregate material is encountered during reclamation that is economical to mine, an updated activities plan will be submitted prior to extraction.

### 3.2 Pit Access

As shown on Drawing No. 2, access to the pit is off of Twp. Rd. 541 and Rge. Rd. 20 with the pit being located approximately 8 km north of Highway 16.

### 3.3 Property Line Buffers, Pit Faces and Extraction Setbacks

A portion of the common property boundary between the NE and the NW 1-54-2-W5M has been mined out by Lafarge with their mining operations within the NW 1-54-2-W5M. In order to mine out this common property boundary, Lafarge extended their operations by approximately 130 to150 m into the Yeoman Pit. As this mined out common property boundary has now been reclaimed and properly integrated with the adjacent lands, the standard $3: 1$ slopes that are typically constructed adjacent to undisturbed property boundaries are no longer required.

For pit activities located within the SE 1-54-2-W5M, there are stockpiles of reclamation material encroaching within the 3 m undisturbed buffer along the common property between the SE 1 and the SW 1-54-2-W5M. Unless an agreement to mine out this common property boundary can be made between Westrock and Yellowhead as outlined in Section 2.2, the 3 m undisturbed buffer will be reestablished with reclaimed slopes of 3:1 or gentler constructed along the pit face as shown on Drawing No. 8.

The culvert which provides access from the south portion of the SE 1-54-2-W5M to the north portion, will also remain in place as part of the reclaimed landscape.

If the reclamation of this pit should not proceed within the next two years, as an interim measure any areas within the pit that have slopes that are greater than $2: 1$ will be re-sloped to $2: 1$ or gentler to ensure public safety and to avoid the potential loss of reclamation material due to erosion or slumping.

### 3.4 Measures to Control Dust, Wind and Water Erosion

In terms of mitigation measures to control wind and water erosion, all stockpiles of reclamation material will remain vegetated until they are ready to be distributed over the recontoured landscape. Once reclamation material has been put in place, re-vegetation of the area will occur as soon as possible. Surface runoff from the disturbed areas within the active pit area will, as much as possible, be maintained onsite and redirected to existing excavations or other low areas within the pit.

### 3.5 Contouring and Soil Replacement

The objective in terms of reclamation is to ensure that the disturbed lands associated with the pit are reclaimed to an equivalent land capability. Due to the timing difference between the operations within the NE 1 and the SE 1-54-2-W5M, the reclamation objectives will be slightly different while trying to meet equivalent land capability.

## Contouring and Soil Replacement for the SE 1-54-2-W5M (Parker Pit)

The disturbance south of the channel represents approximately 6 ha of the current disturbed area. Slopes along the west property line will be reclaimed to no steeper than 3:1; while the remaining slopes along the pit boundary will be reclaimed to no steeper then 10:1.

That being said, Westrock may have the opportunity to import up to $100,000 \mathrm{~m}^{3}$ of fill material to raise the elevation of this area to match pre-disturbance elevations, as shown on Drawing No. 8 and 9. If Westrock is unable to bring or fill or brings in less than $100,000 \mathrm{~m}^{3}$ of fill, the reclaimed landscape will be to a lower elevation, as shown on Drawing No. 8 and 9. Utilizing material stockpiled within this quarter, the topsoil replacement depth will be approximately 18 cm while the subsoil replacement depth will be approximately 9 cm .

## Contouring and Soil Replacement for the NE 1-54-2-W5M (Yeoman Pit)

Areas where dense volunteer tree vegetation or hayland has developed will remain in place as shown on Drawing No. 3. Areas that are comprised of haphazard excavations and stockpiles of soil material will be leveled out and integrated to develop a harmonious landscape. It is anticipated that the slopes of these reclaimed areas will for the most part be 10:1 or gentler but there may be areas where the slopes are closer to $4: 1$ as conditioned in LSCRA Approval No. SG-39-86. It is anticipated that the land will be reclaimed back to an agricultural end land use (pasture/cultivation) with some treed areas.

As mentioned in Section 3.3, the property line buffer and associated 3:1 slopes are not required along the west portion of the NE1-54-2-W5M given that it has been mined out and reclaimed by Lafarge.

Once the site is re-contoured, all available topsoil that is currently stockpile or exhumed during reclamation activities will be spread evenly over all re-contoured areas. In order to increase agricultural production Westrock may import up to $100,000 \mathrm{~m}^{3}$ of topsoil to the pit. Imported topsoil will only be placed within the reclaimed areas in the NE 1-54-2-W5M.

Prior to the placement of topsoil material, any areas exhibiting compaction will be alleviated.

### 3.6 Revegetation

The reclaimed areas within the NE 1-54-2-W5M will be revegetated using a hayland seed mix while in the SE 1-54-2-W5M a seed mix will be used for the reclaimed area that matches the landowner's current crop rotation.

### 3.7 Importation of Reclamation Materials Handing Plan

As mentioned in Section 3.5, Westrock may import material to augment the reclamation of the pit. It is anticipated that up to $100,000 \mathrm{~m}^{3}$ of topsoil and $100,000 \mathrm{~m}^{3}$ of overburden material could be imported from around the Edmonton area.

All imported material will be from sites with no previous disturbance (i.e., agriculture to residential). An inventory of the material received will be maintained that will include such things as:

- the location where the material was obtained and placed;
- the quantity of the material received; and
- random sample analysis to confirm no contaminants.

As there is sufficient topsoil material in the SE 1-54-2-W5M to properly reclaim the site, no imported topsoil material will be applied to this area. Up to $100,000 \mathrm{~m}^{3}$ of imported overburden fill material will be used to fill the excavation area to reestablish positive drainage to the intermittent creek and avoid ponding of water.

To make up for the lack of topsoil material available for the NE 1-54-2-W5M, imported topsoil material may utilized to establish a topsoil layer to a maximum depth of 30 cm .

Imported material will only be brought onsite when it can be direct placed. No imported material will be stockpiled.

### 3.8 Environmental Management Plan

To minimize the impact on the environment during reclamation, Westrock will implement a number of environmental management practices. These environmental management practices include:

- having spill kits on all equipment used for mining purposes;
- use of double wall fuel storage tanks and/or containment berms;
- properly storing and regularly hauling any industrial waste generated at the pit to an approved municipal or class II landfill;
- properly collecting and regularly hauling all sanitary waste to an approved waste management treatment facility;
- an active weed control program that includes mowing, hand pulling, spot spraying, seeding stockpiles of reclamation material, etc. to prevent the initial establishment of weeds;
- the proper application of herbicides; and
- ensuring that no herbicides, pesticides or any other hazardous substance will be stored onsite.


### 4.0 Financial Security

As Westrock has no future intentions of gravel extraction operations, the reclamation security estimate of $\$ 190,705.46$ represents the maximum conservation and reclamation costs (i.e., full-cost security) to reclaim the remainder of the pit. The main cost associated with the security estimate is the amount of reclamation material that is stockpiled. The maximum area of disturbance that will be secured at third party costs is the current limit area of disturbance less the process site/access roads (4.26 ha) of 52.03 ha.

## Security Estimate

```
Mobilization and Demobilization
\$ 5,000.00
Third Party Costs for the NE 1-54-2-W5M (46.03 ha)
Reclamation Material
Topsoil Material Placement \(-4,863 \mathrm{~m}^{3} @ \$ 2.50 / \mathrm{m}^{3} \quad \$ 12,157.50\)
Overburden Material Placement - 10,314 m3 @ \$2.50/m \({ }^{3}\) \$ 25,785.00
Recontouring
D8R Crawler Tractor (Non-Current) - 110 hrs @ \$264.00/hr \$ 29,040.00
Decompaction
D7H Crawler Tractor with ripper (Non-Current) - 40 hrs @ \$266.25/hr \$ 10,650.00
Seedbed Preparation
Tractor with disc - 20 hrs @ \$125.00/hr \$ 2,500.00
Seeding
Tractor with seed drill/seed - 46.03 ha @ \$135.00/ha \$ 6,214.05
Fertilizing
Tractor with fertilizer attachment - 46.03 ha @ \$135.00/ha \$ 6,214.05
Subtotal \(\$ \mathbf{9 2 , 5 6 0 . 6 0}\)
```

Third Party Costs for the SE 1-54-2-W5M (6.0 ha)
Reclamation Material
Topsoil Material Placement $-11,197 \mathrm{~m}^{3} @ \$ 2.50 / \mathrm{m}^{3}$ \$ 27,992.50
Subsoil Material Placement $-5,775 \mathrm{~m} 3 @ \$ 2.50 / \mathrm{m}^{3}$ \$ 14,437.50
Overburden Material Placement - 10,334 m3 @ \$2.50/m ${ }^{3}$ \$ 25,835.00
Recontouring
D8R Crawler Tractor (Non-Current) - 12 hrs @ \$264.00/hr \$ 3,168.00
Decompaction
D7H Crawler Tractor with ripper (Non-Current) - 8 hrs @ \$266.25/hr \$ 2,130.00
Seedbed Preparation
Tractor with disc - 5 hrs @ \$125.00/hr \$ 625.00
Seeding
Tractor with seed drill/seed -6.0 ha @ \$135.00/ha $\$ 810.00$
Fertilizing
Tractor with fertilizer attachment - 6.0 ha @ \$135.00/ha \$ 810.00
Subtotal $\$ 75,808.00$
$\begin{array}{ll}\text { Total Subtotal at Third Party Costs } & \$ 173,368.60 \\ \text { Administration @ 10 \% of Total Third Party Costs } & \$ 17,336.86 \\ \text { Total Security } & \$ 190,705.46\end{array}$
Rates are based on the Alberta Roadbuilders and Heavy Construction Association 2013 Equipment Rental Rates Guide and Membership Roster in combination with ESRD's standard rate of $\$ 2.50 / \mathrm{m}^{3}$ for the movement of reclamation material.

### 5.0 Limitations

This report has been prepared for the sole benefit of Westrock Aggregates Inc. This document may not be used by any other person or entity without the express written consent of Aspen Land Group Inc. and Westrock Aggregates Inc., with the exception of Alberta Environment and Sustainable Resource Development. Any use of this report by a third party, or any reliance on decisions made based on it, or damages suffered as a result of the use of this report are the sole responsibility of the user.

The information and conclusions contained in this report are based upon work undertaken by trained professional and technical staff in accordance with generally accepted scientific practices current at the time the work was performed. The conclusions and recommendations presented represent the best judgment of Aspen Land Group Inc. based on the data obtained during the assessment. Due to the nature of the assessment and the data available, Aspen Land Group Inc. cannot warrant against undiscovered environmental liabilities. Conclusions and recommendations presented in this report should not be considered legal advice.

Registration Application (Revised December 2013)
Westrock Aggregates Ltd. - Yeoman/Parker Pit
E 1-54-2-W5M - Lac Ste. Anne County

## Appendix A: Certificate of Titles



## LAND TITLE CERTIFICATE

```
S
LINC SHORT LEGAL TITLE NUMBER
0035 230 200 5;2;54;1;SE
122 166 512 +1
LEGAL DESCRIPTION
MERIDIAN 5 RANGE 2 TOWNSHIP 54 SECTION 1 QUARTER SOUTH EAST CONTAINING 64.7 HECTARES (160 ACRES) MORE OR LESS. EXCEPTING THEREOUT :
```

HECTARES (ACRES) MORE OR LESS
$0.809 \quad 2.00$
$1.81 \quad 4.47$
$7.66 \quad 18.93$
LS
B) PLAN 9924788 DESCRIPTIVE
C) PLAN 1222170 SUBDIVISION

SHORT LEGAL
5;2;54;1;SE

TITLE NUMBER 122166512 +1

```
EXCEPTING THEREOUT ALL MINES AND MINERALS
ESTATE: FEE SIMPLE
MUNICIPALITY: LAC STE. ANNE COUNTY
REFERENCE NUMBER: 992 218 766 +1
\begin{tabular}{|c|c|c|c|c|}
\hline \multicolumn{5}{|c|}{REGISTERED OWNER(S)} \\
\hline REGISTRATION & DATE (DMY) & DOCUMENT TYPE & VALUE & CONSIDERATION \\
\hline
\end{tabular}
122166512 29/05/2012 SUBDIVISION PLAN
```

OWNERS
CHARLES HERBERT PARKER

AND
MELANIE LOU PARKER
BOTH OF:
BOX 43, SITE 320, RR \#3
STONY PLAIN
ALBERTA T7Z 1X3
AS JOINT TENANTS
(DATA UPDATED BY: CHANGE OF ADDRESS 122413751)

## PAGE 2

\# 122166512 +1

```
    NUMBER DATE (D/M/Y) PARTICULARS
```

762116447 02/07/1976 UTILITY RIGHT OF WAY
GRANTEE - STE ANNE NATURAL GAS CO-OP LIMITED.
992113698 03/05/1999 CAVEAT
RE : ROAD WIDENING
CAVEATOR - LAC STE. ANNE COUNTY.
BOX 219
SANGUDO
ALBERTA TOE2AO
122166511
29/05/2012 CAVEAT
RE : RESTRICTIVE COVENANT PURSUANT TO MUNICIPAL
GOVERNMENT ACT
CAVEATOR - LAC STE. ANNE COUNTY.
BOX 219
SANGUDO
ALBERTA TOE2AO
122166513 29/05/2012 CAVEAT
RE : DEFERRED RESERVE
CAVEATOR - LAC STE. ANNE COUNTY.
BOX 219
SANGUDO
ALBERTA TOE2AO

TOTAL INSTRUMENTS: 004

THE REGISTRAR OF TITLES CERTIFIES THIS TO BE AN ACCURATE REPRODUCTION OF THE CERTIFICATE OF TITLE REPRESENTED HEREIN THIS 26 DAY OF AUGUST, 2013 AT 02:13 P.M.

ORDER NUMBER: 24248750

CUSTOMER FILE NUMBER:

*END OF CERTIFICATE*

## PAGE 3

\# 122166512 +1

THIS ELECTRONICALLY TRANSMITTED LAND TITLES PRODUCT IS INTENDED FOR THE SOLE USE OF THE ORIGINAL PURCHASER, AND NONE OTHER, SUBJECT TO WHAT IS SET OUT IN THE PARAGRAPH BELOW.

THE ABOVE PROVISIONS DO NOT PROHIBIT THE ORIGINAL PURCHASER FROM INCLUDING THIS UNMODIFIED PRODUCT IN ANY REPORT, OPINION, APPRAISAL OR OTHER ADVICE PREPARED BY THE ORIGINAL PURCHASER AS PART OF THE ORIGINAL PURCHASER APPLYING PROFESSIONAL, CONSULTING OR TECHNICAL EXPERTISE FOR THE BENEFIT OF CLIENT (S).


## S

LINC SHORT LEGA
0011350360 5;2;54;1;N
TITLE NUMBER

LEGAL DESCRIPTION

THE NORTH EAST QUARTER OF SECTION ONE (1)
TOWNSHIP FIFTY FOUR (54)
RANGE TWO (2)
WEST OF THE FIFTH MERIDIAN
CONTAINING 64.7 HECTARES (160 ACRES), MORE OR LESS.
EXCEPTING THEREOUT:
(A) 0.809 HECTARES (2 ACRES), MORE OR LESS, AS SHOWN

ON ROAD PLAN 1583T
(B) ALL THAT PORTION DESCRIBED AS FOLLOWS:

COMMENCING AT A POINT OF THE EASTERLY BOUNDARY OF SAID QUARTER SECTION WHERE THE SOUTHERLY BOUNDARY OF A ROADWAY AS SHOWN ON ROAD PLAN 1583T INTERSECTS THE SAID EASTERLY BOUNDARY THENCE WESTERLY ALONG THE SAID SOUTHERLY BOUNDARY OF SAID ROADWAY A DISTANCE OF THREE HUNDRED AND THIRTY
(330) FEET TO A POINT THENCE SOUTHERLY AND PARALLEL TO THE SAID EASTERLY BOUNDARY A DISTANCE OF FIVE HUNDRED AND TWENTY EIGHT (528) FEET TO A POINT THENCE EASTERLY AND PARALLEL TO THE SAID SOUTHERLY BOUNDARY OF SAID ROADWAY A DISTANCE OF THREE HUNDRED AND THIRTY (330) FEET MORE OR LESS TO A POINT ON THE SAID EASTERLY BOUNDARY THENCE NORTHERLY ALONG SAID EASTERLY BOUNDARY TO THE POINT OF COMMENCEMENT CONTAINING 1. 62 HECTARES ( 4 ACRES), MORE OR LESS.
(C) 0.639 HECTARES (1.58 ACRES), MORE OR LESS, AS SHOWN ON ROAD PLAN 7820078
EXCEPTING THEREOUT ALL MINES AND MINERALS

ESTATE: FEE SIMPLE

MUNICIPALITY: LAC STE. ANNE COUNTY
REFERENCE NUMBER: 792031710

| REGISTERED OWNER(S) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| REGISTRATION | DATE (DMY) | DOCUMENT | TYPE | VALUE | CONSIDERATION |

OWNERS

WESTROCK AGGREGATES LTD.
OF 10410 - 81 AVENUE, EDMONTON
ALBERTA T6E 1X5

ENCUMBRANCES, LIENS \& INTERESTS

REGISTRATION
NUMBER DATE ( $\mathrm{D} / \mathrm{M} / \mathrm{Y}$ ) PARTICULARS

882016255 26/01/1988 MORTGAGE
MORTGAGEE - PROVINCE OF ALBERTA TREASURY BRANCHES.
1764, 8770 - 170 STREET, EDMONTON
ALBERTA T5T4J2
ORIGINAL PRINCIPAL AMOUNT: \$1,000,000
892276385 25/10/1989 MORTGAGE
MORTGAGEE - PROVINCE OF ALBERTA TREASURY BRANCHES.
1764, 8770 - 170 STREET, EDMONTON
ALBERTA T5T4J2
ORIGINAL PRINCIPAL AMOUNT: \$750,000

992299717 05/10/1999 UTILITY RIGHT OF WAY
GRANTEE - STE ANNE NATURAL GAS CO-OP LIMITED.

TOTAL INSTRUMENTS: 003

THE REGISTRAR OF TITLES CERTIFIES THIS TO BE AN ACCURATE REPRODUCTION OF THE CERTIFICATE OF TITLE REPRESENTED HEREIN THIS 26 DAY OF AUGUST, 2013 AT 02:13 P.M.

ORDER NUMBER: 24248750
CUSTOMER FILE NUMBER:


PAGE 3

THIS ELECTRONICALLY TRANSMITTED LAND TITLES PRODUCT IS INTENDED FOR THE SOLE USE OF THE ORIGINAL PURCHASER, AND NONE OTHER, SUBJECT TO WHAT IS SET OUT IN THE PARAGRAPH BELOW.

THE ABOVE PROVISIONS DO NOT PROHIBIT THE ORIGINAL PURCHASER FROM INCLUDING THIS UNMODIFIED PRODUCT IN ANY REPORT, OPINION, APPRAISAL OR OTHER ADVICE PREPARED BY THE ORIGINAL PURCHASER AS PART OF THE ORIGINAL PURCHASER APPLYING PROFESSIONAL, CONSULTING OR TECHNICAL EXPERTISE FOR THE BENEFIT OF CLIENT (S).

Registration Application (Revised December 2013)
Westrock Aggregates Ltd. - Yeoman/Parker Pit
E 1-54-2-W5M - Lac Ste. Anne County

## Appendix B: $\quad$ Schedules under the Code of Practice for Pits

# Code of Practice for Pits <br> Registration Application (Schedule 1) 

Date: October 11, 2013 (Revised December 27, 2013)
Previous Environmental Protection and Enhancement Act Registration Number: SG-39-86
Water Act authorization required? $\square$ Yes $\boxtimes$ No
If Yes, $\square$ application submitted and current Water Act Authorization Numbers: application submitted

Name of Applicant (company or person in whose name the pit will be registered):
Company: Westrock Aggregates Ltd.
Address: $\quad$ 10410-81 Ave Edmonton, AB T6E 1X5
Phone: (780) 434-8555 Facsimile:(780) 438-1390
e-mail: thomasfath@ohanlonpaving.com

Name of Person Submitting Application:
Name: Lesley Foy
Company Name: Aspen Land Group Inc.
Job Title: Senior Agrologist
Address: \#201 18311 - 105 Avenue, Edmonton, Alberta T5S 2K9
Phone: (780) 667-7081 Facsimile: Not Available
e-mail: Ifoy@aspenlandgroup.com

Signature:


## Name of Primary Contact for Pit:

Name:
Dean Maurier
Address: $\quad$ 10410-81 Ave Edmonton, AB T6E 1X5
Phone:
(780) 434-8555

Facsimile: (780) 438-1390
e-mail: dmaurier@fathindustries.com

| Pit Location <br> Municipal Address or <br> LSD-Sec-Twp-Rge-Mer | Registered Owners <br> Name, Address and Phone <br> Number | Occupants <br> Name, Address and <br> Phone Number |
| :---: | :---: | :---: |
| NE 1-54-2-W5M | Westrock Aggregates Ltd. <br> 10410-81 Ave <br> Edmonton AB, T6E 1X5 | N/A |
| SE 1-54-2-W5M | Charles and Melanie Parker <br> Box 43, Site 320, RR\#3 <br> Stony Plain AB, T7Z 1X3 <br> $780-967-5443$ | N/A |

## Activities Plan (Schedule 2)

## Part 1 Information

Aggregate Type (check off all that apply): $\boxtimes$ Gravel $\boxtimes$ Sand $\square$ Clay $\square$ Marl
Current Size of Pit: 56.29 ha
Average Thickness (indicate metres or centimetres for each one):
Topsoil: 6-8 inches*
Subsoil: $\quad$ n/a*
Overburden: 10-25 feet*
Aggregate: $\quad \underline{25-50}$ feet ${ }^{\star}$
*Information obtained from Yeoman Construction Ltd. - June 1985 Sand and Gravel Pit Inventory Report
Topsoil Texture (check all that apply):organic soil $\square$ mineral soil $\square$ clay loamsilty loamsand $\boxtimes$ sandy loamloamclaysilt $\qquad$ other

Description of techniques to prevent wind and water erosion, and to limit the movement of dust from the pit: Refer to Section 3.4 of the report.

Participation in local or regional air monitoring initiative: $\underline{\mathrm{n} / \mathrm{a}}$
Inactive pit conservation and reclamation techniques: All inactive pit slopes will be backsloped to a 2:1 slope.
$\boxtimes$ Scale drawings and cross-sections of existing pit conditions and planned sequence of operation attached.

## Part 2 Information

Maximum planned size of pit: 56.29 ha
Depth to groundwater (metres) in test holes (indicate each depth if multiple holes): $\mathrm{n} / \mathrm{a}$
Planned activities at the pit (check off all that apply): $\square$ wet pit excavation
$\square$ concrete production $\square$ mixing salt and aggregate $\square$ mixing recycled asphalt and concrete with aggregate $\square$ spraying truck boxes $\square$ aggregate washing $\square$ use of alternative materials for reclamation $\boxtimes$ other: reclamation and importing topsoil and overburden material

Mitigative measures for all of the above activities: $\underline{\text { Refer to Section } 3.7 \text { of the report. }}$
Proposed land uses for reclaimed pit (check all appropriate boxes):


Pit water release (rationale for release, techniques and discharge points): n/a

## Yeoman Property

Average topsoil replacement depth (cm): approximately 2 cm
Average subsoil replacement depth (cm): no subsoil available for replacement

## Parker Property

Average topsoil replacement depth (cm): 18 cm
Average subsoil replacement depth (cm): 9 cm
$\boxtimes$ Scale drawings and cross-sections of reclaimed pit conditions attached.

Description of surface water bodies in the reclaimed pit: $\mathrm{n} / \mathrm{a}$
Design: n/a
Intended use: n/a
Water elevations at full supply level: $\mathrm{n} / \mathrm{a}$
Slope of land one metre above and one metre below full supply level: $n / a$
Slope of land one metre below full supply level: $n / a$

Signature and title of person who developed Activities Plan:


## Security Estimate (Schedule 3)

Acres of land secured at $\$ 250$ /acre: $\qquad$ Acres $\times \$ 250=\$$ (a)

| $\boxtimes$ Detailed full-cost security calculation attached | Total full-cost $=\underline{\$ 190,705.46}$ (b) |
| :--- | :--- |
| Area of land at full-cost: $\quad 52.03 \quad$ (hectares/acres) | Cost/hectare $=\underline{\$ 3,665.30}$ |

Total security required ((a) + (b)): \$190,705.46

Previous method of payment: $\boxtimes$ Letter of Credit $\square$ Cash $\square$ Other (explain)

Signature and title of person submitting estimate:


See Section 4.0 of the report for the full-cost security calculation.

Registration Application (Revised December 2013)
Westrock Aggregates Ltd. - Yeoman/Parker Pit
E 1-54-2-W5M - Lac Ste. Anne County

## Appendix C: Conservation and Reclamation Plans







## Cross Section A-A' Existing

Distance ( m )


## Cross Section A-A' Reclaimed

Distance ( m )



## WESTROCK AGGREGATES LTD.

| YEOMAN PARKER PIT <br> (REGISTRATION APPLICATION) |  |
| :---: | :---: |
| CROSS SECTION A-A' EXISTING AND RECLAIMED CONDITIONS |  |
| Checked By, Lfor 86. For | Scale AS SHOWN |
|  | rowing |
| Orome By. T.f.er | 6-11 |
| Air Photo: Na |  |

## Cross Section B-B' Existing

Distance (m)



## Cross Section B-B' Reclaimed

Distance (m)


## WESTROCK AGGREGATES LTD.



## Cross Section C-C' Existing



## Cross Section C-C' Reclaimed

Distance (m)


## WESTROCK AGGREGATES LTD.

YEOMAN PARKER
(REGISTRATION APPLCATION)
EXISTING AND RECLAIMED CONDITIONS


Cross Section D-D' Existing
Distance (m)


SCALE: $\mathrm{H}=1: 3000$

## Cross Section D-D' Reclaimed

Distance (m)


## WESTROCK AGGREGATES LTD.

YEOMAN PARKER PI
CROSS SECTION DD
EXISTING AND RECLAIMED CONDITIONS





[^0]:    
    

[^1]:    No Crown Reservations/Notations Found

