

**LAC STE. ANNE COUNTY PROVINCE
OF ALBERTA
BYLAW # 27-2017**

A BYLAW TO CONTROL LAND USE

WHEREAS, Part 17, Section 633 (1) of the Municipal Government Act, being Chapter M-26.1, Division 5, provides for the purpose of providing a framework for subsequent subdivision and development of an area of land within a municipality, and

AND WHEREAS the Council of Lac Ste. Anne County has decided to consider and adopt the Clearview Ridge Area Structure Plan as a means to facilitate country residential growth in Lac Ste. Anne County.

NOW THEREFORE the Council duly assembled hereby enacts as follows:

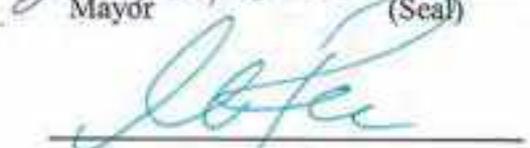
1. That the Clearview Ridge Area Structure Plan attached is hereto and forming part of this bylaw is, hereby adopted.

2. That this Bylaw comes into full force and effect upon third reading of this Bylaw.

First Reading carried the 4th day of October, A.D. 2017

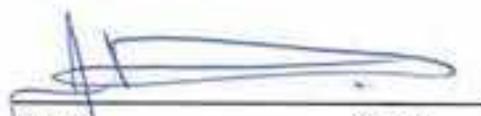


Mayor (Seal)



County Manager (Seal)

Read a second time this 4th day of November, A.D. 2017.



Reeve (Seal)



County Manager (Seal)

Read a third and final time this day of ,A.D. 2017,



Reeve (Seal)



County Manager (Seal)

SCHEDULE "A"
BYLAW NO. 27-2017
Clearview Ridge Area Structure Plan

CLEARVIW RIDGE AREA STRUCTURE PLAN

REVISED March 2018

**CLEARVIEW RIDGE AREA STRUCTURE
PLAN**

Within the

S.E. 1-54-2-5

COUNTY OF LAC ST ANNE

PREPARED FOR

LAUNCH VENTURES LTD.

BY

Robert Wescott, B.Sc. AICP

Wescott Consulting Group Ltd.

Revised March 2018

CLEARVIEW RIDGE AREA STRUCTURE PLAN – FEBRUARY 2018
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Appendices

Appendix 'A'

Biophysical Inventory, S.E. 1-54-2-W5M, January, 2016 prepared by MCA Environmental Management, dated January 2016.

Appendix 'B'

Aggregate Characterization Assessment for Wes Erickson, S.E. 1-54-2-W5M prepared by Twerdoff & Associates Inc., and dated June 2014.

Appendix 'C'

Traffic Impact Assessment, D&A Paulichuk Consultants Ltd. S.E. 1-54-2-W5M dated December 7th, 2015.

Appendix 'D'

Hydrological Assessment for W.E. 1-54-2-W5M prepared by Groundwater Information Technologies Ltd., dated May 6th, 2015.

Appendix 'E'

Pit Registration Application, Yeoman/Parker Pit in the E 1.2 Section 1, Township 54, Range 2, West of the 5th Meridian prepared by Aspen Land Group Inc. dated December 2013.

Appendix 'F'

Clearview Ridge Storm Water Management Plan prepared by Bob Quazi, M.Sc., P.Eng., River Engineering Consulting dated November 18, 2017.

1.0 PURPOSE

The Clearview Ridge Area Structure Plan provides for the orderly and economic approach to the subdivision and development of the lands within that portion of the S.E. ¼ Section 1, Township 54, Range 2, West of the 5th Meridian which lies to the west of Range Road 20 and north of Township Road 540, (commonly referred to as the Heatherdown Road).

The Clearview Ridge Area Structure Plan is intended to identify key issues such as land use, servicing, aesthetic design, transportation network and reserve issues, and to provide viable options in the solution of those issues.

This Area Structure Plan is intended to establish a process of sequencing to ensure that development occurs in a logical, efficient and sequential manner.

1.1 PLAN AREA JURISDICTION

The Plan Area (as shown in Figure 1 Regional Context) is located within the municipal jurisdiction of Lac Ste Anne County. This plan consists of policy statements and conceptual representations that provide the framework to promote the following principles:

- Promote sustainable development;
- Promote the expansion of the tax base within Lac Ste Anne County;
- Acknowledge and promote the development potential of the lands while recognizing the geographical importance within both the local and regional context;
- Create a visually appealing mixed use built environment.

1.2 PLAN COMPLIANCE

This Area Structure Plan hereinafter referred to as the Plan Area', has been prepared as a requirement of Lac Ste Anne County and in particular the Municipal Development Plan # 23-2014 which requires that such plans be prepared for select study areas.

The Area Structure Plan is also prepared in accordance with the requirements as stipulated of the Municipal Government Act. The specific legislation under Section 633 of the MGA enabling the creation of Area Structure Plans states:

- (1) For the purpose of providing a framework for subsequent subdivision and development of an area of land, a council may, by bylaw, adopt an area structure plan.

- (2) An area structure plan
- (a) must describe
- (i) the sequence of development proposed for the area;
 - (ii) the land uses proposed for the area, either generally or with respect to specific parts of the area;
 - (iii) the density of population proposed for the area either generally or with respect to specific parts of the area;
 - (iv) the general location of major transportation routes and public utilities, and
- (b) may contain any other matters the council considers necessary.

1.3 KEY ELEMENTS OF THE PLAN

The two key elements in the Area Structure Plan are:

- A process that is structured to coordinate development intensification in concert with transportation capacities, environmental compatibility and servicing capabilities.
- Policy Statements supplemented with a Land Use Concept Plan to establish a logical and sequential pattern of development.

1.4 POLICY INTERPRETATION

The explanatory text accompanying a policy within the Plan is provided for information purposes only to enhance the understanding of the policy. If an inconsistency arises between this text and a policy, the policy will take precedence.

Where “shall” is used in a policy, the policy is considered mandatory. However, where actual quantities or numerical standards are contained within the policy, such quantities or standards may be varied, provided that the variance is necessary to address unique circumstances that would otherwise render compliance impractical or impossible, and the general intent of the policy is still achieved.

Where “should” is used in a policy, the intent is that the policy is to be complied with. However, the policy may be varied in a specific situation provided that the variance is necessary to address unique circumstances that will otherwise render compliance impractical or impossible, or to introduce an acceptable alternate means to otherwise achieve the general intent of the policy.

1.5 PLAN AMENDMENTS

In order to amend this Plan, including any changes to the text or maps within, an amendment to the Plan will be required to be approved by Bylaw. An amendment will require the holding of a statutory public hearing together with public notification carried out in accordance with procedures established by Lac Ste Anne County.

Where an amendment to the Plan is requested, the applicant will be required to submit supporting information necessary to evaluate and justify the amendment. Such changes will be made from time to time as determined necessary to ensure that the text and maps remain accurate.

1.6 MAP INTERPRETATION

Unless otherwise specified within the Plan, the boundaries or locations of any symbols or areas shown on a map are approximate only, not absolute, and shall be interpreted as such. They are not intended to define exact locations except where they coincide with clearly recognizable physical features or fixed boundaries, such as property lines or road and utility rights-of-way.

1.7 CONSISTENCY AND MONITORING OF THE PLAN

It is intended that consistency between the Plan and any other policy directives which have been approved by Council be maintained, including but not limited to, the Municipal Development Plan # 23-2014 of Lac Ste Anne County.

In order to ensure the Plan remains current and relevant, it will be monitored over time. If any changes are deemed necessary as a result of future monitoring, the Plan will be modified through the amendment process.

2.0 PLAN AREA

2.1 REGIONAL CONTEXT

The Lac Ste Anne County is located at the junction of Highway 2A, Highway 13 and the Canadian Pacific railroad and approximately 12 kms east of Queen Elizabeth II Highway. In addition, the city is approximately 43 miles south of the City of Edmonton and the Edmonton International Airport. Anecdotally, Wetaskiwin as a stage coach stop between Edmonton and Calgary.

The Lac Ste Anne County is the center of a thriving mixed farming and oil and gas region, and is the gateway to some excellent recreational facilities centrally located amongst many lakes and rivers, and close to the foothills.

Based on the 2014 census the Lac Ste Anne County supports a primary trading area of 50,264 people and a secondary trading area population of 93,637.

3.0 EXISTING SITE FEATURES & CHARACTERISTICS

The Plan Area (as shown in Figure 2) consists of approximately 54.47 ha. (134.47Acres) of land intended to accommodate a mixed use development which shall include Industrial / Country Residential / Public Utilities, and Reserve.

3.1 SITE CHARACTERISTICS

The Plan Area as shown in Figure 3 (*Topographical Features*) has varied relief. The plan area varies approximately 25 meters in elevation with the high point located in the north west quadrant of the plan area to the lowest point which is the south east quadrant of the plan area. Within the north quadrant of the parcel and north of the proposed development area is a ravine or channel which severs approximately 9 hectares of land from the remainder of the plan area.

3.2. DRAINAGE BASIN

The plan area is part of the Kilini Creek drainage system and as is the case with most undeveloped parcels of land the plan area is the subject of imperfect drainage.

Drainage within the parcel flows in an easterly direction. Historically there are two drainage patterns within the plan area. The first drainage pattern is the flow of surface drainage from higher elevation lands in the north west to lower elevation lands in the extreme south east. Again, this is surface drainage with no clearly defined channelization. The second drainage channel is clearly defined and is located within the north quadrant of the parcel and flows in an easterly direction towards lands east of the development. This drainage channel traditionally facilitated the flow of drainage from lands to the west of the plan area, however, aggregate extraction on lands west of the plan area have now altered that historic drainage channel and only drainage from the existing parcel are now captured within this channel. This view is supported through the interpretation of both current and historical aerial photography supplemented by field verification.

Due to changing Water Act Approvals of an adjacent property a complete Storm Water Management Report should be provided prior to approval of phase one's subdivision approval

3.3 UTILITY RIGHTS OF WAY & ROAD DEDICATIONS

Road Right of Way Plan 782-0078 has been removed from the lands in title. This Road Plan consists of approximately 2.0 acres (more or less) for road widening was removed from the east boundary of the parcel.

In addition, a Natural Gas Right of Way, as shown in *Figure 4 (Pipelines and Rights of Way)* is dedicated along the south boundary of the plan area, is owned operated by the Lac Ste Anne Gas Coop and serves the existing country residential parcels located within the plan area.

3.4 EXISTING USE OF LANDS

The existing use of lands can best be described as a mixed land use. This mixed use reflects the existence of a gravel extraction operation which has

ceased to exist and is awaiting reclamation as per an reclamation permit issued by Alberta Environment and Parks. There are three (3) country residential parcels under separate certificates of title located in the south east corner of the quarter section. There is a dwelling unit located within proposed Lot # 8 as shown within Figure 5 (*Conceptual Plan*). In addition, Lac Ste Anne County has recently approved an industrial storage facility on the site as a discretionary use. The balance of the land is in agricultural production.

3.5 ADJACENT LAND USES

Lands to the north of the plan area have been reclaimed from aggregate removal activities and are in cultural production. Lands to the east of the plan area are Industrial (aggregate extraction and wetlands) while south of the plan area the lands are predominantly cultivated lands.

The area to the west of the plan area contains both cultivated soils and a large aggregate extraction operation.

3.6 VEGETATION & SOILS

The CLI capability for agriculture is rated as class 4M (60%) and O (40%) within the Plan Area. Soils that are rated as class 4 have severe limitations and have restrictions on the range of crops or require special conservation practices due to moisture limitations. The areas that are rated as O are considered organic soils.

3.7 SURFICIAL GEOLOGY

Based on the Quaternary Geology, Central Alberta map (I. Shetsen 1990) the Plan Area is located in an area described as a stagnation moraine which typically exhibits till of uneven thickness with local water sorted material up to 30 m thick. This stagnation moraine also exhibits undulating to hummocky topography reflecting variations in till thickness. In this area, the topography is described as undulating with local relief generally less than 3 m. Bedrock at the base of the deposit consists of grey feldspathic, clayey sandstone, grey bentonitic mudstone and carbonaceous shale of the Horseshoe Canyon Formation of the Cretaceous age. The bedrock contact is non marine and is often characterized by a concretionary ironstone beds, scattered coal and bentonite beds of variable thickness, minor limestone beds.

3.8 EXISTING TRANSPORTATION NETWORK

Figure 3 (*Pipelines & Rights of Way*) illustrates the major features of the area's existing transportation network. The system impacting the plan area is comprised of Range Road 20 which is constructed to a paved industrial road standard to the east and Township Road 540 to the south and is constructed to an 'all weather' gravel standard.

3.9 ACCESS

There are two access points serving the plan area. The area north of the ravine has a direct access to Range Road 20 while access to the balance of the parcel is gained via Township Road 540 with an existing approach located along the west boundary of the plan area.

3.10 MAJOR FRANCHISED UTILITIES

Fortis Alberta owns an overhead power transmission line, which is located on the west and south boundary of the Plan area. Future subdivision will require connection to this line for electrical servicing.

Lac Ste Anne Gas Coop is presently located along the south boundary of the plan area and serves the three existing country residential lots which are under separate titles. Future subdivision and subsequent development will require connection to this line for natural gas service.

3.11 ENCUMBERANCES

A restrictive Covenant pursuant to the Municipal Government Act is registered against the lands contained within the 'Plan Area'. The intent of the covenant was to restrict future residential development until the aggregate mining activity which was located in the north quadrant of the quarter section had been exhausted.

4.0 STRATEGY

4.1 PLAN PRINCIPLES

4.1.1 SUSTAINABLE DEVELOPMENT

The first principle is focused on a sustainable community or sense of place that will encompass the core values of a 'Triple Bottom Line' consisting of social responsibility, economic viability and ecological integrity.

- All development shall be in an environmentally sustainable manner, which includes the protection of groundwater supply to ensure that this resource lasts well into the future.
- Development shall be restricted to non-polluting uses and practices.

4.1.2 SIGNIFICANT ENVIRONMENTAL FEATURES

The second principle is one of identifying and protecting environmental features of significance.

- Low lying areas, which at the present time serve as a seasonal surface runoff retention area, need be protected where possible.

4.1.3 INTEGRITY OF TRANSPORTATION INFRASTRUCTURE

The third principle is maintaining the integrity of the transportation network:

- Ensuring that development is accommodated in a fashion that public safety is first and foremost.

4.1.4 EXPANSION OF THE TAX BASE

The fourth principle is taking the necessary steps to broaden the tax base of Lac Ste Anne County in a manner that limits the demands of new development on the County's existing infrastructure.

- Proposing the uses of land that demand reasonable and practical levels of service from the County of Lac Ste Anne.
- Encourage the location of land uses in which assessment are sustained at a high level (minimal depreciation).

4.1.5 GEOGRAPHICAL IMPORTANCE AND VISUAL APPEAL

The last principle relates to the highest and best use of lands:

- Ensuring that the nature of development is one that reflects positively on the County of Lac Ste Anne.
- Development espouses the values of a proud, prosperous, healthy, and vibrant community.
- Consistent site development guidelines are implemented to ensure that development within the Plan Area is visually appealing and environmentally sustainable.

4.2 PLAN PROCESS

The Plan preparation process began by gathering, reviewing and analyzing all relevant information pertaining to future development options within and around the Plan Area. In addition, inventories relating to hydrology and soil permeability are referenced in support of this Plan.

4.2.1 BIOPHYSICAL ASSESSMENT

The overall purpose of the Biophysical Assessment (*Appendix 'A'*) was to determine the environmental features found on the subject property, including any watercourses or wetlands subject to the *Alberta Water Act* or the *Public Lands Act*.

- To determine the importance and conservation value of various natural areas located in the study area with respect to future development;
- To determine the value of existing wetlands, woodlots and other ecological features within the study area;
- To assess the connectivity with other surrounding ecological features;
- To identify potential environmental and municipal reserves;
- To identify applicable legislation; and
- Provide recommendations for conservation measures.

4.2.2 GEOTECHNICAL INVESTIGATION

In compliance with the requirements stipulated within Lac Ste Anne County Design Guidelines and Construction Standards for Development, a geotechnical investigation was undertaken by Twerdoff & Associates Inc. (*Appendix 'B'*) The objectives of the aggregate assessment were:

- Determine subsurface soil and groundwater conditions. This was completed by drilling a series of boreholes within the property and installing groundwater monitor wells.
- Evaluate the proposed work plan and make recommendations on soil densification and/or preloading for the foundation system.
- Provide a summary of the subsurface soil and groundwater conditions, summary of stratigraphy, suitability as fill soils, and make recommendations with respect to foundation types, pavement design and lateral earth pressure calculations to be used in the design limitations for local infrastructure and buildings.

4.2.3 TRAFFIC IMPACT ASSESSMENT

The purpose of the Traffic Impact Assessment (*Appendix 'C'*) prepared by D & A Paulichuk Consulting Ltd. was to assess the potential impact of additional traffic on both the existing local and regional transportation network as a direct result of the proposed development.

The assessment identified and defined the study area, the planning horizon, the analysis period and estimated traffic demand coupled with existing traffic conditions.

In addition, the assessment incorporated a safety analysis, site access analysis, traffic collision analysis, sight distance evaluation and provides overall recommendations for addressing local and regional traffic impacts for incorporation into the ASP.

4.2.4 HYDROLOGICAL ASSESSMENT

A hydrological assessment was prepared by Groundwater Information Technologies Ltd. to determine if a safe 20 yield of potable water was available to sustain the proposed development. A copy of the analysis is attached hereto and referenced as Appendix 'D'.

4.2.5 STORM WATER MANAGEMENT PLAN

A Storm Water Management Plan (SWMP) was prepared by River Engineering Consulting to identify the implications of a 1:100 year flood event as well as the Management of Storm Water as it relates to development within the plan area. The SWMP identified the two catchment areas and provided an analysis of the catchment area that is impacted by Phase I of the Clearview Ridge development plan. Analysis of the other catchment area was not completed as it did not directly affect or was affected by the development and is the subject of a reclamation plan which is yet to be determined.

5.0 PLAN OBJECTIVES

In the preparation of the ASP a detailed review of existing planning policy was conducted to ensure alignment with Lac Ste Anne County planning and policy principles. Without restricting the generality of the foregoing the following documents were referenced within the ASP:

- Lac Ste Anne County Land Use Bylaw;
- Lac Ste Anne County Municipal Development Plan;
- Lac Ste Anne County Development and Design Standards;
- Lac Ste Anne County Corporate Strategic Plan;
- Lac Ste Anne Rural Road Study;
- Lac Ste Anne Master Transportation Plan;

Having regard for the Lac Ste Anne County planning and policy principles and based on the premise of the triple bottom line as the foundation for creating a sustainable development the objectives of the ASP are to:

- Prepare a future development concept for the plan area and provide policy direction describing the manner in which land may be developed.
- Promote a transportation network that includes multiple modes of transportation which shall include automobiles, cycling and walking.
- Identify a strategy for providing open space and trails, including linkage to the existing open spaces and trail system and promote interconnected road and path systems that facilitate efficient provision of municipal services and maintenance.(Trail Plans will be incorporated as outlined in the phase 1 subdivision application
- Provide for the efficient and phased conceptual design of development.
- Identify lands suitable for public recreational opportunities and include both active and passive recreational opportunities.
- Ensure proper protection for environmentally sensitive areas.
- Recognize and maintain the landscape and other environmental qualities of the plan area.
- Promote the diversification of the local economy by examining economic opportunities and economic viability.
- Incorporate diversity of use that may include and promote home, work and play in close proximity.

- Promote the inclusion of various energy efficiencies in the design of the community by orientating streets to maximize solar gain and reduce northern exposures.
- Promote crime deterrence through safe neighbourhood design by incorporating into the design visible public spaces, clear boundaries between public and private spaces, and identifiable points of entry.
- Promote development which advocates live, work and play opportunities within a mixed use built environment.

6.0 CONCEPT PLAN

The land use prescribed for the site is reflected in Figure 5 (*Land Use Concept Plan*). This concept plan outlines the land uses, transportation network, municipal reserve, open space and utility infrastructure for the Plan Area.

It is acknowledged that much of the plan area is considered as land possessing limited agricultural capabilities. It is further acknowledged that existing development patterns, development pressures, existing parcel configuration, general public input and the needs of the community to broaden the economic base of the County suggests strong support for the proposed use of the lands.

As such, the plan area has been identified as having characteristics favorable for future development as per Figure 5 (*Land Use Concept Plan*).

Development of the Plan Area could provide the opportunity for properties adjacent to the plan area to take advantage of upgraded infrastructure.

The Land Use Concept Plan proposed for the Lac St Anne Area Structure Plan is shown on Figure 5 (*Land Use Concept Plan*).

Specifically, the Land Use Concept Plan responds to the following critical factors:

- The Lac Ste Anne County existing statutory plans and in particular the Municipal Development Plan.
- Acknowledgment of the existing conditions, such as natural features, current uses of land, parcel boundaries, and subdivision and development opportunities, which result in both opportunities and constraints for future land use.
- The existing transportation network infrastructure including proposed improvements.
- Acknowledgement that infrastructure services are readily available.
- The existence of an aggregate extraction activity which has ceased operation.

6.1 PHASED DEVELOPMENT

The Clearview Ridge Area Structure Plan envisions that the Plan Area will be a phased development, the timing of which is influenced by several factors notwithstanding:

- The immediate needs of the landowner.
- Market conditions.

- A coordinated approach in the provision and construction of 'on-site' infrastructure requirements such as road design, storm water management, and shallow utility installation.
- The establishment of an economic model that reflects the nature of the development and the limitations of front-loading financial obligations.
- The completion of the Resource Extraction Reclamation Plan prepared by Aspen Land Group Inc. on behalf of Westrock Aggregates Ltd. dated December 2013, attached herewith, and referenced as Appendix 'E'.

6.2 PLAN POLICIES

The policies listed below are unique to the Clearview Ridge Area Structure Plan and are to be applied at the time of the new subdivision and development. All existing statutory plans and policies, particularly those policies referenced in the Lac Ste Anne County Municipal Development Plan, as well as the Land Use Bylaw shall be applied.

Municipal officials and industry representatives have indicated that the demand for a demand for lots ranging from two acres to parcels in excess of five acres in size. To address the need for flexibility in parcel size, a 'scalable' development is proposed in which parcels may be consolidated to achieve specific needs

In addition, it is widely recognized that there is an immediate need for the provision of Country Residential parcels which will support and facilitate the creation and operation of minor and major home based business pursuits which are supplemental or ancillary to the primary country residential use.

6.2.1 PHASE I

As referenced in Section 6.1 the development of the overall plan area is directly impacted by the completion of a Resource Reclamation Plan, the timing of which is at the sole discretion of the third party.

The reclamation plan, which provides various reclamation options, is to be undertaken by the third party which mined the resource on this parcel prior to purchase by the existing landowner. Until such time as the reclamation plan has been completed the identification of such features as open space, pathways, gradients etc., within the area identified as future phasing cannot be completed.

6.2.1.1 Phase I shall consist of 9 Country Residential lots, Municipal Reserve dedication and a Public Utility lot, as well as, the provision of a road system necessary to support the phase I development containing approximately 18.24 ha (45.07 acres) as shown in Figure 5 (Future Land Use Concept Plan).

6.2.2 FUTURE PHASING

Future phasing is categorized as two distinctly separate tracts of lands. One tract is referred to as Future Country Residential Phasing or Phase II and consists of Lots 2, 3, 4 & 5 as shown in *Figure 5 (Future Land use Concept Plan)*.

The other tract is referenced as the residual parcel which contains the existing industrial activity as well as lands which are identified as being outside Phase I or II. Determining future land uses, services or surface features within this tract is entirely dependent on the reclamation option which is implemented as part of the Reclamation Plan referenced above.

6.2.2.1 Phase II shall consist of Lots 2,3,4 &5 as shown in Figure 5 (Future Land Use Concept Plan).

6.2.2.2 Prior to development of Phase II or lands referenced as Future Phasing additional technical investigations as deemed necessary by Lac St Anne County, such as Storm Water Management etc. shall be provided and the plan updated to reflect the change in status.

6.3 LAND USE

6.3.1 LAND USE POLICIES

Notwithstanding the above, all future subdivision and development within the Plan Area shall have regard to the spirit and intent of the Lac Ste Anne County Municipal Development Plan.

- 6.3.1.1 All future subdivision and development within the Plan Area shall comply with the generalized Land Use Concept shown in *Figure 5 (Land Use Concept Plan)*.
- 6.3.1.2 All future subdivision and development within the Plan Area shall be in accordance with the requirements of the Lac Ste Anne County.
- 6.3.1.3 Prior to approval of future phasing as shown in *Figure 5 (Land Use Concept Plan)*, a comprehensive geotechnical investigation may be required by the developer in support of future phasing development.
- 6.3.1.4 The introduction of future phasing shall be contingent on the completion of the reclamation works as identified in the reclamation plan as prepared by Aspen Land Group on behalf of Westrock Aggregate Ltd.

- 6.3.1.5 The dedication of future open space, pathways and cycle trails shall be contingent on the completion of the reclamation works as identified in the reclamation plan as prepared by Aspen Land Group on behalf of Westrock Aggregate Ltd.
- 6.3.1.6 Development of land within the plan area shall be in strict accordance with provincially mandated setbacks from water bodies and environmentally sensitive areas.
- 6.3.1.7 The Developer shall be required to enter into a deferred services agreement with the County prior to receipt of subdivision approval of Phase I, and the County may register a caveat respecting the said deferred services agreement for the purpose of informing lot owners of the requirement to connect to a municipal water and/or waste water system when such services become available.

6.4 TRANSPORTATION NETWORK

The proposed development is to be served by both a proposed access to Township Road 540 as well as an additional access point on Range Road 20 as shown in *Figure 5 (Land Use Concept Plan)*.

6.4.1 TRANSPORTATION POLICIES

- 6.4.1.1 All road improvements, including new construction shall be constructed to municipal standards as stipulated in the Lac Ste Anne County General Municipal Servicing Standards dated January 2008, and shall be at the sole cost and expense of the Developer.
- 6.4.1.2 Required future road widening to any existing roads including Township Road 540 and Range Road 20 shall be dedicated at the time of subdivision.
- 6.4.1.3 All internal development (Future Phasing) within the Plan Area will access a proposed internal road system as shown in *Figure 5 (Land Use Concept Plan)*.
- 6.4.1.4 The Developer shall enter into an agreement with the landowner of N.E. of 1-54-2-5 relating to the relocation of an existing approach with the N.E. of 1-54-2-5 to become a shared approach to access proposed lot 14 as shown in *Figure 5 (Land Use Concept Plan)*.
- 6.4.1.5 The identification and dedication of future open space, pathways and cycle trails shall be contingent on the completion of the reclamation works as identified in the reclamation plan as prepared by Aspen Land Group on behalf of Westrock Aggregate Ltd.
- 6.4.1.6 To ensure that a secondary transportation outlet is available for safety and security purposes a fire lane as shown on *Figure 5 (Land use Concept Plan)* shall be provided.

6.5 ENVIRONMENTALLY SIGNIFICANT FEATURES

Policies are proposed to ensure that the existing environmentally significant features within the Plan Area are protected, while also providing opportunities for the development of new environmental features.

6.5.1 ENVIRONMENTALLY SIGNIFICANT FEATURES & SUSTAINABILITY POLICIES

- 6.5.1.1 All development within the plan area shall be the subject of Smart development principles and the application of the Triple Bottom Line approach to development.
- 6.5.1.2 The use of high efficiency and energy efficient building materials, fixtures and appliances shall be encouraged.
- 6.5.1.3 The individual harvesting of rain water for irrigation purposes on each lot shall be encouraged.
- 6.5.1.4 Where possible buildings shall be orientated to provide the greatest exposure to the sun and create solar heating and solar capture opportunities.
- 6.5.1.5 Each lot owner shall be encouraged to plant shelter belts along the north boundary of their lot to provide additional protection from the northern winds.
- 6.5.1.6 When feasible the harvesting of surface runoff and rainfall captured from buildings and used in the business process shall be encouraged.

6.6 MUNICIPAL RESERVE /OPEN SPACE

The Clearview Ridge Area Structure Plan supports the policies within the Lac Ste Anne County Municipal Development Plan with respect to encouraging development of an integrated and contiguous trail system.

- 6.6.1 Municipal Reserve, as shown in Figure 5 (*Land Use Concept Plan*) will be dedicated in a manner which will enhance and compliment both the Plan Area and the community at large.
- 6.6.2 Municipal Reserve shall be provided by either the dedication of land, disposition through cash in lieu, or a combination of both. The disposition of reserve including the disposition of the existing reserve caveat shall be at the sole discretion of the Lac Ste Anne County at the time of subdivision.

- 6.6.3 Pedestrian and bicycle linkages to the adjacent community as shown in *Figure 5 (Land Use Concept Plan)* shall be incorporated into Phase I of the development.
- 6.6.4 Pathways/trail systems shall be aligned with existing and future external pathway/trail systems where possible.
- 6.6.5 Pathway/trail systems shall, where possible, be incorporated into landscape buffers and sound attenuation berms thereby increasing both the utility and the aesthetics of the berms and buffers.
- 6.6.6 Pathway/trail systems shall where possible serve as a 'transition' between differing proposed and existing land uses.
- 6.6.7 The proposed community mail box as well as a school bus collection site may be located as shown in *Figure 5 (Land Use Concept Plan)*.

6.7 SERVICING INFRASTRUCTURE

The purpose of the strategy is to provide for the installation of appropriate utility infrastructure necessary to support the Plan area. Water and Waste Water will be the responsibility of the individual lot owner. Storm water ponding will be constructed within the Plan area to prevent downstream impacts due to surface water runoff.

Sizing and detailed design of the servicing infrastructure including the storm management will be completed in conjunction with the subdivision and development of the project and administered through the development agreement process.

All infrastructure service development shall be designed and constructed in accordance with the specifications and guidelines as provided in the Lac Ste Anne County General Municipal Servicing Standards dated January 2008.

- 6.7.1.1 All infrastructural development within the plan area shall have regard for the possible extension of services to adjacent lands.
- 6.7.1.2 Where identified, oversizing of infrastructural services within the plan area will be incorporated into the design and construction process.
- 6.7.1.3. Where possible, the developer in conjunction with the Lac Ste Anne County shall examine an innovative approach to infrastructure improvements and services that are based on sound economic business practices.

6.7.2 POTABLE WATER POLICIES

The existing West Inter Lake Regional Water line is in close proximity to the plan area.

6.7.2.1 All lots created within the plan area shall be serviced with potable water.

6.7.2.2 Lots within the plan area may be serviced with individual wells and drilled in accordance with provincial regulations.

6.7.2.3 Access to the West Inter Lake Regional Water Line to serve the plan area shall be investigated.

6.7.3 SANITARY SEWER SYSTEM

All subdivision and development shall be serviced with individual Private Sewage Disposal Systems and shall include secondary waste water treatment.

6.7.3.1 SANITARY SEWER SYSTEM POLICIES

6.7.3.1 Prior to the issuance of a development permit for any construction on each specific lot, the Developer shall provide the approved plans of the proposed construction including a site plan indicating the topographical features.

6.7.3.2 Design for constructed waste water collection system shall follow the 'Standards and Guidelines for Private Sewage Disposal Systems, Alberta Municipal Affairs, and latest edition as a minimum.

6.7.4 STORM WATER MANAGEMENT

The Storm water management system as referenced in Appendix 'F' will consist primarily of piping and overland drainage. Storm water run-off from the Plan Area will be restricted to pre-development flows in accordance with the County and Alberta Environment and Parks standards unless otherwise approved by the Lac Ste Anne County. This will be accomplished by using dedicated public utility lots, joint use recreational lands, roadway ditches, culverts, and drainage swales along lot lines. Individual lots will be graded to direct runoff water to the drainage swales, ditches or catchment basins. These ditches will be used to convey water from the lots to the storm pond.

The layout of the overland flow system is designed to work closely with existing topography, as well as the lot layout. The slopes of the ditches and the drainage swales should be maintained between 0.5% and 2.0%. The ditches and swales should be protected with grass vegetation as soon as possible to

reduce erosion, and help with storm water quality.

STORM WATER MANAGEMENT POLICIES

- 6.7.4.1 Prior to the subdivision and development of Phase II or 'Future Phasing' the Developer shall implement a storm water management plan to the satisfaction of the Lac Ste Anne County.
- 6.7.4.2 Design for constructed storm water management facilities shall follow the 'Standards and Guidelines for Municipal Waterworks, Wastewater and Storm Drainage Systems,' Alberta Environment and Parks, latest edition as a minimum.
- 6.7.4.3 All necessary regulatory approvals shall be acquired prior to the construction of the storm water facilities and infrastructure.
- 6.7.4.3 Native soils shall be salvaged and stockpiled and reused as topsoil and planting bed material.
- 6.7.4.4 Prior to the subdivision and development of each lot a 'lot grading plan' in accordance with the Storm Water Management Plan will be provided for each lot created.
- 6.7.4.5 Prior to the subdivision and development of Phase I as shown in Figure 6 – (*Storm Water Management Plan*) the developer and the Lac Ste Anne County shall enter into an agreement with respect to the implementation of a phased storm water management plan.
- 6.7.4.6 Storm water retention devices such as 'dry ponds' may be designed as joint utility/recreational uses.
- 6.7.4.7 Overland drainage over adjacent lots will not be permitted.



FILE # RTW107

DATE: NOV 2014



FIGURE 1 REGIONAL CONTEXT

SIMPSON PARK AREA STRUCTURE PLAN





FILE #RTW 087
DATE: MAY 2016


SCALE: 1: 1000

FIGURE 3 TOPOGRAPHICAL FEATURES
LAC STE ANNE AREA STRUCTURE PLAN

WESCOTT CONSULTING GROUP
25 Copperstone Common S.E. Calgary, Alberta T2H 3P5
Phone 403-901-7586 Fax 403-902-6333
info_wescott@telus.net



TILE #RTW 087

DATE: MAY 2016


 SCALE: 1:1000

FIGURE 4 PIPELINES & RIGHTS OF WAY

LAC STE ANNE AREA STRUCTURE PLAN

WESCOTT CONSULTING GROUP

26 Co-operative Common S.E. Calgary, Alberta T2H 5P5
 Phone 403-901-7556 Call 403-903-9303
info_wescott@shaw.ca







FILE #RTW 087

DATE: FEB 2018

SCALE: 1:1000

FIGURE 5. CONCEPT PLAN
LAC STE ANNE AREA STRUCTURE PLAN

WESCOTT CONSULTING GROUP
25 Cooperative Common S.E. Calgary, Alberta T2H 0P5
Phone 403-981-7066 Fax 403-982-8323
info_wescott@shaw.ca



FILE # RTW 087

DATE: FEB 2018

SCALE: 1" = 1000'

FIGURE 6 STORM WATER MANAGEMENT

LAC STE ANNE AREA STRUCTURE PLAN

WESCOTT CONSULTING GROUP

28 Congression Corner S.E. Calgary, Alberta T2H 3P9
 Phone 403-981-7560 Fax 403-982-8223
 bob_wescott@wescott.ca

“LAC ST ANNE Biophysical Assessment”

S.E. 1-54-2-5

PREPARED BY

MCA Environmental Management

APPENDIX ‘A’

January 2016



MCA
Environmental Management

**Biophysical Inventory
For SE ¼ Section 1-54-2 W5M
County of Lac Ste. Anne
MCA File#2015-1222**

**Submitted to:
Wescott Consultants Ltd.**

November 2015 (revised January 2016)

Executive Summary

This biophysical assessment has been completed for Launch Venture Ltd. and their consultants Wescott Consulting Group (Cochrane, Alberta), as part of the County of Lac Ste. Anne planning requirements. The client's objective is to develop a parcel of land that is approximately 44 ha at the northeast corner of Lac Ste. Anne, Alberta.

The land description of the property is a portion of the SE 1-54-2 W5M in the County of Lac Ste. Anne.

The biophysical assessment will serve as the base for planning the future development of the property as required by an area structure plan (ASP).

A biophysical assessment is a necessary requirement for the approval of a more detailed ASP for the proposed project site, as well as approval of the storm water management plan for the subdivision, under the *Alberta Water Act* and the *Environmental Protection and Enhancement Act*.

A biophysical assessment is conducted to identify significant and sensitive environmental components on the project site prior to the development of an ASP, and to make recommendations on the sustainability of the site, whether parts of it can or should be preserved in the natural state, and if so, what mitigation and monitoring measures are necessary to achieve sustainability. The Assessment provides recommendations for dedication of lands to be conserved in their existing state within the context of the proposed development project, for the purposes of conservation of habitat, hydrology, and protection of erodible land, water quality or other environmental needs.

The results of the biophysical assessment indicate that the property presents two distinct biological areas. The upland crop area where there is very low diversity with a small wetland on the east border and the creek ravine where there is a great deal of biological diversity. There were no indications of potential species at risk on either site however a detailed investigation on the south side of the ravine was not completed due to the naturally occurring steep slope. The ravine provides a significant ecological linkage with adjacent properties and is considered valuable not only to the property but also to the regional movement of species.

The County of Lac St Anne has identified this particular ravine as a main drainage area (creek) and requires setbacks of 70 meters from the top of the ravine. The ravine has a very steep banks and drivers for this designation may include value of the ecological setting as well as the lack of stability on the slope.

A former gravel pit in the north west corner of the property is registered under the name of Westrock Aggregates Ltd. The mine is exhausted and the pit owner has submitted a pit registration application to Alberta Environment in order to pursue pit reclamation. The reclamation plan encompasses an area of approximately 54 ha which includes the pits located in NE 1 and the SE 1 -54-2W5M. Specifics related to the proposed reclamation are provided in the registration application on file with Wescott.

The subject property has been in use for agricultural purposes since at least 1949 and consists of productive agricultural land in the form of a class 2 soil. While there are no specific limitations related to the development of the land for wildlife habitat, regulators and the developer will likely recognize that development of the site will result in a decrease to local productive agricultural land.

Recommendations for future studies prior to the development of the land include the following.

- Identify timing of the pit reclamation program

- Develop a master drainage plan prior to site development as per the Storm water management guidelines
- Fire prevention and control strategies
- Site hazard management planning
- Weed management and control

These recommendations are not intended to be exclusive. Regulators may have additional requirements not listed here. Reference should be made to the recommendations provided in this report along with the Lac St Anne Municipal Development Plan.

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1.0 INTRODUCTION

1.1 Project Purpose

This biophysical assessment has been completed for Launch Ventures Ltd. and their consultant Wescott Consulting Group (Cochrane, Alberta), as part of the County of Lac Ste. Anne Planning requirements. The client objective is to develop a parcel of land comprising approximately 55 ha. The proposed development will entail residential acreage properties on the central portion of the site and potential gravel extraction on the northwest portion of the property.

The legal land description of the property is SW 1-54-2 W5M. The land is located within the jurisdiction of County of Lac Ste. Anne. In order to allow for the land to be designated as country residential (for the purposes of subdivision), the County requires that an Area Structure Plan (ASP) be developed for the site.

A biophysical assessment is a necessary requirement for the approval of a more detailed ASP for the proposed project site, as well as approval of the storm water management plan for the subdivision, under the *Alberta Water Act* and the *Environmental Protection and Enhancement Act*.

A biophysical assessment is normally conducted in order to identify significant and sensitive environmental components on the project site prior to development. The biophysical assessment will provide recommendations on the sustainability of the site, whether parts of it can or should be preserved in the natural state, and if so, what mitigation and monitoring measures are necessary to achieve sustainability. The Assessment provides recommendations for dedication of lands to be conserved in their existing state within the context of the proposed development project, for the purposes of conservation of habitat, hydrology, and protection of erodible land, water quality or other environmental needs.

Accordingly, the purpose of this Assessment is:

- To identify and evaluate existing ecological features on the site as they appear at the present time;
- To provide practical recommendations for preserving or enhancing ecologically significant features within the context of the ASP;
- To provide general recommendations for mitigation of potential adverse environmental effects resulting from the development, on the site and on surrounding lands;
- To identify potential environmental and/or municipal reserves;
- To identify applicable legislation; and,
- Provide recommendations for conservation or mitigation measures.

1.2 Project Overview and Site Location

The quarter section of land containing the proposed development site is located at the northwest corner of the intersection of range road 20 and township road 540. The legal land description is SE 1-54-2 W5M containing approximately 55 ha. According to the Lac Ste. Anne Municipal Development Plan (Map 2) the site is currently located in an area identified for future rural

residential development. This site however is still classified as agricultural however although there are two subdivided lots in the southeast corner of the site and a new residence located near the west access road.

Figure 1 Shows the location of the project site in regional contexts. Figure 2 shows the site location on the County Map. Figure 3 shows a legal survey of the property.

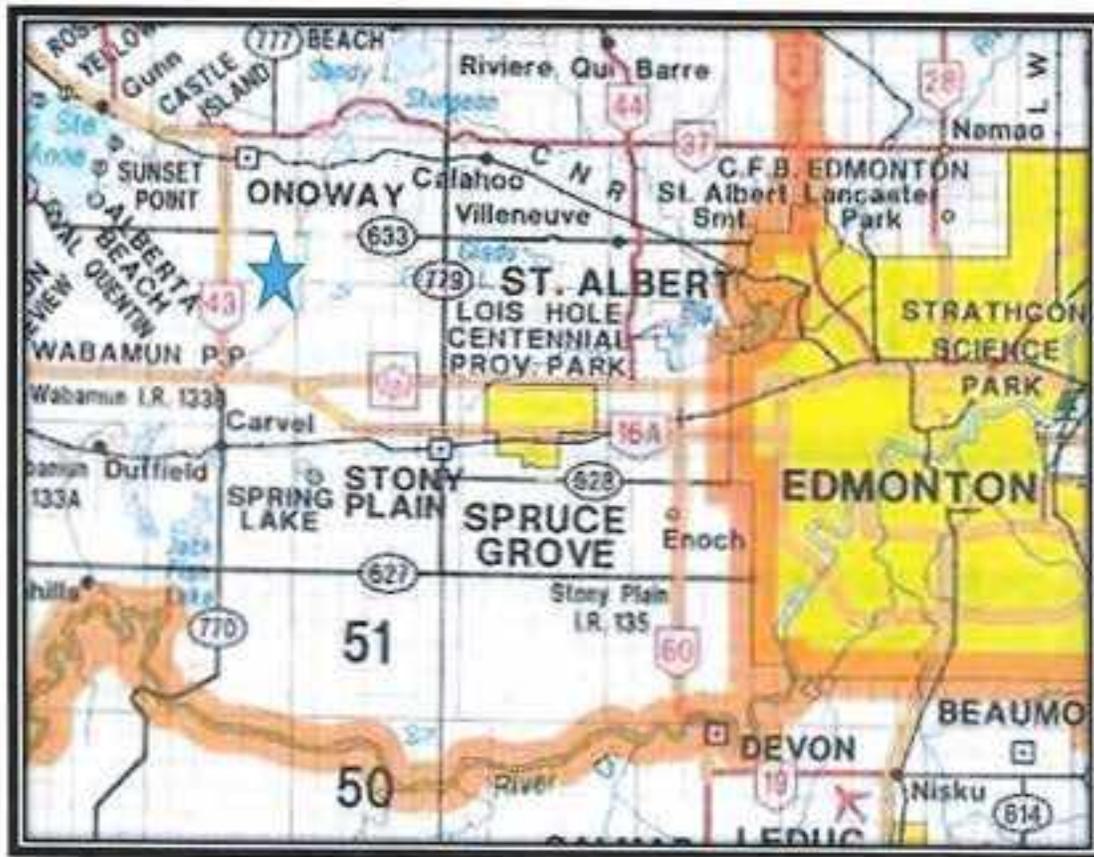


Figure 1: Site location 

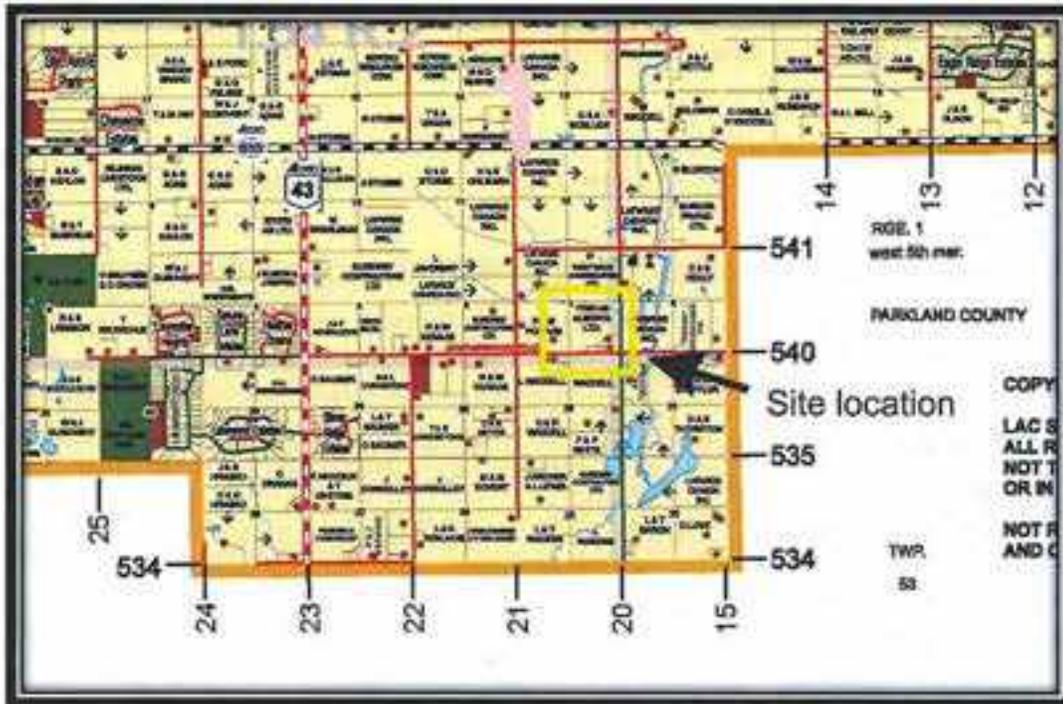


Figure 2: Site Location Lac Ste. Anne County Map

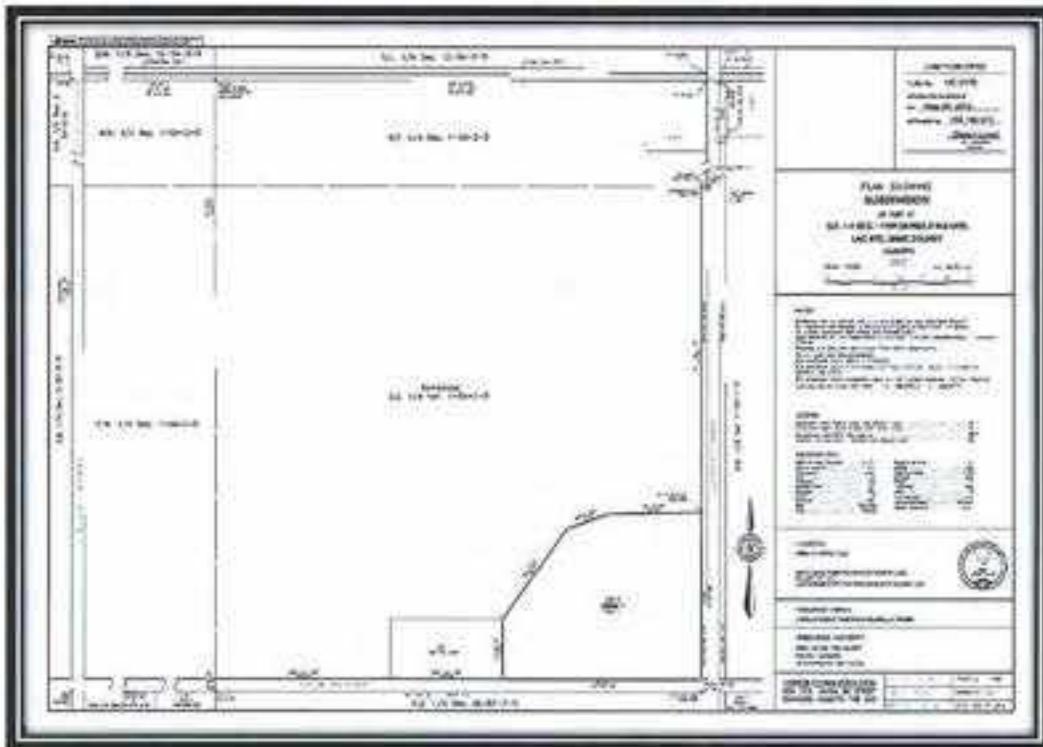


Figure 3: Legal Survey of Site Boundaries

1.3 Scope of the Assessment

The biophysical assessment addresses all parts of the natural environment, and includes:

- Topography, geology and soils;
- Hydrology (surface water, ground water);
- Vegetation (terrestrial, wetland);
- Wildlife (birds, fish, herptiles, invertebrates, mammals) and potential habitat;
- Sustainability of ecosystems;
- Linkages with adjacent ecosystems (connectivity); and,
- Biodiversity and species at risk (rare, threatened and endangered species).

The geographical scope of the assessment is the proposed project site, where boundaries are shown in Figure 3. The assessment also takes into account adjacent land uses and ecological linkages with the subject property in a regional context if warranted.

2.0 APPROACH AND METHODOLOGY

The biophysical assessment was conducted to describe and interpret site features as they existed at the time of the field reconnaissance, which took place on Tuesday August 25, 2015. The study included the following activities:

- Consultation with Wescott Consulting Group undertaking the detailed design of the proposed site development;
- Discussions with site owner on proposed future activities;
- Review of any maps, previous reports, etc., completed for this project;
- Examination of historical aerial photographs, to assess surrounding land use, vegetation areas, developments, etc.;
- Database searches, e.g., ANHIC database for tracked and listed species;
- Field reconnaissance of the site; and,
- Analysis of the information, and drafting the biophysical assessment report.

The scope of the field reconnaissance was to observe features of vegetation, drainage, wildlife and other components of the natural environment, as existed at the time of the field reconnaissance. The types and distribution of vegetation, the type of forest, drainage features, wildlife and wildlife signs (e.g., tracks, feces, hair, burrows, nests, rubs, scrapes, etc.), and any other environmental features, were noted by location and documented with photographs.

The contours of the site were observed on topographical map sheets and directly in the field, and note was made of any depressions which might form a wetland, gully or natural drainage course.

Digital historical aerial photos were obtained from Alberta Sustainable Resource Development (ASRD) and examined at intervals of 5 to 10 years from July 1949, through to August 2011 to learn about past and present vegetation cover, earthworks, human activities or places on the site water tends to collect. Potential wetlands were also identified by noting lower topographical areas, and dark-shaded areas in the aerial photographs. The potential wetlands were then observed in the field, and the vegetation growing there was examined. Classification of these areas as potential wetlands was done mainly according to the Stewart and Kantrud Wetland Classification System (Stewart and Kantrud, 1971). The historical aerial photographs were examined to determine the configuration of each low, wet area and the amount of surface water present in various years.

2.1 Previous Assessments and reports

Background information for the site were provided by Wescott Consulting and included the following.

- Aggregate Characterization Assessment
- Gravel permit (not reviewed)
- Pit registration application (Aspen Land Group Ltd.)

2.1.1 Aggregate Characterization Assessment

An aggregate characterization assessment was completed by Twerdof and Associates Inc. (June 2014). The characterization revealed gravel resources beneath the site at a depth of 7.6 to 11.3 meters (25 to 37 feet) below overburden. The sand and gravel seam appears to be approximately 3 – 6 meters (10-20 feet) thick. The overburden to gravel ratio is estimated at around 3:1.

2.1.2 Pit registration application

A pit registration application was prepared for an exhausted gravel pit located in the northwest corner of the site. The former gravel pit is registered under the name of Westrock Aggregates Ltd. Aspen Lands Group Inc. prepared the pit registration submission in December 2013. The mine is exhausted and the pit owner has submitted a pit registration application to Alberta Environment in order to pursue pit reclamation. The reclamation plan encompasses an area of approximately 54 ha which includes the pits located in NE 1 and the SE 1 -54-2W5M referred to as the Yeoman and Parker Pits. Specifics related to the proposed reclamation are provided in the registration application on file with Wescott. It is unclear whether the application was accepted by the Alberta government. No dates appear to have been identified for the reclamation. Access into the site for reclamation was proposed from the NE 1 over the culvert in the ravine. The culvert was proposed to remain in place.

Under the Migratory Birds Regulation (under MBCA), no person shall hunt a migratory bird except under authority of a federal permit under this Act/Regulation. Subject to subsection 5(9), no person shall (a) disturb, destroy or take a nest, egg, nest shelter, eider duck shelter or duck box of a migratory bird, or (b) have in his possession a live migratory bird, or a carcass, skin, nest or egg of a migratory bird, except under authority of a permit licensed under this Act/Regulation. This Act and its Regulation become particularly important when removing trees or clearing open pasture to facilitate development, to landscape, or to regrade the land. The lack of tree cover on the property combined with the consistent agricultural land use appear to negate concerns related to migratory birds nesting and rearing on the upland area however, there is tree cover and bird habitat in the ravine to the north where birds may nest and rear young.

3.1.2 Fisheries Act

If there is any proposed activity that would destroy or adversely affect fish or fish habitat, such proposed activity would require an Authorization from Fisheries and Oceans Canada (DFO) under the *Fisheries Act (FA)*. The Fisheries Act {R.S. 1985, c. F14}, applies to all Canadian fisheries waters and assigns the Department of Fisheries and Oceans Canada (DFO) the responsibility to administer and enforce the conservation and protection of fish habitat on private property and on provincial and federal lands. Section 35 of the FA states “ No person shall carry on any work, undertaking or activity that results in serious harm to fish that are part of a commercial, recreational or Aboriginal fishery, or to fish that support such a fishery”.

A recent change to the FA defers fisheries management to local provinces where equivalent fisheries protection measures are in place. Section 36 of the fisheries act prohibits the deposit of deleterious substances. Environment Canada is responsible for administering this subsection.

There is no water body on this parcel of land to which the fisheries act would apply.

3.1.3 Species at Risk Act

The Species at Risk Act (SARA) was passed in 2002 as part of Canada’s commitment to the international Convention on Biological Diversity. The intent of the SARA legislation is to prevent species that are listed in Schedule 1 of the Act from becoming extinct, threatened, or extirpated. Additionally, SARA strives to help in the recovery of any listed species through protecting the critical habitats of at-risk species. Under SARA, it is illegal to kill or harm any listed species, or to destroy the residences of any listed species that occur on federal lands. For listed species that are found outside of federal lands, it is the duty of the province or territory to protect listed species through legislation. This legislation covers birds, plants, fish, mammals, insects, amphibians and reptiles.

In the present case, no rare or endangered species were expected or observed on the portion of the land in which development is proposed. The south slope in the ravine is quite steep and may have species located in areas that were not accessible during the biophysical assessment site visit. The preservation of the ravine as a wildlife corridor should offset any potential concerns related to species at risk in the area.

3.1.4 Federal Policy on Wetland Conservation

The Federal Policy on Wetland Conservation was passed by Cabinet in 1991, with the objective of promoting “the conservation of Canada’s wetlands to sustain their ecological and socio-economic functions, now and in the future” (Government of Canada, 1991). The Federal Policy on Wetland Conservation applies to the full range of federal activities and drives management decisions regarding the protection of wetland habitat on federal lands.

The two key commitments of the Federal Policy on Wetland Conservation include:

- no net loss of wetland functions on federal lands and waters through mitigation of all impacts of development related to these wetlands; and
- enhancement and rehabilitation of wetlands in areas where the continuing loss or degradation of wetlands has reached critical levels

There is one small wetland observed on the subject property.

3.2 Provincial Legislation

3.2.1 Public Lands Act

The bed and shores of all watercourses and water bodies are considered public lands unless the Government of Canada owns them. As such, approvals under the Public Lands Act {R.S.A. 2000, c. P-40} are required for any activity on the bed or shore of Crown owned rivers, streams, or lakes. Any activity that alters or occupies the bed and shore of a water body may be done only after written approval.

A Licence of Occupation (LOC) would be required for in stream structures and permanent or temporary facilities on Crown Land. A Licence of Occupation (LOC) is required under the *Public Lands Act* to build any structures that could have a negative impact on the bed and shore of a water body (e.g., retaining walls, boat launching facilities, breakwater structures, and causeways).

There were no flowing creeks, streams or rivers observed on the subject property. The drainage area in the ravine on the north end however is shown on topographical map sheets.

3.2.2 Environmental Protection and Enhancement Act 1983

The Alberta *Environmental Protection and Enhancement Act (EPEA)* support and promote the protection, enhancement and wise use of the environment. It recognizes the impact of development, polluters paying for their actions, and other such acts.

The Act deals with the release of substances into the environment, regulating releases, and creating general prohibitions with respect to substance release, and also provide the necessary powers to regulate the handling of storm drainage and wastewater. A key part (Section 109) states that no person shall release or permit the release into the environment of a substance in an amount, concentration, or level or at a rate of release that causes or may cause a significant adverse effect, thus covering a very broad range of anti-pollution prohibitions.

Under the Wastewater and Storm Drainage Regulation, EPEA gives powers to Alberta Environment for the regulation of storm water drainage and wastewater systems. The Wastewater and Storm Drainage Regulation and the Wastewater and Storm Drainage (Ministerial) Regulation enable the Department to regulate the operation of storm drainage and wastewater systems and establish standards for such facilities and their operators. This legislation sets out requirements for the construction and operation of municipal plants for handling of storm water drainage and wastewater.

Among other things that the Act covers are the following:

- Harmful emissions to the air (Air Emissions Regulation);
- Release of harmful/toxic substances to the environment (Substance Release Regulation);
- Reclamation of disturbed lands (Conservation and Reclamation Regulation);
- Ozone-depleting substances (Ozone-Depleting Substances Regulation);
- Handling, use and application of pesticides and herbicides (Pesticide Sales, Handling, Use and Application Regulation);
- Potable water (Potable Water Regulation); and,
- Reporting of releases to the environment (Release Reporting Regulation).

EPEA allows for anti-litter orders to be issued for the control of waste on highways, water, ice and public and municipally owned land (which is referred to as enforcement orders). Orders for the cleanup of unsightly property are referred to as environmental protection orders. The forms of both types of orders are set out in the regulations.

EPEA regulates the handling, storage and disposal of hazardous wastes under the Waste Control Regulation. Hazardous wastes are defined in the Regulation.

The Wastewater and Storm Drainage Regulation under AEPEA gives Alberta Environment the responsibility of regulating storm drainage and wastewater systems, including the establishment of standards for such facilities in their operation. This includes naturalized wetlands, other storm water management facilities, outfalls and related piping.

3.2.3 Water Act

The Alberta *Water Act*, which came into force in 1999, supports and promotes the conservation and management of water. It regulates withdrawals and diversions of water, including drilling water wells, through a licensing and authorization system. It regulates water management works and undertakings, and authorizes temporary diversions through a licensing process.

Watercourse crossings (road, bridge, pipeline, telecommunications, etc.) are authorized/regulated through the Alberta Watercourse Crossings Codes of Practice. A Notification must be submitted to Alberta Environment detailing any watercourse crossing structures, and explaining how the construction and operation of the crossing meets the requirements of the Code.

Approval would be required under the *Water Act* from Alberta Environment in respect to any watercourses or wetlands that might be affected, or whose flows may be affected, by the proposed development.

An approval is required to conduct an activity in a water body. An activity is defined generally to include placing or constructing works within a water body, removing or disturbing ground and/or

vegetation that results in altering the flow, level, direction and/or location of a water body. A license is required to divert or transfer water from a water body.

Construction of an outfall would also require that the Code of Practice for Outfall Structures on Water bodies under the Water Act be followed. This Code of Practice dictates restricted activity periods on water bodies, and requires that certain design standards be followed. The Code of Practice also requires that notice be issued to the Director, Alberta Environment, prior to commencement of the work. Hydrological issues are discussed later in this report.

Approvals would be required under the *Water Act* to modify or fill any wetlands that might occur on the Property, and to construct any outfall or drainage channel into a water body.

The Act would also apply if withdrawal of water from a Lake/wetland is being proposed as part of the operation, such as for a domestic water supply or other use.

There is one small wetland observed on the east side of the property.

3.2.4 Alberta Weed Control Act

On June 16, 2010, the new Alberta Weed Control Act received Proclamation and came into force. The new Act is a comprehensive re-write of the old Act for the purposes of re-organizing, updating, and providing greater clarity to the existing provisions of the latter.

This Act aims to regulate noxious weeds, prohibited noxious weeds, and weed seeds through various control measures, such as inspection and enforcement, together with provisions for recovery of expenses in cases of non-compliance. Additionally, it mandates the licensing of seed cleaning plants and mechanisms. The Act requires that the owner or occupant of lands control noxious weeds and destroy prohibited noxious weeds on land the person owns or occupies. In 2012 the Alberta Invasive Plant Identification Guide was published to facilitate the identification of prohibited noxious and noxious plants.

Under provincial legislation, only pesticides that have been registered for use in Canada by the Pest Management Regulatory Agency under the *Canada Pest Control Products Act* can be used in Alberta. Pesticides and herbicides are regulated in Alberta under the *Alberta Environmental Protection and Enhancement Act* and supporting regulation (Pesticide Sales, Handling, Use and Application Regulation).

This legislation provides for the regulation of sales (pesticide vendors) and use (pesticide applicators) of pesticides in Alberta. In addition, there is the Environmental Code of Practice for Pesticides which provides more detailed direction for pesticide sales and use in Alberta.

In the field reconnaissance, various weedy species were observed in the laydown area, but for the most part the land was either plowed for cultivation or dominated by grass species. It is recommended that invasive plant species be managed on site and during future site development (e.g., filling and stockpiling of soil) and that where feasible existing weeds be controlled. Manual and cultural methods should be the priority, but where this is not practical, chemical weed control should follow the above regulations and standards; and in such a manner as not to affect the vegetation of any wetland areas within or near the property.

3.2.5 Wildlife Act

Alberta's Wildlife Act is the main piece of provincial legislation that deals with wildlife. Under the Act, hunting without a licence or out of season is prohibited, as is the possession of wildlife and controlled animals (defined in the Act). The Act also covers diseased animals, damage or threat caused by private animals, and the closing of areas to the public to protect wildlife, where necessary. Licences and permits are issued under the Act to regulate hunting or other activities, as outlined above.

3.2.6 Historical Resources Act

Section 37 of the Historical Resources Act provides the framework for Historical Resources Impact Assessments (HRIAs) and mitigative studies. When, in the opinion of the Minister of Alberta Community Development (ACD), an activity will or will likely result in the alteration, damage or destruction of an historic resource, the person or company undertaking the activity can be required to

- conduct an HRIA on lands that may be affected by the activity;
- submit to ACD a report discussing the results of the HRIA;
- avoid any historic resources endangered by activity; or
- mitigate potential impacts by undertaking comprehensive studies.

HRIAs and mitigative studies are paid for by the person or company undertaking or proposing to undertake the activity. ACD regulates archaeological and paleontological fieldwork through a permit system. All decision-making concerning the management of historical resources rests with ACD.

One of the requirements of an HRIA is to address compliance requirements associated with the Historical Resources Act of Alberta. As such, it becomes incumbent for the proponent to comply with any government requirements that result from a Historical Overview if one has been done. At a minimum, ACD should be contacted, to determine if they have any concerns about the Property in question, from an historical, archaeological or related perspective.

3.2.7 Species At Risk Program

Alberta has a Species at Risk Program, which was initiated as a response to the provinces commitment to the Accord for the Protection of Species at Risk in Canada. The intent of the Accord is to prevent species in Canada from becoming extinct as a consequence of human activity. As part of the assessment procedure, all species of concern are generally assessed and are classified as one of the following categories 1) At Risk; 2) May Be at Risk; 3) Sensitive; 4) Undetermined; and 5) Secure. Any species that is designated as "At Risk" or "May Be at Risk" undergoes a detailed status assessment and is formally designated as Endangered, Threatened, Special Concern, Data Deficient, or Not At Risk. Any species that is designated as Endangered or Threatened becomes legally protected under Alberta's Wildlife Act {R.S.A 2000, c.W-10}. This legal designation prohibits the disturbance, killing or trafficking of these species, and provides immediate protection of nests and den sites. Any species that is designated as "Sensitive" after

a general assessment, or as "Special Concern" after a detailed assessment becomes eligible for special management actions designed to prevent the species from becoming "At Risk".

3.3 Standards Policies and Guidelines

Alberta Wetland policy (2013)

Interim policy for wetland management in the settled areas of Alberta and Guidelines for wetland habitat Compensation

Initially developed in 1993, this interim policy provides direction on the management of wetlands in the settled areas (white zone) of Alberta (Alberta Water Resources Commission, 1993). The primary goal of the policy is to "sustain the social, economic, and environmental benefits that functioning wetlands provide, now and in the future" by conserving wetlands in a natural state, mitigating the degradation and loss of wetlands, and enhancing, restoring, or creating wetlands in areas where they have been depleted or degraded.

If the flow of surface water is altered or blocked, or if a wetland is being altered or destroyed by filling in or draining, the Alberta Policy on Wetlands and the *Alberta Water Act* would apply. The Policy, in essence, requires that there be no *net* loss in wetland habitats in Alberta. If a wetland is destroyed, then compensation must be provided by the person or persons responsible for affecting such damage. Under the policy, compensation can be through directly creating equivalent wetland habitat in another location, or by paying a recognized wetland manager (e.g., Ducks Unlimited) to accomplish this. If the compensation sites are within a certain distance from the affected one, the compensation ratio is 3 hectares of new wetland to 1 hectare of affected wetland. Beyond a certain distance between compensated wetlands and affected wetlands, the ratio becomes higher, and increases with progressive distance. This would apply if any wetland that might occur on the Property was affected by the development or related construction activities.

3.3.1 Standards and Guidelines for Municipal Waterworks and wastewater and stormwater drainage systems

Alberta Environmental Protection has established standards and guidelines for the design and operation of municipal waterworks, wastewater and storm drainage systems (Alberta Environmental Protection, 1997). These standards and guidelines outline four types of requirements: Performance Standards, Design Standards, Design Guidelines, and Operating and Monitoring Requirements and Guidelines. These requirements are all directed towards ensuring public health and environmental protection.

3.3.2 Wastewater and Storm drainage regulation and wastewater and storm drainage (ministerial) regulation

The Wastewater and Storm Drainage Regulation {AR 119/93} and the Wastewater and Storm Drainage (Ministerial) Regulation {AR 120/93} fall under Part 4, Division 1 of the Environmental Protection and Enhancement Act {R.S.A. 2000, c. E-12}. These regulations create general

prohibitions with respect to substance release to the environment and provide powers to regulate the handling of wastewater and storm drainage

3.3.3 Stormwater Management Guidelines for the Province of Alberta

These guidelines were developed as a result of increased urbanization and public expectation for improved runoff control. These guidelines direct the planning, analysis, design, construction, operation, and maintenance of storm water management systems to address concerns associated with storm water runoff and its impact on urban and rural development, and aquatic resources. These guidelines include Best Management Practices for storm water management and quality control.

3.3.4 Municipal Government

Most Municipal Government bodies in Alberta now require that a biophysical assessment and/or Environmental Impact Assessment or the equivalent be done prior to subdivision of land, and before the completion of an ASP or other site-specific development plan. One of the purposes of this Assessment is to provide a specific assessment process for dedication of Environmental Reserve, Municipal Reserve and Conservation Easement based on municipal, community and environmental needs. In addition, it makes recommendations as to how to avoid, minimize or control adverse effects on the existing environment resulting from the development, if it is to proceed, and how to incorporate the principles of sustainability in designing and constructing the development.

The County of Lac St Anne currently has a draft municipal development plan (MDP) on the public website. The MDP Map 12 Setbacks from Key Water Bodies, identifies the ravine as a key waterbody requiring a 70 meter setback from the top of the ravine. Other relevant parts of the MDP for this site include the following sections as an example.

Part II – Administrative Procedures section 3.7.1 The Development Authority may require a complete Hydrological Ground Water Impact Report for any commercial, industrial, or multi-parcel development.

Part IV – Development Regulations

Aggregate Resource Extraction and Aggregate Resource Processing

11.1.9 All sites must be re-districted as AR – Aggregate Resources Extraction and Processing prior to any application for Aggregate resource extraction and/or processing and will be subject to the regulations as set out in that land use district.

11.1.10 Aggregate resource extraction operations are classified as follows:

a) Class I: Operations equal to or greater than 5.0 ha (12.35 acres) of disturbed area on a site. Operations under this classification shall require municipal development approval. Reclamation plans shall be under the direction of AESRD and the Development Authority.

b) Class II: Operations less than 5.0 ha (12.35 acres) of disturbed area on a site. Operations under this classification shall require municipal development approval. Reclamation plans for existing pits shall be under the direction of County administration. New Class II pits shall be prohibited in Lac Ste. Anne County.

Minimum Separation Distance and Restrictive Covenants

11.1.11 For lands districted as country residential, all uses within the district shall comply with the following minimum separation distances from all aggregate resource extraction or aggregate resource processing uses based on the nature of the resource development. The minimum separation distance shall be measured from the property line of the lands districted as country, residential. The minimum separation distances are outlined in Table 11.1.1 and Figure 11.1.1.

Table 11.1.1 – Minimum Separation Distances from Aggregate

Operations

Operation Separation Distance

Extraction: 800.0 m (2,624.67 ft)

Reclamation: 800.0 m (2,624.67 ft)

Crushing: 1,500.0 m (4,921.26 ft)

Wash Plant: 1,500.0 m (4,921.26 ft)

Asphalt Plant: 1,500.0 m (4,921.26 ft)

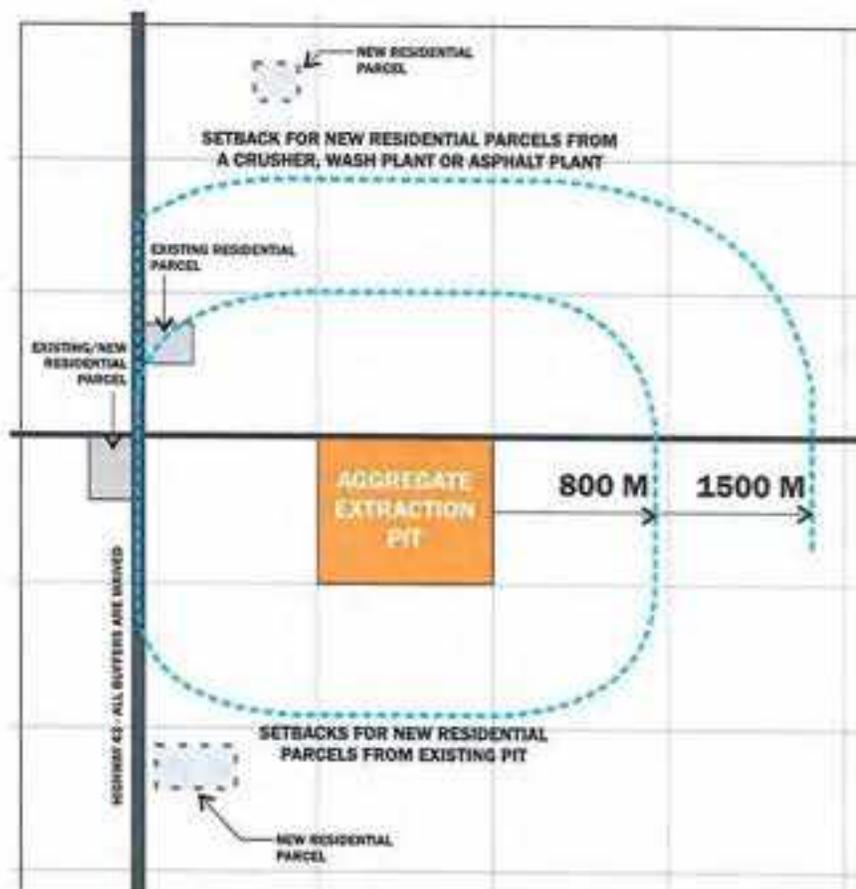


Figure 11.1.1 Setbacks from Aggregate Resource Extraction

(Upon receipt of scaled drawings of existing sites, the minimum separation distance can be determined. Measurement is related to the distance that a new dwelling can be from an aggregate area.)

Land Use By-Law 24-2014

10.2.19 Development shall not be allowed to detrimentally affect natural features such as non-artificial ponds, streams, wetlands and forested areas, but shall preserve and incorporate such features into the site design so that their key functions can be maintained. In addition:

- a) Development of, or in proximity to, wetland areas shall only be undertaken where:
 - i) It minimizes alterations in the natural flow of water, including surface and groundwater sources, which nourishes the wetlands; and
 - ii) It protects wetlands from adverse dredging or infilling practices, situation, or the addition of pesticides, salts, or toxic materials.
- b) The location of natural features and the site's topography shall be considered in designing and siting of all physical improvements.

10.2.20 Developments must adhere to the following land management practices:

- a) Stripping of vegetation or grading shall be done in a manner that will minimize soil erosion by ensuring that the extent of the disturbed area and the duration of its exposure is minimized, and that all grading work should be designed to blend with the natural contours of the land;
- b) Natural vegetation shall be retained and protected whenever possible;
- c) Natural drainage patterns should not be disturbed and changes to watercourses shall be avoided except where controlled improvements are warranted subject to approval from AESRD; and
- d) Developments shall not adversely affect groundwater resources or increase storm water runoff velocity in a way that water levels on other lands are substantially raised or the danger from flooding increased.

This report should address the County of Lac Ste. Anne requirements for a biophysical assessment. The investigation also provides information that would be useful for regulatory determinations under the *Water Act*, the *Public Lands Act* and other applicable acts and policies as outlined above.

3.3.5 Code of Practice for Pits

Gravel pits 5 ha (12.5 acres) or larger on private land are regulated through the Code of Practice for Pits. The Code of Practice outlines the requirements for all phases of pit development, operation, and responsibilities. Registration holders are required to meet the requirements of the code of practice and must obtain written permission from the current land owner prior to any activity or access to the pit. A new land owner means new written permission must be obtained. The end use of the pit must be discussed with the land owner to ensure they concur.

The code of practice outlines the requirements to be met in order ensure compliance with current environmental legislation and approvals. Setbacks from unstable slopes, drainage areas and

sensitive areas are specifically addressed. A copy of the code of practice has been provided as reference in Appendix C.

4.0 CURRENT AND HISTORICAL LAND USE

4.1 Site Visit

A site visit was completed in August, 2015 by Melinda McLauchlin of MCA Environmental Management. Observations were made from the road side on the east and south borders of the property and from on site in the north and central portion of the site. The site is bounded on the east by range road 20 and further east by a gravel pit and Kilini Creek. The gravel pit to the east of the site goes north for another quarter section. To the south is township road 540, and agricultural lands and further south more agricultural land. To the west is agricultural land and more gravel extraction to the north and west of the site. The activities in the surrounding area is predominantly gravel extraction and smaller acreage sized developments.

The site was accessed from the west boundary of the property. The access road provides access to a new residential unit near the center of the west side of the site and continues north to a laydown area for heavy equipment.

The site is discussed in five general areas for ease of reference. North ravine, laydown area, former gravel pit, central crop and south farm land.

The North ravine is a steeply sloped forested corridor with the signs of a dry creek bed at the base. There are access trails cut through the forest along the top of the ravine on the north side. The current owner indicated that past owners had cut the trails for hiking or horseback riding. The ravine appears to provide surface water drainage across the northern portion of the site from west to east. Drainage from the west contributes to the ravine flows which ultimately end up at Kilini Creek in the section to the east.

There are berms just south of the forested area on the top of the ravine on the south side (Figure 2). The current owner said he was told the berm was created from topsoil or overburden taken from the gravel pit in the northwest.

The laydown area in the northwest of the property has been cleared in order to create visual barriers in the form of berms and therefore most of the productive soils have been scraped into the surrounding berms. This has opened the site up to a range of weed species. The equipment storage area contains a range of heavy equipment, culverts metal towers or scaffolds, small sheds for additional storage and various other materials. There are a few piles of debris in the form of wood and sheet metal noted on site as well.

The former gravel pit is located north of the laydown area and was previously accessed from the north quarter. Access to the gravel pit from the north crosses a shallow portion of the ravine that

may have been previously filled. A large diameter culvert was observed with some flowing water draining to the east. Access to the site from the north was blocked by large concrete barriers put in place by the current owner. The gravel pit appears to have been mined out with only fine grained materials observed on site.

The central portion of the site is currently canola crop. A new residence was observed in the central west portion of the site just south of the laydown area. There were no other structures or storage of materials or equipment noted in the crop portion of the site.

The southeast portion of the quarter shows two privately owned acreage properties as shown on the survey plan in Figures 3 & 4.

4.2 Historical Aerial Photo Review

A random sample of historical aerial photos was selected for examination based on the best available scale, variation in the time of year, and level of coverage. The earliest photo on file at ASRD was 1949 and the most recent was from 2011. An aerial photo from Wescott Consulting was examined from 2012 as well. The following paragraphs provide a summary of relevant site observations. A table of the aerial photo review is provided as Table 1.0.



Figure 5: Aerial photo gravel extraction 1967



Figure 6: Aerial photo showing gravel pit on site in 2000

Table 1: Aerial Photo Review

Year	Observations	Scale
1949	No time of year indicated but leaves are visible. The north side of the ravine is sparsely treed. From the Ravine south the majority of the central portion of the quarter section appears fully cleared and cultivated but growth is not yet evident (Spring?). The south end and southeast is still forested and there is a farm located to the southeast. There is a building or something at the central portion of the west side of the quarter. The majority of the site appears well drained Township road 540 is fully developed to the south and range road 21 is fully developed to the east. No sign of gravel extraction in the surrounding quarters. Agriculture dominates the surrounding area.	1:40,000
1967	(August) Date on the photo is August. The north side of the ravine appears to be infilling with vegetation. The site is cultivated with little change from 1949. The central west portion of the site has a cluster of trees where an anomaly appeared in 1949. In the centre of the quarter there is a disturbance visible and may be equipment storage. The farm in the southeast now appears to have two separate driveways and maybe two farms. Two entrances are on the southeast and the other is on the east side. There is gravel extraction east of the site in the adjacent quarter. Agriculture is on all other surrounding lands.	1:31,680

Year	Observations	Scale
1976	(September) Site is fully cultivated and very little change on site is evident from previous photo with the exception of increased density of trees on the north side of the ravine. The gravel extraction to the east appears to have expanded north and there are several settling ponds adjacent the Kilini Creek.	1:20,000
1984	(June) The site is cultivated. Looks like there has been some clearing of trees on the south side of the ravine at the north end of the site to expand cultivated lands. There is a wetland at the east central portion of the site visible at this scale and very clear. There is extensive sand and gravel extraction to the east still and now in the quarter to the north.	1:15,000
1992	(May) The same cultivated lands are still visible and there appears to be more clearing at the south end of the site around the farm. The north and east side of the ravine shows signs of an extensive trail network in the forest. The gravel extraction from the north has dipped into the north west corner of the quarter on the north side of the ravine.	1:10,000
2000	(August) The quarter is cultivated. There is a gravel extraction in the northwest corner on the south side of the ravine and access to the pit is from the north. A crossing over the creek/ravine has been constructed from the north. Clearing for the gravel extraction area appears to have created stockpiles of soil near the middle of the quarter at the north end adjacent the ravine. Long berms are evident and they appear to be vegetated. The wetland at the east side of the quarter is visible as a depression but there does not appear to be open water. Many of the trees in the southeast around the farm have been cleared and it appears there are two distinct access routes to the farm in the south east one from the south and one from the east. There appears to be another residence in the central part of the south side of the quarter. The residences are separated by cultivated and cleared lands where previously there was forest. The gravel pit to the east of the site appears to be	1:20,000
2005	(May) The gravel extraction in the northwest corner is still visible. The majority of the land is still cultivated. The wetland re are rows of round bales visible. No sign of wet area in the north anymore.	1:20,000
2011	(Time of year unknown) Fully cultivated. No signs of wet area but vegetation is evident in the low area. Gravel pit does not appear to be in use.	1:20,000

Aerial photos reveal that the site has been cultivated since prior to 1949 and gravel extraction in the quarter to the east was observed as early as 1967. The ravine across the north boundary provides a history of intermittent drainage to the east. Observations also seem to indicate that the wetland in the east portion of the site may once had been a class V with open water however field observations in August of 2015 show cattails dominate the vegetation across the entire low area. Periods of low precipitation have been evident this summer impacting wetlands, local agriculture and crops in Central Alberta.

5.0 THE EXISTING ENVIRONMENT

5.1 Climate and Air Quality

The proposed development site lies very near the division between the Dry Mixedwood Sub-Region of the Boreal Forest Natural Region of northeastern Alberta, and the Central Parkland Sub-Regions (Alberta Environmental Protection, 2006 Natural Regions and Sub-Regions of Alberta (map)).

The climate is sub-humid, continental with short, cool summers and long, cold winters. The mean May - September temperature within the Sub-Region is about 11-13 degrees celcius and the growing period is about 80 - 90 days. Annual precipitation averages about 380 mm with June and July the wettest months. Winters are relatively dry with about 60 mm of precipitation.

The most frequent wind direction varies minimally over the year. Wind directions are mainly from the northwest except during February, April, July and December when they are predominately from the west. Monthly wind speeds are fairly consistent throughout the year (9.3 to 11.9 km/hr.).

Climatic data from Environment Canada was compiled for the 30 year period, 1978 to 2008, from the Edmonton Stony Plain Station; located approximately 10 km southwest of the Property.

Over the period 1978 to 2008, the average monthly temperature for the winter months (December to February) was -9°C with extreme monthly temperature fluctuations (-17.5°C to +39°C) over the 30 year period. Average monthly winter precipitation is 19.15 mm; 92% of which consists of snow.

The average monthly temperature for the spring months (March to May) was 4.3°C; increasing from a March average of -2.7°C to a May average of 10.8°C, with extreme (average) monthly temperature fluctuations of -11°C to +20°C over the 30 year period. Average monthly precipitation is 33 mm; 67.6% of which consists of rain.

The average monthly temperature for the summer months (June to August) was 15.8°C; with mean minimum and maximum temperatures of 10°C and 20°C, respectively, over the 30 year period. Average monthly precipitation is 85 mm; all of which consists of rain.

The average monthly temperature for the fall months (September to November) was 4.1°C; decreasing from a September average of -11°C, an October average of 5.6°C, to a November average of -4°C over the 30 year period. Average monthly precipitation is 32 mm; with 97% falling as rain in September, 54% as rain in October and <1% as rain in November.

Gravel extraction related industrial activities would be expected to contribute to local air emissions. Higher particulate air quality levels may occur on a temporary basis as a result of agricultural and industrial (e.g., construction, road dust, fires) activity. Vehicle exhaust fumes may be another source of minor air pollution, given the proximity to populated areas and roads.

5.2 Physiography and Topography

Dominant Landforms of the Dry mixed wood region include undulating plains and hummocky uplands.

The slope of the Property is generally trending to the southeast. At the north end of the site the land drops steeply into the Ravine that bisects the northern portion of the quarter from west to east. The base of the ravine shows evidence of an intermittent drainage course flowing to the east. The 1:50:000 topographic map sheet 83G/9 shows prior to any manmade intervention, elevations on site ranged from 740 in the northwest to 720m in the southeast.

One of the lowest drainage areas on site is near the wet land on the east border of the property. This low area acts as a minor collection point for surface water drainage during spring melt and major precipitation events.

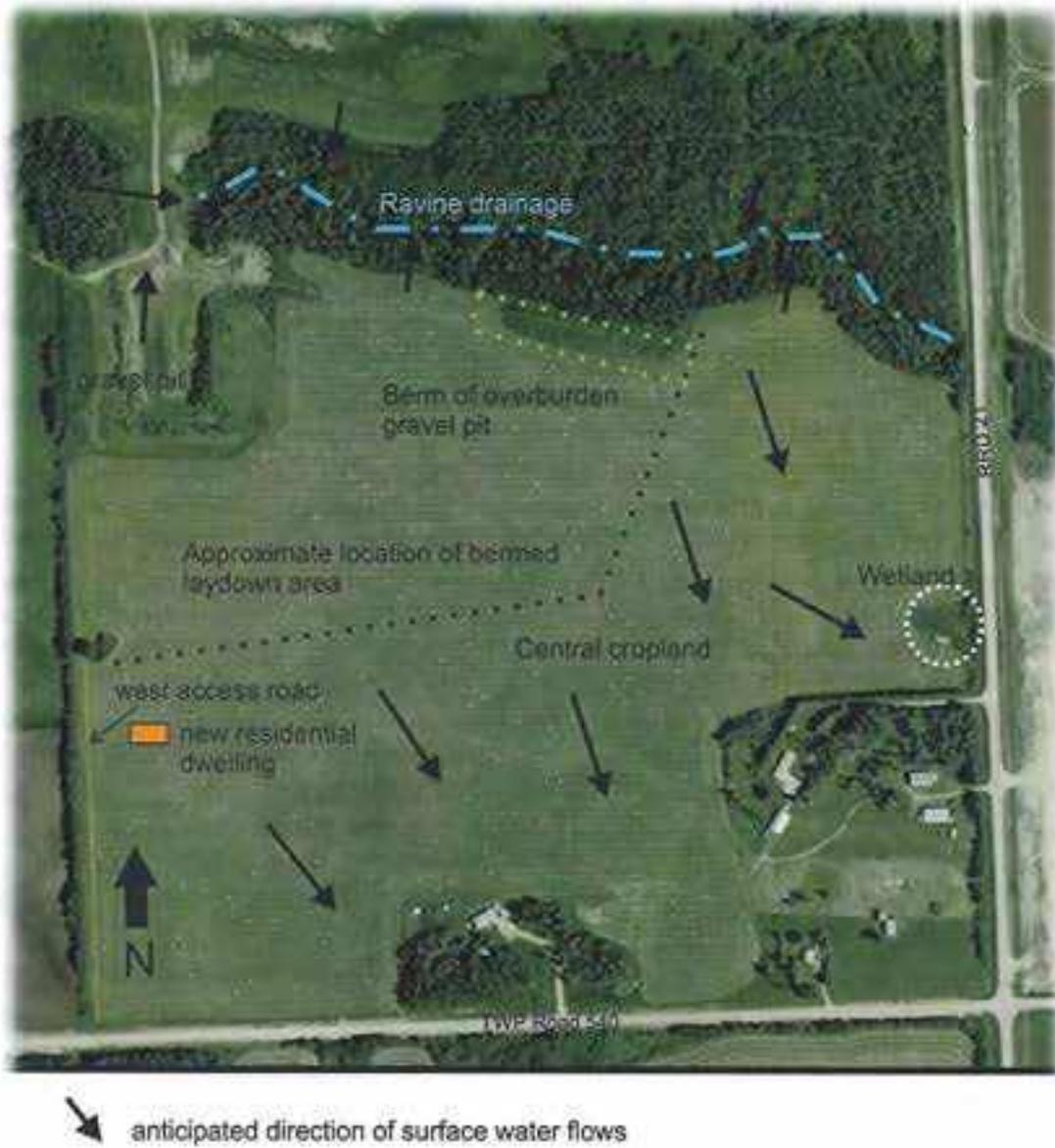


Figure 7: Site Drainage (2011 Aerial photo)

On-site topography was recently confirmed through a survey completed by Airborne Imaging. The site surveys indicate drainage is toward the south and southeast towards areas of lower elevation on site. A copy of the survey figures is contained in Appendix B.

5.3 Geology and Soils

Surficial materials are dominantly medium to moderately fine textured, moderately calcareous glacial till that may be a thin (less than 2 m) blanket over bedrock in some of the low-relief plains. In the eastern part of the Natural Subregion, about 15 percent of the area is covered by glaciolacustrine and glaciofluvial sediments occurring as inclusions within the till plains.

The AGRISID database search was completed for the parcel of land. The database listed the central portion of site as polygon 19125 WSR16/IUh. Based on published information within Agrisid, native soils on site are expected to consist of Orthic Gray Luvisol (CL, SCL, SiCL) on moderately fine textured sediments deposited by water. This area also includes Chernozemic soils (16).

The land suitability rating is listed as 2H10 which indicates the site is 80% class 2 soils sub-class H, and 20% class 2 sub-class W. As mentioned previously the soils on site are considered productive for agricultural land.

5.4 Surface Water

5.4.1 Drainage Patterns on the Property

Surface water drainage on site appears to follow topographic contours directing overland flows toward the southeast portion of the site. The ravine provides periodic drainage service for the regional set to the north and west.

From the recent contour data it appears that generally the surface drainage flows in a south to east direction however local drainage does accumulate in the east border of the site as a small wetland. Over time the wetland appears to have transitioned from an open water wetland to more of a depression. This may be the result of lower than average ranges of precipitation during 2014 and 2015.

5.4.2 Drainage around the property

The closest mapped water body within 5 km of the site is Kilini creek, located approximately 2 km to the east. Based on a review of elevations on the topographic map sheet for the area it appears that the general regional trend of surface water drainage is to the north and east of the subject site towards Coal Lake. Highway 13 shows a definite drop in elevation as it moves east from Lac Ste. Anne. This tends to be reflected in the contours on the map sheet of the area as well

5.5 Groundwater

A search of the Alberta water well database revealed six groundwater well records for the SE 1-54-2 W5M. Based on an overview of the six records on file groundwater could be encountered at depths of approximately 8.5 meters to 20 meters below ground level. A summary of the wells reconnaissance report is listed in Appendix G.

5.6 Vegetation

The biophysical assessment was conducted in part to describe and interpret vegetation communities as they existed at the time of the field reconnaissance, which took place in late August 2015. Most species of plants are visible and identifiable at that time of year but because they are not all in flower some may have been missed. In some cases, while the genus of the plant was recognized, it may not have been possible to identify it to species. In addition the slope on the south side of the ravine was too steep to closely inspect vegetation.

This section provides a description of the vegetation communities observed on the site in the field reconnaissance, preceded by a general description of vegetation in a regional context

5.6.1 Regional Context

The Property is located within the Dry mixed wood Natural Sub-region. This sub-region is characterized by warm summers and a long growing season. Aspen forests with mixed understories are normally found in the uplands and fens with sedges or shrubs commonly found in the low areas

5.6.2 Site Context

The following basic types of vegetation community were observed on the study site:

Central crop land – one new residential house, canola and various weeds such as Canada thistle, scentless chamomile, and

Ravine – the ravine consists of an Aspen and Balsam forest ranging in size from 2-3 cm to 40 cm at breast height (ABH). A few white spruce were noted in the base of the ravine. Understory species included a combination of beaked hazelnut, highbush cranberry, wildrose and bearberry with sasparilla, horsetail and fern. Ground cover included strawberry and grasses.

North side of the Ravine – the north side of the ravine has a gradual slope where the south side is very steep. Both sides are forested but understory vegetation appears more dense on the north side likely due to the more gradual and stable slope. There is a range of trails on this side of the ravine providing access east and west across the top of the ravine as well as offshoots that provide access south into the ravine base. There is a good range of size in Aspen and Balsam poplar from 2-3 cm up to 30 cm ABH. Suckering of the trees is evident into the trails and open cut areas. The understory here is similar to the lower ravine but includes Saskatoon berry, as well. No spruce was observed on the upland sites on either side of the of the ravine. For the most part, the trees in this stand appeared to be vigorous, without signs of excessive or abnormal dieback, breakage or signs of disease. There were some snags and hanging branches and some trees showing signs of fungus which did not seem abnormally represented. The canopy seems robust and healthy.

Northwest side of the ravine – there is an open area just north of the forest where the grade and elevation of the area is higher than the surrounding. It is possible that topsoil or subsoils were located in this area when gravel was extracted. The site is now occupied by willow, purple pea vine and low brush species.

Northwest laydown area – extensive weeds such as thistle, scentless chamomile, meadow hawkweed, dandelion, common tansy and leafy spurge.

Wetland – cattails, sedges, and willow.

5.7 Wetlands and Other water bodies

There is one wetland in the east central portion of the site where historically it appears surface water periodically collects and pools. The vegetation on site includes cat tails, grasses sedges and willow. The wetland was not closely assessed on foot for additional detailed species list. Historical aerial photos show open water during past years and the presence of *Typha latifolia* (cattails) around the perimeter of the site would indicate a class V wetland. The lower levels of precipitation this year however appear to have reduced the amount of open water and cat tails appear to be filling in toward the middle of the wetland thus it could be considered a class IV.

5.8 Wildlife

5.8.1 Regional Context

The subject Property is located in the Dry mixed wood sub-region of Alberta. Bird species typically inhabiting forested areas in the area include, warblers, like Black-And-White Warblers, American Redstart, Song Sparrow, Northern Water Thrush, Fox Sparrow and Philadelphia Vireo.

Some of the species listed in this sub-region include, Canadian Toad, Northern Leopard frog, red-sided garter snake. Also expected in the area would deer, coyote, cougar, and moose.

5.8.2 Wildlife on the property

With the exception of birds, warbler and woodpecker, wildlife was not observed on the property. Signs of scat from coyotes and ungulates were observed in various locations. The Fish and wildlife internet mapping tool was used to generate a list of species expected in the area. Animals in the area include Canadian Toad, Red Sided Garter Snake and Northern Leopard Frog. A copy of the results is provided in Appendix E.

6.0 BIODIVERSITY

6.1 Species Richness

Site observations were made in August, when flowers were still evident and in some cases seeds. The three areas on site offer very different ecological settings. The central agricultural lands under crop provides little variation in ecological setting and therefore low biodiversity.

The ravine presented a wide range of species from the upland slopes down into the base of the ravine. There is a good amount of succession evident in the deciduous tree species with diameter at breast height ranging from 3 cm to 40 cm. The understory is well developed with a healthy ground cover presenting a wide range of species including fungi and lichen. There are several fallen logs and leaners left to decay offering a suitable habitat for a range of plant and animal species.

The wetland to the east is somewhat isolated from the ravine and is bordered by the cropland. There is a bit of a buffer around the wetland where some transitional vegetation such as willow are evident.

6.2 Species at Risk

No rare, threatened or endangered species of plants or animals were observed during the field surveys however site visits are limited by the time of year and the extent of the survey limited to a one-day visit.

A database search for rare, endangered or listed species by the Alberta Conservation Information Management System (ACIMS) was completed for this Property. In its response, ACIMS showed no sensitive occurrences noted in the study area. Nonsensitive occurrence's included *Cladonia stygia* commonly referred to as reindeer lichen. No provincially protected areas (PPA's) were returned by the inquiry. A copy of the ACIMS generated report is provided as Appendix D.

In respect to animal species, there were no reported occurrences received from the ACIMS enquiry. This information indicates that the site has little potential to provide habitat for sensitive species.

6.3 Threats to Biodiversity

In general, disturbance to, or loss of undisturbed vegetation translates into loss of ecological habitat niches, resulting in a decline in biodiversity. The more sensitive and the less resilient the predominant vegetation community is, the greater the risk of biodiversity loss in a given area. The ravine provides a unique and meaningful area for biodiversity and an important natural corridor for wildlife to travel safely and take cover.

Risks to biodiversity include the following:

- Soil erosion and instability near the top of the ravine bank,
- Weed infestations, and;
- Changes in run off patterns which could affect the trees and other vegetation through desiccation or flooding or;
- Run off patterns that would divert drainage from the wetland at the east central portion of the site.

7.0 SUSTAINABILITY

Ecosystem sustainability can be impacted by several variables such as, the size of the system (e.g., the length and width of a stand of trees), the health and sensitivity of the soil, land slope angles and aspect, soil drainage and supply of moisture, the depth of the water table and supply of groundwater, and the exposure and susceptibility of the system to the extremes of weather. The ravine at the north end of the site provides a setting for a sustainable ecosystem to exist. The preservation of buffer zones around the top of the Ravine will help maintain structural stability of the ravine as well as provide a valuable drainage area and support diversity in the overall ecosystem for the long term. The maintenance of the feature as a drainage route will also help preserve the ecological setting. The ecosystem has evolved to act as a drainage route during higher than usual precipitation events, meltwater and heavy overland flows.

The crop land in the central area does not represent a sustainable natural ecosystem as it has been cultivated since prior to 1949. The wetland at the east border of the site has appeared over time to act as a natural drainage feature which is protected through legislation. The depression in that area appears to have recently been impacted by lower than average precipitation reducing its visible surface water size, however vegetation observed on site indicate that it is a permanent pond and should be considered a class V wetland. The wetland has the potential to expand during years of increased precipitation. The sustainability of a wetland is dependent on the maintenance of the surrounding lands that contribute drainage to the site. The key concern in future development of the central portion of the site will be to design storm water management plans in such a manner that post-development surface water flows are approximately similar in flow rates and periodicity to what was prevalent in the pre-development condition.

7.1 Ecological linkages

In the central parkland ecological region, ecological connectivity has been impacted by the extensive agricultural and resource extraction land uses. Linkages that do exist rely mainly on forested areas, river and creek valleys which provide visual and thermal cover for many forms of wildlife, as well as nutritional support. Moose and deer, for example, move readily along corridors of forest, because it provides them with visual cover from predators, as well as providing them with a source of food in the form of leafy vegetation. Although they venture out into the open to travel or to access other sources of food, deer prefer to stay within about 180 m of forest cover. Extensive forest areas, therefore, serve as movement corridors for these animals.

Similarly, birds use forest corridors for habitat, visual and thermal cover and the acquisition of food, and because they can fly, easily disperse among patches of forest on the landscape. Mammalian wildlife such as moose, deer, coyotes, furbearers and even amphibians, also use patches of treed areas to disperse with greater safety. These adjacent patches are referred to as "stepping stones", while long bands of forest are called "corridors". Patches of forest that is sufficiently large to support all of the life-cycle activities of animals are equally important. Wetland habitats are also important as stepping stones to habitat with secure brood water. Generally, smaller temporary wetlands are used by waterfowl to disperse pairs into nesting habitat on the landscape, thereby maximizing breeding opportunity. However, after nests have hatched broods are often moved to more secure water until young have fledged.

Amphibians, generally, require upland sites in which to feed and over-winter, but must return to water to reproduce. The distance between wetlands or other waterbodies, and suitable undisturbed upland sites becomes critical when land developments threaten amphibian habitats of one type or another.

In the broader context, aerial photos of the property and surrounding lands as they appeared in 1949 and 2011, shown in Figures 4 and 5 respectively, indicate that the ravine has offered sustainable ecological linkages to adjacent areas.



Figure 8: Potential ecological linkages, based on wooded areas and wetland/thicket areas in the vicinity of the project site (imagery from Google Earth, 2011). White arrows indicate potentially "permeable" wooded corridors or stepping stone patches amongst core habitats (implies travel in both directions). Orange line indicates boundary of the study site

7.2 Mapped and Classed Waterbodies in the Area

The Alberta Environment classification system for waterbodies in Alberta is based generally on the abundance and quality of fish habitat. Class A is the highest priority class, and class D is the lowest (except for "unmapped" watercourses which are not shown on the maps at all). Any mapped waterbody must follow the Code of Practice for Watercourse Crossings, which falls under the *Alberta Water Act*. Any classed waterbody must also observe a Restricted Activity Period specified for it, for conducting in-stream construction or related activities.

The only mapped waterbody within 5 km of the site is Kilini Creek, which is 0.8 km east of the subject site. Drainage from the ravine is expected to flow to the east toward Kilini Creek.

8.0 RECOMMENDATIONS

The following recommendations are made for consideration in the possible future development of residential lots at the site for design, construction and post-construction phases. If gravel extraction is to be pursued at the site the specific requirements for pit development, operation and management can be found at Alberta Environment and Parks. A copy of the Code of Practice for pits has been included as Appendix B as part of this report.

8.1 Residential subdivision design

To the extent feasible, the natural contours of the land should be considered in order to conserve the natural drainage patterns and flows, and to moderate storm water drainage patterns, thus damping out extremes of overland flow, avoiding erosion and promoting the settling of solid particulate matter.

Gradual slopes and vegetated swales should be incorporated in the contouring and landscaping of the development in the flat areas surrounding the laydown area. By slowing down the runoff, this provides more opportunity for particulate matter to settle, and nutrients to be removed.

8.2 Opportunities for habitat conservation

The ravine and surrounding upland should be protected with a development buffer. Opportunities to enhance habitat around the subdivision development can be taken through the addition of trees as part of site restoration and landscaping efforts.

Invasive weedy plants should be prevented upon construction completion through the use of re-vegetation strategies where applicable. The use of native grasses will reduce future management requirements if pursued immediately upon construction completion.

8.3 Surface water and groundwater

In addition to any Lac St Anne MDP requirements, a master drainage plan as per the Storm water Management Guidelines for the Province of Alberta would be beneficial as part of the overall storm water surface water management plan for any developed area on the Property. The plan should aim at maintaining overland runoff at approximately pre-development conditions, to reduce the risk of flood on adjacent properties as a result of altered runoff patterns. Consideration of changing weather systems as a result of climate change should be incorporated into future designs. This may require anticipation of major rainfall events and related impacts on surface water drainage.

Preservation of the existing wetland on the east central portion of the site should be made a priority by preserving existing drainage to and around the area.

The subdivision should be designed to achieve effective settlement of suspended solids in accordance with the current Alberta Environment guidelines (see above). Otherwise, discharge points from the ditches should be into a vegetated or other area where energy dissipation will occur, or the equivalent, in order to prevent local erosion. Where necessary, roads and their

ditches should have flow-limiting structures (e.g., ditch blocks) along sloped stretches so that water draining off or along them does not cause erosion.

Surface paving should be minimized, other than what is necessary for transportation, parking and other infrastructure in order to maintain permeability and thus groundwater re-charge.

Where excavated, topsoil should be conserved and stockpiled with protection from wind and water erosion, and then replaced after construction, being careful not to mix topsoil and subsoil layers

Where any clearing or earthworks are necessary, strict erosion control and reclamation measures should be taken. Contouring should avoid significantly increasing the angle of slopes, such as would lead to erosion and instability.

In order to conserve water landscaping should be completed through the use of native, drought-tolerant plants. This will reduce the need to water plants and reduce the need to use chemical fertilizers.

8.4 Environmental sustainability and Community Participation

The use of fixtures that reduce light pollution, especially avoiding projecting light into natural areas. Fixture design and placement should ensure that most of the light produced should be projected downwards, rather than laterally into the surrounding environment. The use of long wavelength bulbs should be encouraged, as they have less impact on wildlife.

8.5 Ecological linkages

The ravine located on the north end of the property offers important ecological linkages to the properties to the east and west of the site. The ravine provides a movement corridor, food and shelter for wildlife. The Lac St Anne MDP has identified this creek as an important waterway with a 70 meter set back buffer.

8.6 Hazards Wastes and Disturbances

The laydown area in the northwest contains heavy equipment such as graders, backhoes, forklifts, trucks and trailers. There are also wooden rig mats, metal towers, plastic containers, metal culverts, and smaller storage sheds. Various piles of debris on site appeared to contain metals and wood. There is also an ATCO trailer on site. Should the land use in this area change it may be necessary to complete a phase I ESA to verify the environmental integrity of the area as this land use offers the potential to cause contamination as a result of leaks or spills from equipment and storage.

8.7 Gravel Pit Reclamation

Prior to gravel pit reclamation the registration application must be accepted by the Alberta Government and the plan deemed acceptable. Access to the site will need to be coordinated with the current land owner. Due to the ecological significance of the drainage area and the extremely

steep slope, adequate buffers from the top of the ravine must be maintained. The site reclamation plan has been outlined in the Pit registration application mentioned in section 2.1.2.

9.0 CONCLUSIONS

Due to the agricultural activities on site, the majority of the upland property presents a lack of biological diversity with poor range of habitat available for plant and animal species. There do not appear to be any biological restrictions to the development of this area.

The ravine across the north end of the property does provide an ecologically diverse setting that provides valuable habitat and cover for a wide range of species. The area serves as a movement corridor providing cover and habitat as well as an ecological linkage across property borders. This area should be protected from future development through the maintenance of setbacks and buffers. There were no species at risk noted for the property.

There is one small wetland located on the east border of the property that has been evident in aerial photos since 1949. The wetland is a naturally occurring low area and serves as a catchment for the north and east side of the property. Drainage toward the wetland should not be significantly modified so as to allow for the wetland to continue to thrive and provide a catchment for local overland flows.

The subject property has been in use for agricultural purposes since at least 1949 and consists of productive agricultural land in the form of a class 2 soil. While there are no specific limitations related to the development of the land for wildlife habitat, regulators and the developer will likely recognize that development of the site will result in a decrease to local productive agricultural land. This can be considered a common consequence of development however it should be planned around compatible land uses at appropriate times.

Recommendations for future studies prior to the development of the land include the following.

- Master drainage plan and erosion control strategies
- Fire prevention and control strategies
- Site hazard management planning
- Gravel extraction planning and timing of activities

These recommendations are not intended to be exclusive. Regulators may have additional requirements not listed here. Reference should be made to the recommendations provided in this report along with various other planning tools during future site development including the municipal Development Plan.

10.0 CLOSURE

We trust this report meets your current requirements. If you require any additional information please do not hesitate to contact the undersigned.



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APPENDIX A



Photo 1: Southeast corner of the property has a sign warning against the spread of weeds.



Photo 2: North east side of the laydown area in the northwest corner of the property. Various weed species present due to clearing of topsoil.



Photo 3: Berm surrounding the laydown area provides a visual barrier between cropland and weeds.



Photo 4: Forested area at the base of the ravine where evidence of cut trails exist.



Photo 5: Typical understory in the base of the ravine.

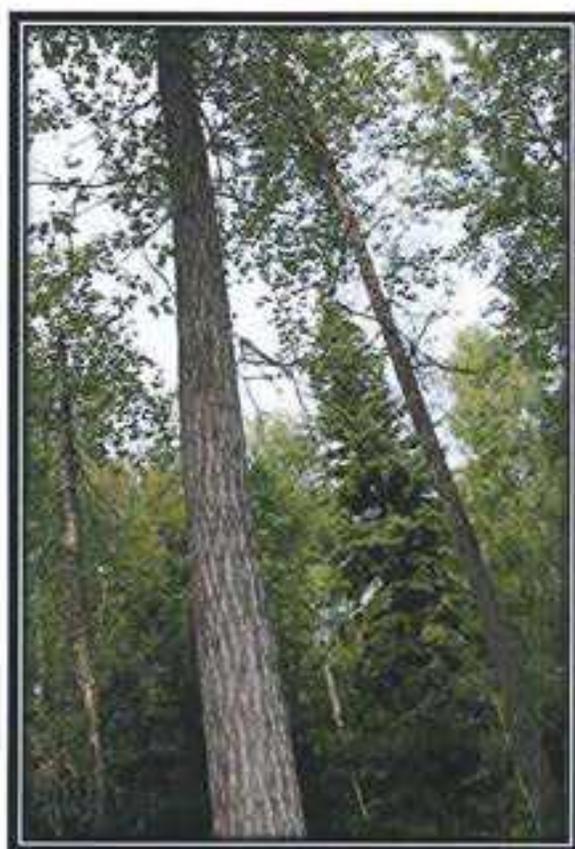


Photo 6: Larger diameter trees in the base of the ravine ~45 cm ABH



Photo 7: Creek in this portion of the ravine did not have running water evident. Portion of the creek in the base of the ravine.



Photo 8: Healthy detritus in the base of the ravine provides valuable habitat.



Photo 9: Sample of the understory in the ravine on the north side of the drainage course.



Photo 10: North side of the ravine part way up the hill. Slope on the north side is gradual.



Photo 11: Upland species on the north side of the ravine surrounding the trail system.



Photo 12: Trees are starting to show some succession but average DBH is about 20 cm.



Photo 13: Diverse health understory at the junction of trails to the north and south as well as east and west.



Photo 14: Trail system on the north side of the ravine shows smaller diameter trees similar in diameter.



Photo 15: At the edge of the forested ravine on the north side Peavine.



Photo 16: At the northwest side of the ravine. Forest to field transition with willow.



Photo 17: Snags provide habitat.



Photo 18: Looking south at the ravine/creek crossing at the north west.



Photo 19: Water in the culvert at the crossing site in the northwest.



Photo 20: Looking north from the NW gravel pit at access road blocked by concrete.



Photo 21: Gravel pit in the northwest has not been reclaimed.



Photo 22: Equipment storage in the laydown area.

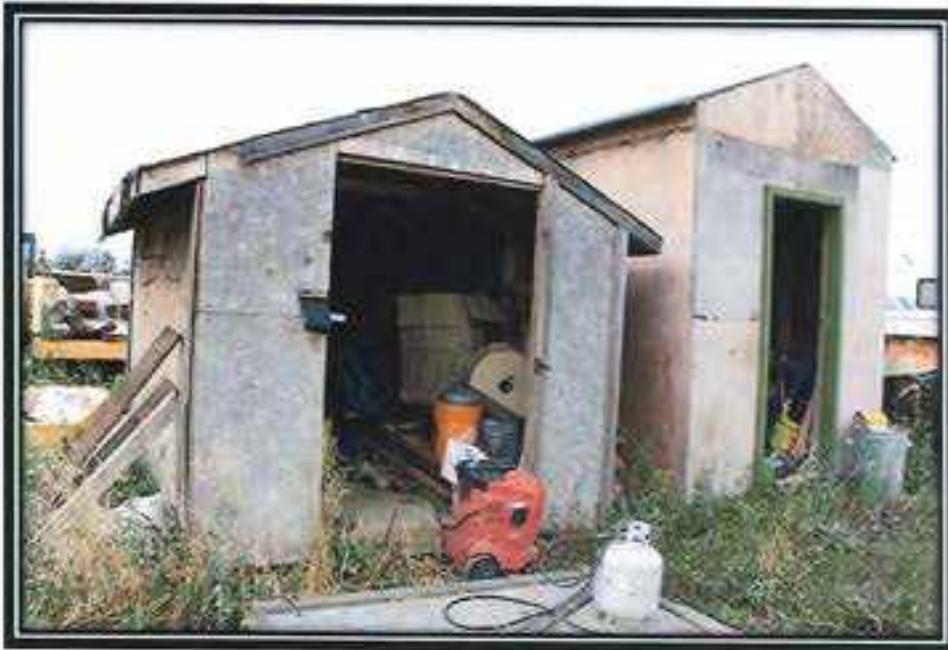


Photo 23: Sheds and material storage.



Photo 24: Looking south on the access road from the laydown area.



Photo 25: New house at the west side of the crop land.



Photo 26: Wetland at the east side of the property. Cattails are infilling.



Photo 27: Shrubs surrounding the wetland at the east side of the property.

APPENDIX B

APPENDIX B: List of plant species observed in vegetation survey of Lac St. Anne property, August 2015. Arranged in alphabetical order by scientific name

Common Name	Scientific Name	B. Forbs, Herbs and Other Vascular Plants:
A. Trees and Shrubs:		
Alder	<i>Alnus rugosa</i>	Yarrow
Saskatoon	<i>Amelanchier alnifolia</i>	Baneberry
White birch	<i>Betula papyrifera</i>	Slender wheat grass
Siberian Peashrub	<i>Caragana arborescens</i>	Rough hair grass
Red-osier dogwood	<i>Cornus stolonifera</i>	Canada anemone
Beaked Hazelnut	<i>Corylus cornuta</i>	Wild sarsaparilla
White Spruce	<i>Picea glauca</i>	Fringed aster
Balsam poplar	<i>Populus balsamifera</i>	Milk vetch
Trembling aspen	<i>Populus tremuloides</i>	Slender wheat grass
Chokecherry	<i>Prunus virginiana</i>	Short-awned foxtail
Beaked Willow	<i>Salix bebbiana</i>	Blunt-leaved sandwort
Sandbar Willow	<i>Salix exigua</i>	Brome grass
Western Shining Willow	<i>Salix lasiandra</i>	Marsh Reed Grass
High bush cranberry	<i>Viburnum opulus</i>	<i>canadensis</i>
		Shepherd's purse
		Awned Sedge
		Canada thistle
		Bunchberry
		Fairy bells
		Shield fern
		<i>Achillea millefolia</i>
		<i>Actaea rubra</i>
		<i>Agropyron trachycaulium</i>
		<i>Agrostis scabra</i>
		<i>Anemone canadensis</i>
		<i>Aralia nudicaulis</i>
		<i>Aster ciliolatus</i>
		<i>Astragalus sp.</i>
		<i>Agropyron trachycaulium</i>
		<i>Alopecurus aequalis</i>
		<i>Arenaria lateriflora</i>
		<i>Bromus spp.</i>
		<i>Calamagrostis</i>
		<i>Capsella bursa-pastoris</i>
		<i>Carex atherodes</i>
		<i>Cirsium arvense</i>
		<i>Cornus canadensis</i>
		<i>Disporum trachycarpum</i>
		<i>Dryopteris spinulosa</i>

Spike rush	<i>Eleocharis sp.</i>	Wild raspberry	<i>Rubus idaeus</i>
Fireweed	<i>Epilobium angustifolium</i>	Trailing raspberry	<i>Rubus pubescens</i>
Common Horsetail	<i>Equisetum arvense</i>	Arum-leaved arrowhead	<i>Sagittaria cuneata</i>
Swamp Horsetail	<i>Equisetum fluviatile</i>	Buffaloberry	<i>Shepherdia canadensis</i>
Meadow Horsetail	<i>Equisetum pratense</i>	Star-flowered False Solomon's Seal	<i>Smilacina stellata</i>
Fescue	<i>Festuca sp.</i>	Goldenrod	<i>Solidago canadensis</i>
Wild strawberry	<i>Fragaria virginiana</i>	Sow thistle	<i>Sonchus arvensis</i>
Narrow-leaved Hawkweed	<i>Hieracium umbellatum</i>	Bur-reed	<i>Sparganium sp.</i>
Foxtail barley	<i>Hordeum jubatum</i>	Marsh Hedge Nettle	<i>Stachys palustris</i>
Creamy Peavine	<i>Lathyrus ochroleucus</i>	Chickweed	<i>Stellaria, Cerastrium sp.</i>
Twining honeysuckle	<i>Lonicera dioica</i>	Snowberry	<i>Symphoricarpos albus</i>
Bracted honeysuckle	<i>Lonicera involucrata</i>	Dandelion	<i>Taraxacum officinale</i>
Wild lily-of-the-valley	<i>Maianthemum canadense</i>	Veiny meadow rue	<i>Thalictrum venulosum</i>
Yellow sweet-clover	<i>Medicago officinalis</i>	Cattail	<i>Typha latifolia</i>
Tall Lungwort	<i>Mertensia paniculata</i>	Stinging nettle	<i>Urtica dioica</i>
Timothy	<i>Phleum pratense</i>	Wild vetch	<i>Vicia americana</i>
Common Plantain	<i>Plantago major</i>	Western violet	<i>Viola canadensis</i>
Fowl bluegrass	<i>Poa palustris</i>	Kidney-leaved Violet	<i>Viola renifolia</i>
Wintergreen, Pink	<i>Pyrola asarifolia</i>		
Yellow Water-crowfoot	<i>Ranunculus gmelinii</i>	<u>C. Non-Vascular Plants:</u>	
Skunk currant	<i>Ribes glandulosum</i>	Sphagnum moss	<i>Sphagnum sp.</i>
Northern gooseberry	<i>Ribes oxacanthoides</i>	Club moss	<i>Lycopodium sp.</i>
Prickly rose	<i>Rosa acicularis</i>	Forest floor moss	<i>Polytrichum sp.</i>

Feather moss

Brachythecium sp.

APPENDIX C

Alberta Code of Practice for Pits

<http://www.qp.alberta.ca/documents/codes/PITS.PDF>

Guide to the Code of Practice for Pits

<http://environment.gov.ab.ca/info/library/5997.pdf>

APPENDIX D

Search ACIMS Data

Updated: July 22, 2015
 Today: September 16,
 2015

1 Select Requester: * 

2 Select Reason for Request: *

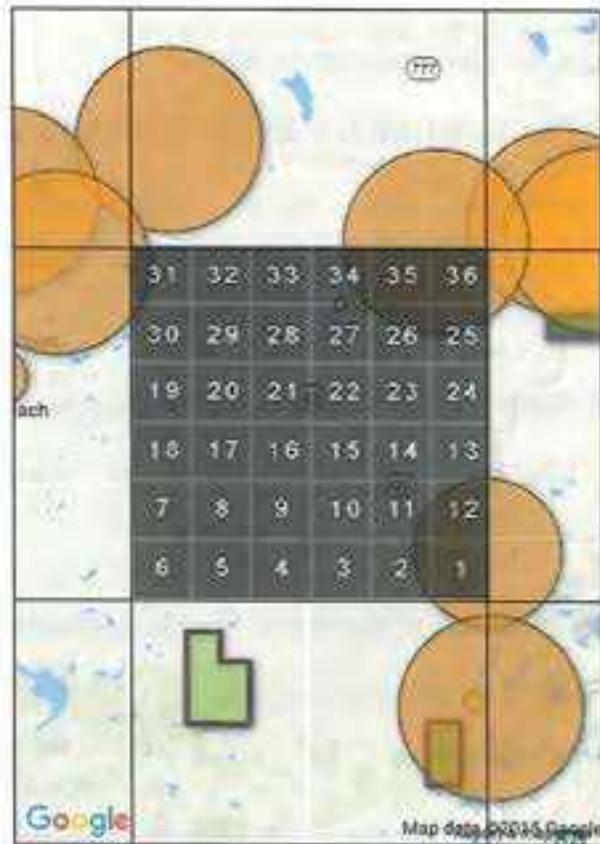
3 SEC TWP RGE MER

(option) Convert Lat/Long to Township

Layers

-  Element Occurrences (part one, non-sensitive) 
-  Element Occurrence (part two, sensitive) 
-  Protected Areas 
-  Crown Reservation/Notation 
-  100 m Proximity - Protected Areas 
-  100 m Proximity - Crown Reservation/Notation 

* Required

Table of Results [Print Preview](#)

Date: 16/9/2015
 Requestor: Consultant
 Reason for Request: Land Use Planning
 SEC: 01 TWP: 054 RGE: 02 MER: 5



 **Non-sensitive EOs: 1** (Data Updated: July 2015)

M-RR-TTT-SS	EQ_ID	ECODE	S_RANK	SNAME	SCOMNAME	LAST_OBS_D
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Table of Results [Print Preview](#)

Date: 16/9/2015
 Requestor: Consultant
 Reason for Request: Land Use Planning
 SEC: 01 TWP: 054 RGE: 02 MER: 5



■ Non-sensitive EOs: 1 *(Data Updated: July 2015)*

M-RR-TTT-SS	EO_ID	ECODE	S_RANK	SNAME	SCOMNAME	LAST_OBS_D
5-02-054-01	8059	NLTEST7140	S2S4	Cladonia stygia	reindeer lichen	1-Jun-87

Next Steps: See FAQ

■ Sensitive EOs: 0 *(Data Updated: July 2015)*

M-RR-TTT	EO_ID	ECODE	S_RANK	SNAME	SCOMNAME	LAST_OBS_D
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No Sensitive EOs Found; Next Steps - See FAQ

■ Protected Areas: 0 *(Data Updated: May 2015)*

M-RR-TTT-SS	PROTECTED AREA NAME	TYPE	IUCN
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No Protected Areas Found

■ Crown Reservations/Notations: 0 *(Data Updated: May 2015)*

M-RR-TTT-SS	NAME	TYPE
-------------	------	------

No Crown Reservations/Notations Found

REAL DIVERS: ONE PAGE

■ Sensitive EOs: 0 (Data Updated: July 2015)

M-RR-TTT	EO_ID	ECODE	S_RANK	SNAME	SCOMNAME	LAST_OBS_D
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No Sensitive EOs Found: Next Steps - See FAQ

■ Protected Areas: 0 (Data Updated: May 2015)

M-RR-TTT-SS	PROTECTED AREA NAME	TYPE	IUCN
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No Protected Areas Found

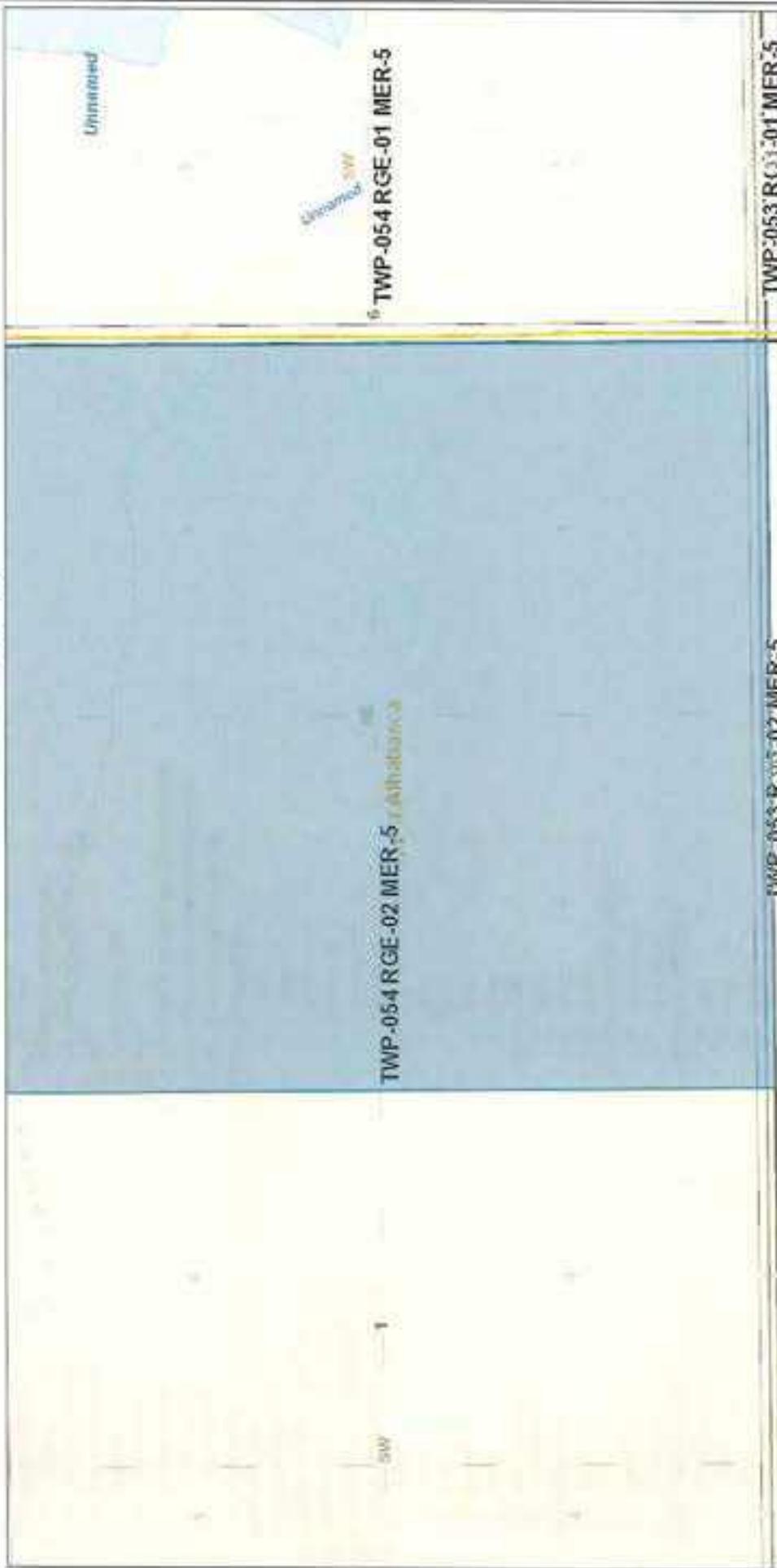
■ Crown Reservations/Notations: 0 (Data Updated: May 2015)

M-RR-TTT-SS	NAME	TYPE
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No Crown Reservations/Notations Found

APPENDIX E

Fish and Wildlife Internet Mapping Tool



TWP-054 RGE-02 MER-5

TWP-054 RGE-01 MER-5



Projection: NAD_1983_10TM_AEP_Forest

Map Scale: 5.735

Printed on: September 16, 2015 14:17:20 -06:00

Display map content: Base Map Data provided by the Government of Alberta under the Alberta Open Government License. Cultural and Disposition Data provided by Alberta Data Partnerships. GeoEye. All Rights Reserved.
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The Crown provides this information without warranty or representation as to any matter including but not limited to whether the data/information is correct, accurate or free from error, defect, danger or hazard and whether it is otherwise useful or suitable for any use the user may make of it.

This site is created, maintained, and controlled by ESRI in direct consultation with the data authority.

Legend

- Stocked Waterbodies
- FWMIS River and Stream Label
- FWMIS Lake Label
- Other Waterbody Label
- FWMIS River and Stream
- River or Stream
- Substrate Stream
- Canal
- Aqueduct
- Representational Flow
- FWMIS Major Waterbody
- Perennial Lake
- Intermittent Lake
- River
- Isolated
- Other Waterbody
- Perennial Outflow
- Intermittent Outflow
- Canal
- Wetland
- Lagoon
- Reservoir
- Deposition
- Provincial Sanctuary Corridor Wildlife
- Provincial Sanctuary Game Bird
- Provincial Sanctuary Restricted Area
- Provincial Sanctuary Seasonal
- Paved Road Label (20K)
- Paved Road (20K)
- Primary Divided
- Primary Undivided 4L
- Primary Undivided 2L
- Primary Undivided 1L
- Interchange Ramp
- Secondary Divided
- Secondary Undivided 4L
- Secondary Undivided 2L
- Secondary Undivided 1L
- Gravel Road Label (20K)
- Gravel Road (20K)
- Primary Undivided 2L
- Primary Undivided 1L
- Secondary Undivided 2L
- Secondary Undivided 1L
- Intermittent Lake (20K)
- Intermittent Lake
- Intermittent Outflow
- Sandbar / Wetland / Lagoon
- Sandbar
- Wetland
- Lagoon
- Stream (20K)
- Stream
- Canal
- Outflow
- Ditch
- Intermittent Stream / Aqueduct / Spillway
- Reservoir Outflow
- Intermittent Stream
- Arroyo Flow
- Reservoir Outflow
- Aqueduct
- Spillway
- ATS Township Index
- ATS Section with Road Allowance
- ATS Quarter Section with Road Allowance
- ATS Legal Subdivision with Road Allowance
- Town
- Urban Service Area
- City
- Exploration Restricted Area Label
- Exploration Restricted Area
- Active
- Cancelled
- Environment and Sustainable Resource Development Operations
- Secondary Undivided 2L
- Secondary Undivided 1L
- Lake/River (20K)
- Lake or River
- Reservoir
- Isolated
- Major Canal
- Outflow
- Quarry
- Dugout
- Intermittent Lake (20K)
- Intermittent Lake
- Intermittent Outflow
- Sandbar
- Wetland
- Lagoon
- Stream (20K)
- Stream
- Canal
- Outflow
- Ditch
- Intermittent Stream / Aqueduct / Spillway
- Reservoir Outflow
- Intermittent Stream
- Arroyo Flow
- Reservoir Outflow
- Aqueduct
- Spillway
- ATS Township Index
- ATS Section with Road Allowance
- ATS Quarter Section with Road Allowance
- ATS Legal Subdivision with Road Allowance
- Town
- Urban Service Area
- City
- Exploration Restricted Area Label
- Exploration Restricted Area
- Active
- Cancelled
- Environment and Sustainable Resource Development Operations

APPENDIX F

PROPOSED DEVELOPMENT
 OF
 S.E. SEC. 1-54 S1/4SW4
 WITHIN
 COUNTY OF LAC ST ANNE
 FOR
 LAUNCH VENTURES

PLAN AND/OR SHOWN
 S.C.P. / AREA 134 # 41
 NAME / BOUNDARY *****

NOTES
 1. ALL DIMENSIONS ARE IN METERS AND
 DECIMALS THEREOF.
 2. DIMENSIONS ARE APPROPRIATE AND
 DERIVED FROM A.S.I.C.A.

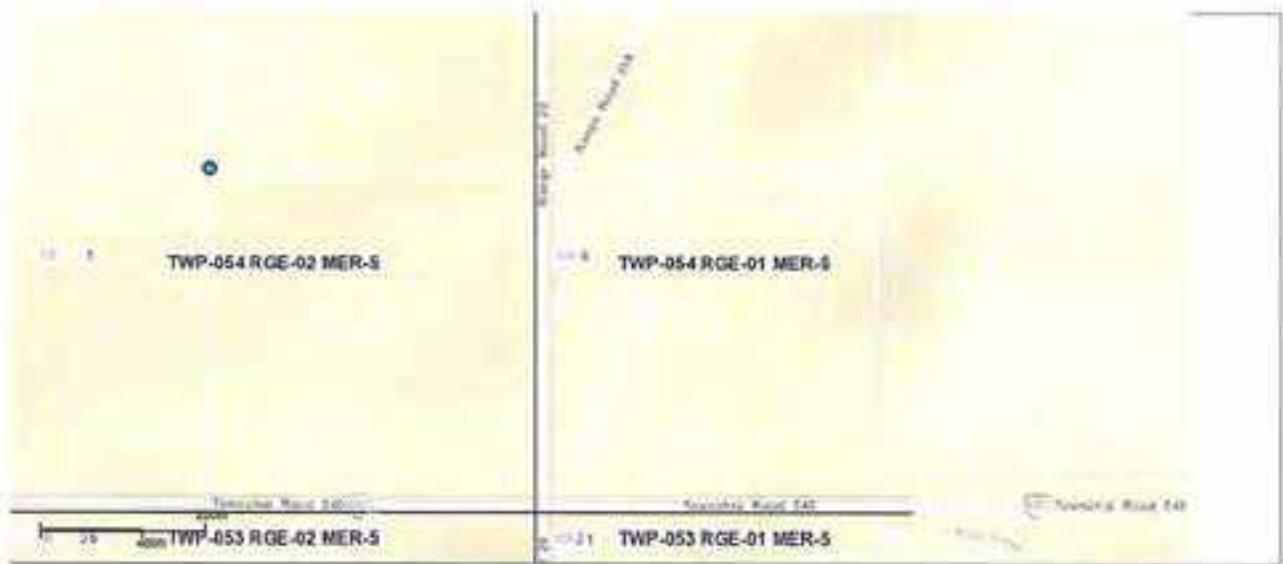
NO.	SYMBOL	DESCRIPTION
1	-----	BOUNDARY

PROJECT PROPOSED DEVELOPMENT
 CLIENT
 LAUNCH VENTURES LTD.

PROJECT TITLE
 SITE PLAN OPTION # 2
 SHEET NO.
 WINDOOST CONSULTING GROUP
 18 Progress Avenue, St. Marys, Ont. N4Y 2K1
 Tel: 705-882-1100 Fax: 705-882-1101
 www.windoost.com



APPENDIX G



Lac St Anne

Projection
Web Mercator (Auxiliary Sphere)
Datum
WGS 84
Date
9/9/2015 11:10:08 AM

Legend
 Groundwater Drilling Report
 Baseline Water Well Report
<http://groundwater.alberta.ca/WaterWells/>

Information as depicted is subject to change; therefore the Government of Alberta assumes no responsibility for discrepancies at time of use.
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Groundwater Wells

Please click the water Well ID to generate the Water Well Drilling Report.

Well ID	LSD	SEC	TWP	RGE	M	DRILLING COMPANY	DATE COMPLETED	DEPTH (ft)	TYPE OF WORK	USE	CHM	LT	PT	WELL OWNER	STATIC LEVEL (ft)	TEST RATE (gpm)	SC DIAM (ft)
362224	SE	01	054	02	5	GERALD MCGINN DRILLING LTD.	1993-01-23	105.00	New Well	Domestic & Stock		8		PARKER, CHARLES	40.00	7.00	6.00
362055	SE	01	054	02	5	UNKNOWN DRILLER		110.00	Chemistry	Domestic				DUBE, LEO			0.00
444095	SE	01	054	02	5	GERALD MCGINN DRILLING LTD.	1974-10-23	94.00	New Well	Domestic		11		PARKER, CHARLIE	38.00	7.00	4.50
444096	SE	01	054	02	5	GERALD MCGINN DRILLING LTD.	1971-05-08	106.00	New Well	Domestic		12		PARKER, CHARLES	68.00	3.00	4.50
444097	SE	01	054	02	5	GERALD MCGINN DRILLING LTD.	1983-09-30	94.00	New Well	Domestic & Stock		7		PARKER, CHARLES	40.00	3.50	5.56
405870	SE	01	054	02	5	RODICO DRILLING	1999-09-14	108.00	New Well	Domestic		11	7	DUBE, LEO	50.00	9.00	6.00

“AGREGATE & GEOTECHNICAL ASSESSMENT”

S.E. 1-54-2-5

PREPARED BY

TWERDOFF & ASSOCIATE

APPENDIX ‘B’

June 2014

Aggregate Characterization Assessment

**Prepared for:
Wes Erickson**

**On
SE 1-54-2-W5M**

**Prepared by:
TWERDOFF & ASSOCIATES INC.**

**Dennis Twerdoff, M.Sc., P.Geol., P.Ag.
Senior Project Manager**

June 2014



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1 Introduction

Twerdoff & Associates (Twerdoff) was retained by Wes Erickson determine the location and extent of potential aggregate reserves within SE 1-54-2-W5M (Figure 1). This land is surrounded on 3 sides by aggregate operations. An old pit is present in the far northwest corner of the property.

The property is located approximately 5 miles southeast of the town of Onoway, Alberta. The land is currently agricultural with areas being developed into a lay-down yard. There are also 2 subdivision lots with 3 residences on the south and southeast portions of the $\frac{1}{4}$ section.

1.1 Objective

The objective of the Aggregate Characterization Assessment was to delineate potential aggregate reserves, to assess the general aggregate quality and provide a general estimate of potential reserves.

1.2 Scope

The scope of work for this Assessment included the following principle tasks:

- Review Alberta Geological Survey (ASG) sand and gravel information for the area,
- Design a rough exploration plan,
- Attend auger drilling and log test hole lithology,
- Prepare an Aggregate Characterization Assessment report to summarize findings.

1.3 Geology

A search of surficial geology on the property was conducted. The AGS identified areas within the SE 1-54-2-W5M along with surrounding land as having Dirty Sandy Gravel with an average of 5 m of aggregate (AGS, 2014) (Figure 2).

Quaternary geology consists Stagnation Moraine. The Stagnation Moraine is characterized by till of uneven thickness, local water sorted material, it is up to 30 m thick, and is undulating hummocky reflecting variations in till thickness. Topography is undulating with local relief of up to 3 m (Shetson, 1990).

A Water Well search was conducted on the Alberta Environment Water Well Information Database website for wells located within SE 1-54-2-W5M. Five (5) water well records were returned. Well lithology on records that recorded lithology did not show any layers of sand and gravel.

Bedrock geology consists of the Horseshoe Canyon Formation of Mesozoic age (Abadata,2013).

2 Methods

2.1 Drill Program

Mobile Augers from Edmonton Alberta conducted auger drill testing from May 5 to May 9, 2014. A 6" auger was used to drill boreholes between 27 and 74 feet below ground level. A total of 20 boreholes were advanced throughout the property.

Subsurface lithology was recorded by Dennis Twerdoff, P.Geol. P.Ag.

The testhole number, location and general lithology for each test hole is illustrated in Figure 3, **Appendix A.**

A detailed description of each test hole lithology is outlined in **Appendix B.**

3 Results

3.1 2014 Drill Program

In general, testholes encountered a silty sand on the surface followed by a brown clay layer. In northwest portions of the property, a seam of sand and gravel was observed under the brown clay which seemed to pinch out into a gravelly clay south and east where topography dropped in elevation. Below this, another layer of clay was observed with a few holes drilled to bedrock.

The sand and gravel seam was encountered in the northwest portion of the property. The potentially mineable area is outlined in Figure 3, **Appendix A**.

The deposit was relatively sandy with an estimated 40% gravel content and 60 % sand content. The sand and gravel seam along the edges of the deposit were mixed with clay while the majority of the seam was quite clean. Within the heart of the deposit, the sand and gravel seam was covered with 25-37 feet of overburden and the seam was approximately 10-20 feet thick. The approximate overburden to gravel ratio was 3:1.

4 Discussions and Conclusions

There are active sand and gravel pits north, west and east of the property. Very old gravel pits are located approximately 2 miles south. The gravel pit west and northwest of the property are thought to be operated by Lafarge Canada. The gravel pit immediately north of the property is now operated by The Fath Group of Edmonton, AB (pers. comm. with pit operator).

The Alberta Geological survey identified this property as possessing sand and gravel. Water well logs on the SE 1-54-2-W5M did not reveal any sand and gravel layers within reported logs. There is an old excavation approximately 1.4 ha in size in the northwest corner of the property that appears to be largely depleted.

Test hole results indicated that sand and gravel was present in the northwest portion of the property. Typically, when sand and gravel is mixed with clay, as in the outer fringes of this deposit, reserves are not economic because it is difficult to separate the clay from the gravel using conventional processing.

The sand and gravel seam was present on the north side of the steep valley and creek that intersects the northern portion of the property. Typically, a 10 m buffer from the valley break would be required if mining operations encroached on the steep valley and creek.

The area where the sand and gravel seam was present was approximately 18 ha in size, less 1.4 ha that have been previously excavated and another approximately 4 ha encompassed by a steep valley and creek.

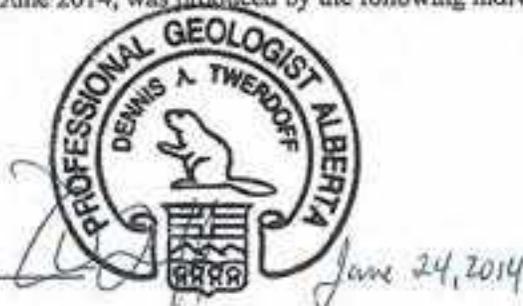
It is estimated that there is approximately 12.6 ha of area that is potentially mineable. It is estimated that there are approximately 400,000 m³ of sandy gravel reserves within the mineable area. There is an approximate 3:1 overburden to aggregate ratio.

Since Wes Erickson is not in the sand and gravel business, it is recommended that Mr. Erickson contact Lafarge Canada and The Fath Group to see if they would be interested in mining the area under a lease agreement. This should be done fairly soon as both pits operated by both operators appear to be nearly depleted and they may commence with reclamation soon. Getting an operator

in the future may be difficult because the area is relatively small and there are large amounts of overburden to move to access the gravel seam.

4 Closure

This report, entitled *Aggregate Characterization Assessment*, prepared for Wes Erickson.; prepared by Twerdoff & Associates Inc.; June 2014, was produced by the following individual(s):



Dennis Twerdoff, M.Sc., P.Geol., P.Ag.
Senior Project Manager

APEGA Permit to Practice
Permit No. 12187

This report was prepared by Twerdoff & Associates Inc. ("Twerdoff"), for Wes Erickson. The information in it reflects the opinion of Twerdoff in consideration of the information available to it at the time of writing. This report is not to be used by anyone other than Wes Erickson, without the prior written consent of Twerdoff. Whatever uses any third party makes of this report, or any reliance on or decisions made based on it, are the responsibility of those third parties. Twerdoff accepts no liability for damages, if any, suffered by any third party as a result of decisions made or actions based on this report.

5 References

1. Abadata, 2014. Abacus Datagraphics. <http://www.abacusdatagraphics.com/>
2. Shetson, 1990. Quaternary Geology. Central Alberta. Alberta Research Council. http://www.agr.gov.ab.ca/publications/MAP/PDF/MAP_213.PDF
3. <http://groundwater.alberta.ca/WaterWells/d/>

Appendix A

Figures

WES ERICKSON
FIGURE 1: SITE DIAGRAM
 WITHIN SE SEC. 1, TWP. 54, RGE. 2, W5M
 LAC STE. ANNE COUNTY, ALBERTA



Taken From: <http://www.gov.ab.ca/agriculture/landuse/landuse.asp>



LEGEND

FEATURES

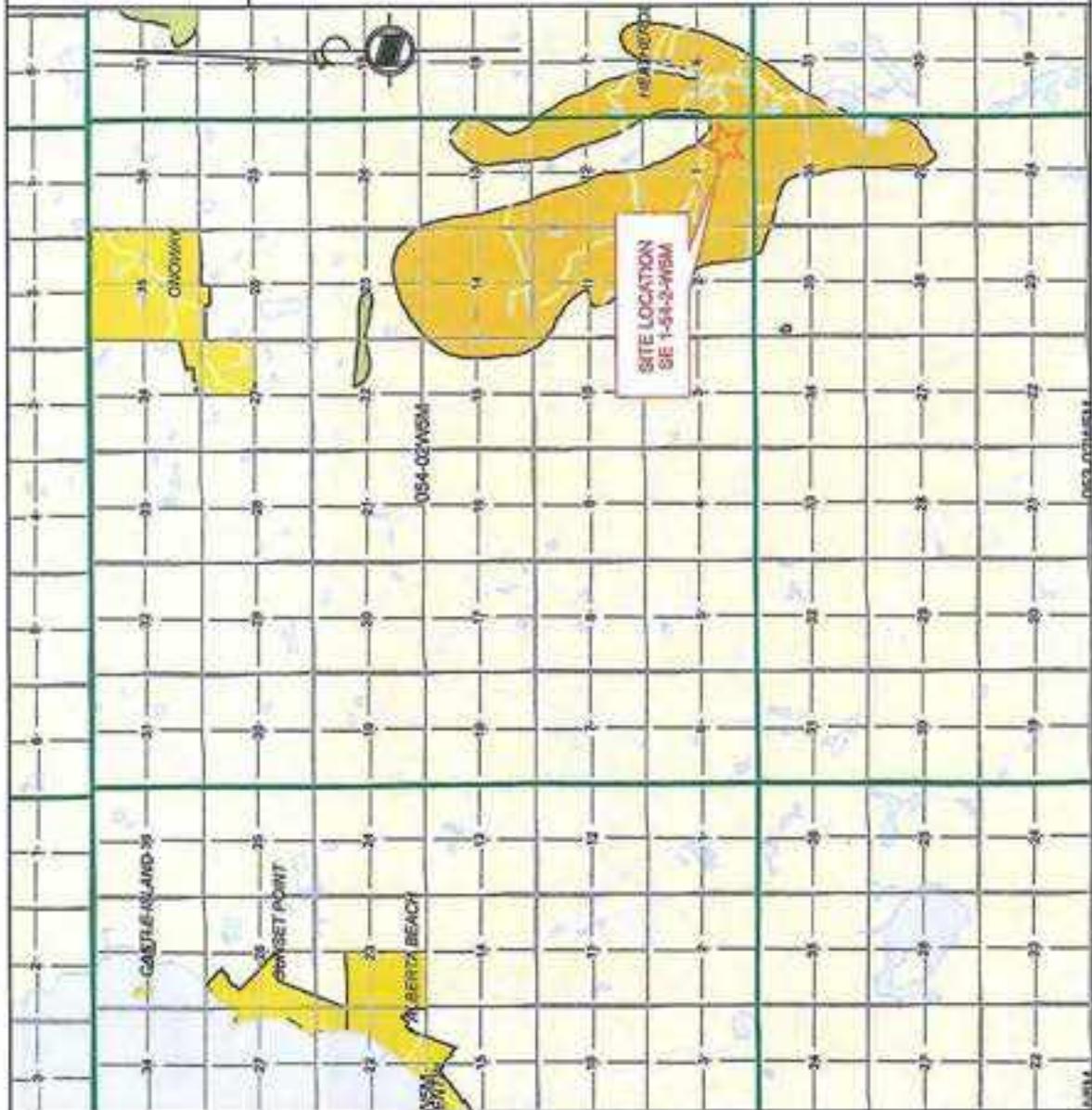
Site Location



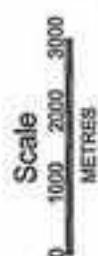
Rev.	Description	Date
0	ORIGINAL ISSUE	Jun 24, 2014
Alpha Date: 2013/07/23 Drawn CCS		Checked DT
TWERDOFF & ASSOCIATES INC.		File No. 14-102

WES ERICKSON

FIGURE 2: AGS MAP
 WITHIN SE SEC. 1, TWP. 54, RGE. 2, W5M
 LAC STE. ANNE COUNTY, ALBERTA



Taken From <http://www.gov.bc.ca/arcgis/arcgis/rest/services/arcgis/rest/info?map>



LEGEND

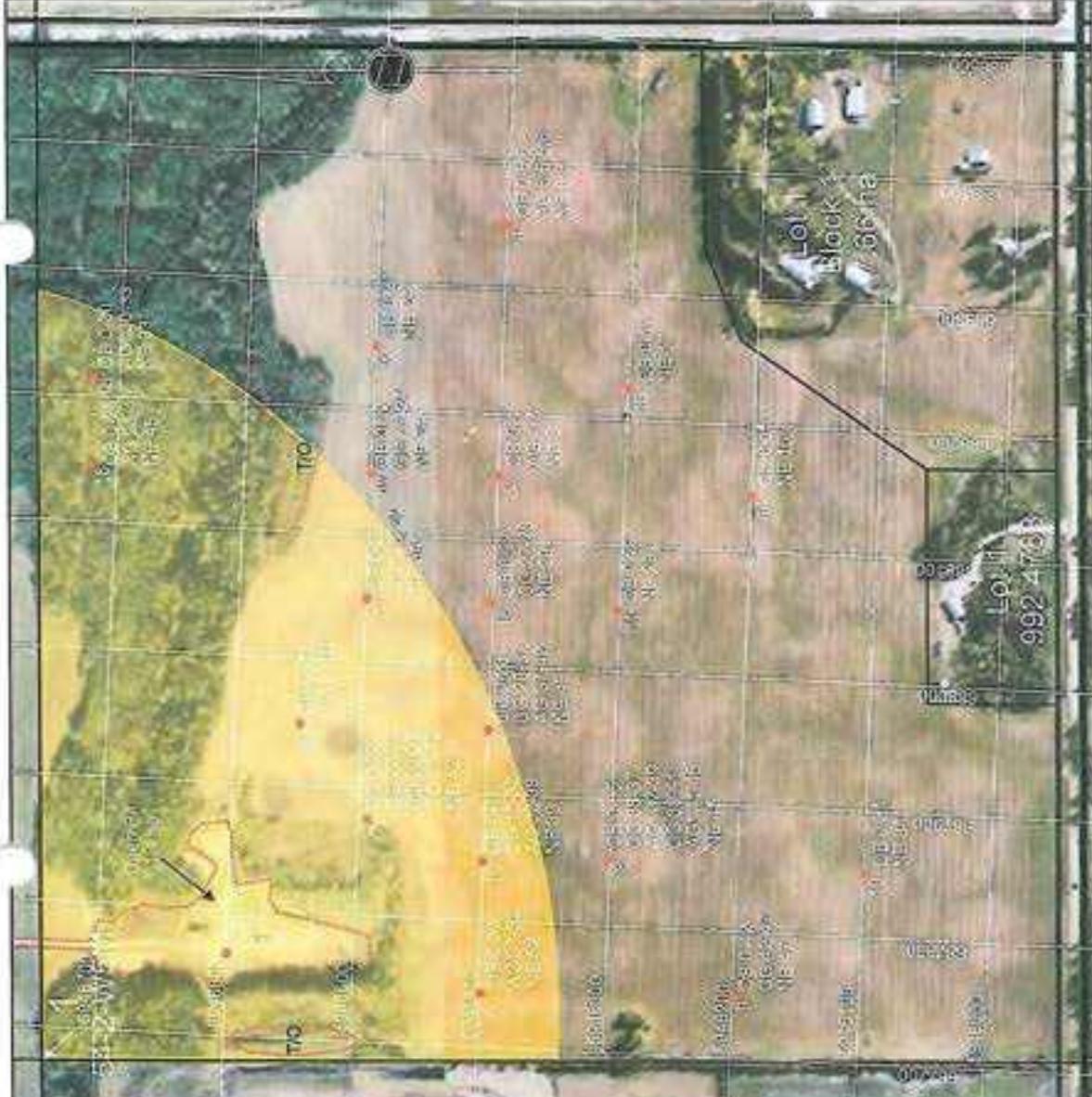
- FEATURES**
- Site Location.....☆
 - Dirty Sandy Gravel.....
 - (Defined by the Alberta Geological Survey)



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WES ERICKSON

FIGURE 3: TEST HOLE DIAGRAM
 WITHIN SE SEC. 1, TWP. 54, RGE. 2, W5M
 LAC STE. ANNE COUNTY, ALBERTA



Taken From: <http://www.gov.bc.ca/land/landuse/ab.ca/keppel/landuse/landuse.html>

Scale = 1:5 000



LEGEND

FEATURES

- Property Boundary.....
- Test Location.....
- Topsoil/Overburden Stockpiles.....
- Potential Mineable Reservoir.....

- Overburden..... OB
 - Clay/Gravel..... CG
 - Good Quality Gravel..... AG*
 - Aggregate..... AG
 - None Economic Material..... NIE
- (measurements in feet)



Rev.	Description	Date
0	ORIGINAL ISSUE	Jan 24, 2014
As photo Date: 2013/9/22 Drawn CCB		Checked DT
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Appendix B

TEST HOLE LOGS



Test Hole #1	Description	UTM Zone 11
Depth (ft)		5946400 N, 687750 E
0-10	Sand and silt	
10-15	Clay w/ small amount of gravel	
15-26	Dark brown clay	
26-37	Clay w/ small amount of gravel	
37-39	Sand w/ minor gravel	
39-42	Sandy gravel	
42-44	Sand	
44-50	Sandy gravel	
50-52	Gravel	
52-57	Clay	
57-68	Clay	
68-74	Hard clay	
74	Bedrock	

Test Hole #2	Description	UTM Zone 11
Depth (ft)		5946400 N, 687950 E
0-5	Sandy clay	
5-6	Gravel lense	
6-12	Clay w/ minor gravel	
12-28	Clay	
28-37	Clay gravel	
37-39	Gravel	
39-42	Sandy gravel	
42-52	Clay	
52-62	Clay/sand	

Test Hole #3	Description	UTM Zone 11
Depth (ft)		5946400 N, 688150 E
0-4	Sand silt	
4-5	Sand/gravel	
5-8	Sand	
8-37	Clay	
37-52	Hard clay/sand	
52	Bedrock	

Test Hole #4	Description	UTM Zone 11
Depth (ft)		5946400 N, 688450 E
0-5.5	Sand silty	
5.5-5.8	Gravel seam	
5.8-37	Clay	
39-47	Hard clay/sand	

Test Hole #5	Description	UTM Zone 11
Depth (ft)		5946500 N, 688050 E
0-3	Sand silty	
3-8	Clay	
8-25	Brown clay/dark brown clay	
25-34	Gravel*	
34-37	Clay	
37-38	Gravel	
38-42	Clay/silt	
42-47	Brown clay/sand	

Test Hole #6	Description	UTM Zone 11
Depth (ft)		5946600 N, 687750 E
0-12	Sandy gravel	
12-17	Sandy clay	
17-27	Clay/silt	

Test Hole #7	Description	UTM Zone 11
Depth (ft)		5946500 N, 687850 E
0-5	Clay till	
5-8	Sand (medium - course grained)	
8-12	Sandy clay	
12-27	Dark grey clay	
27-28	Clay w/ minor gravel	
28-37	Sand w/ minor gravel	
37-42	Clay w/ sand and gravel	
42-47	Hard sandy clay	
47-52	Silty clay	

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 Calgary, AB T2K 6J0

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Test Hole #8	Description	UTM Zone 11
Depth (ft)		5946300 N, 687850 E
0-0.25	Medium grained sand	
2.5-25	Clay	
25-35	Dark grey clay	
35-37	Clay w/ gravel	
37-39	Gravel*	
39-41	Clayish gravel	
31-45	Gravel*	
45-47	Clay w/ sand and gravel	
47-52	Clay w/ minor sand and gravel	
52-57	Light brown hard sandy clay	

Test Hole #9	Description	UTM Zone 11
Depth (ft)		5946200 N, 687750 E
0-3	Sand silty	
2-18	Brown clay	
18-42	Dark grey clay	
42-45	Gravel and sand w/ clay	
45-46	Gravel lense	
46-47	Gravel and sand w/ clay	
47-52	Brownish grey clay	

Test Hole #10	Description	UTM Zone 11
Depth (ft)		5946400 N, 688050 E
0-2	Sand silty	
2-15	Brown clay	
15-34	Dark grey clay	
34-37	Light brown clay sand	

Test Hole #11	Description	UTM Zone 11
Depth (ft)		5946300 N, 688050 E
0-2	Sand silty	
2-18	Brown clay	
18-29	Dark grey clay	
29-47	Light brown dry platy hard sand/clay	

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Test Hole #12	Description	UTM Zone 11
Depth (ft)		5946500 N, 688250 E
0-3	Sand silty	
3-24	Brown clay	
24-37	Dark grey clay	

Test Hole #13	Description	UTM Zone 11
Depth (ft)		5946300 N, 688250 E
0-2	Sand silty	
2-17	Brown clay	
17-42	Dark grey clay	

Test Hole #14	Description	UTM Zone 11
Depth (ft)		5946200 N, 688150 E
0-1.5	Sand silty	
1.5-15	Brown clay	
15-33	Dark grey clay	
33-37	Light brown sand/clay	

Test Hole #15	Description	UTM Zone 11
Depth (ft)		5946750 N, 688250 E
0-6	Silty sand	
6-12	Clay sand	
12-25	Brown clay	
25-30	Dark grey clay	
30-33	Gravel*	
33-40	Brown clay	
40-46	Light brown sand/clay	

Test Hole #16	Description	UTM Zone 11
Depth (ft)		5946750 N, 688150 E
0-2	Silty sand	
2-25	Brown clay	
25-27	Gravel	
27-30	Sand	
30-45	Gravel	
45-52	Brown clay	

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Test Hole #17	Description	UTM Zone 11
Depth (ft)		5946500 N, 688150 E
0-2	Silty sand	
2-17	Brown clay	
17-24	Dark grey clay w/ gravel	
24-34	Brown clay	
34-37	Light brown sand/clay	

Test Hole #18	Description	UTM Zone 11
Depth (ft)		5946400 N, 688050 E
0-4	Silty sand	
4-15	Brown clay	
15-27	Dark grey clay	
27-29	Dark grey clay w/ gravel	
29-34	Dark grey clay	
34-36	Light brown sand/clay	

Test Hole #19	Description	UTM Zone 11
Depth (ft)		5946400 N, 687850 E
0-2	Silty sand	
2-8	Clay w/ sand	
8-15	Brown clay	
15-32	Dark grey clay w/ minor gravel	
32-39	Sandy gravel	
39-42	Light brown sand/clay	

Test Hole #20	Description	UTM Zone 11
Depth (ft)		5946550 N, 687950 E
0-2	Silty sand	
2-16	Brown clay	
16-25	Dark grey clay	
25-36	Sand (coarser w/ depth)	
36	Boulder	

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“TRAFFIC IMPACT ASSESSMENT”

S.E. 1-54-2-5

PREPARED BY

D & A PAULICHUK CONSULTANTS LTD.

APPENDIX ‘C’

DECEMBER 2015

TRAFFIC IMPACT ASSESSMENT

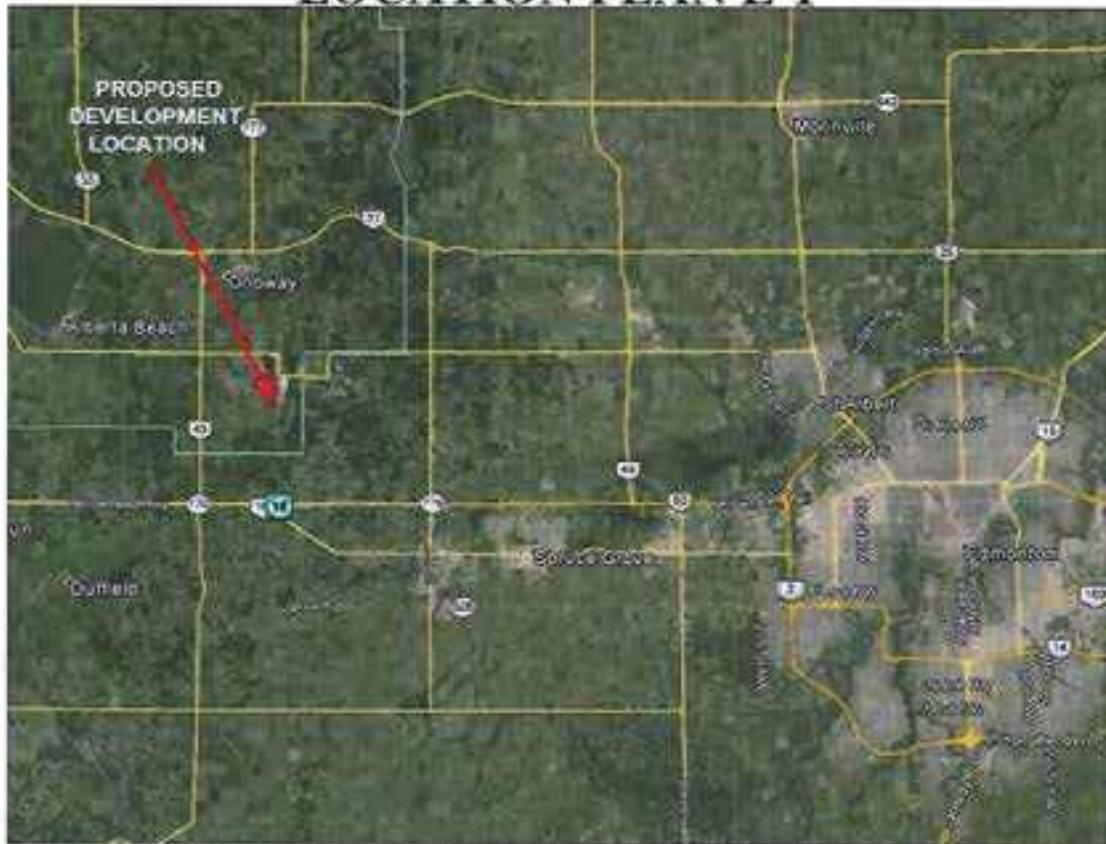
LAUNCH VENTURES RESIDENTIAL DEVELOPMENT
SE 1-54-2-W5M
Lac Ste. Anne County
Twp. Rd. 540, Hwy. 43, Rge. Rd. 20, Hwy. 16:14



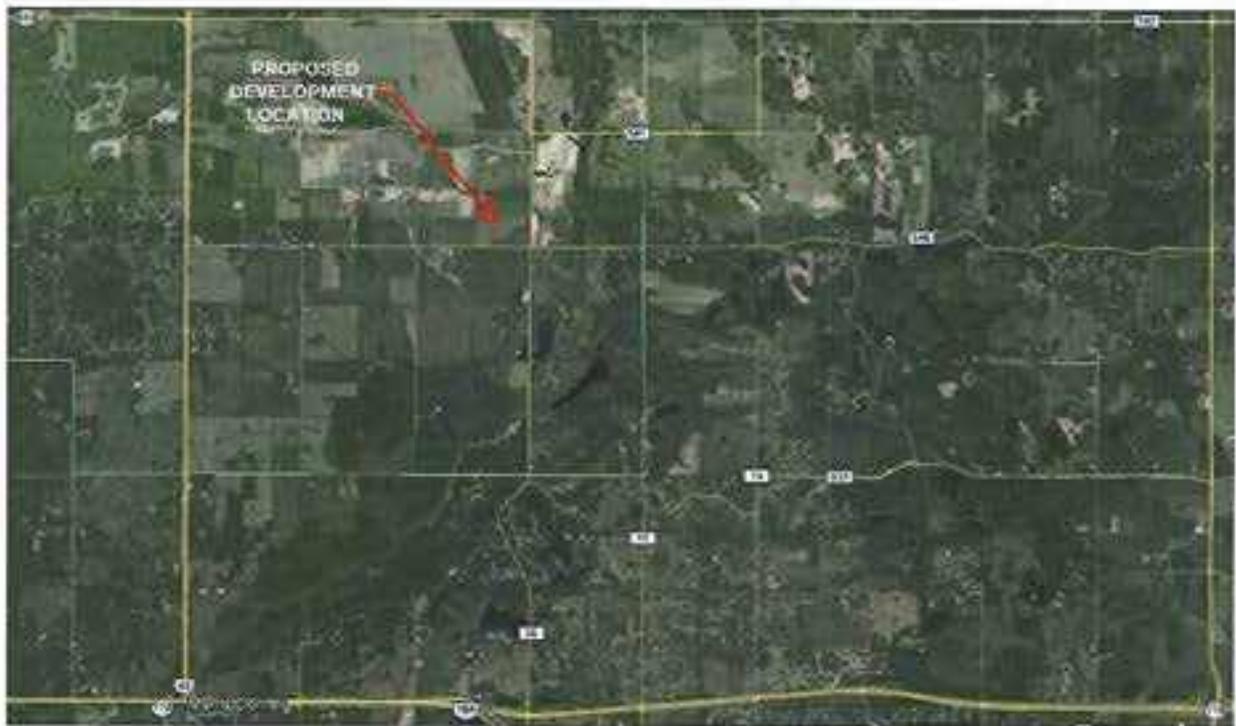
D&A PAULICHUK
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December 7, 2015

LOCATION PLAN L-1



LOCATION PLAN L-2



LOCATION PLAN L-3



LOCATION PLAN L-4



DEVELOPMENT PLAN D-1



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TRAFFIC DATA – TURNING MOVEMENT DIAGRAMS
TRAFFIC COUNT DATA

APPENDIX B - **Development Access & Rge. Rd. 20**
TURNING MOVEMENT DIAGRAMS

APPENDIX C - **Hwy. 16:14 & Rge. Rd. 20**
TURNING MOVEMENT DIAGRAMS
SYNCHRO ANALYSIS

APPENDIX D - **Hwy. 43:22 & Twp. Rd. 540**
TURNING MOVEMENT DIAGRAMS
SYNCHRO ANALYSIS
ILLUMINATION WARRANT



1. INTRODUCTION

This report is a Traffic Impact Assessment (TIA) report for a Residential development located in the SE quarter section of Section 1, Township 54, Range 2, West of 5th Meridian. See Location Plans before this report. The residential subdivision is proposed to contain 13 lots.

This report has been prepared to determine the impact of the proposed development on traffic using Highway 43:22 & Township Road 540 (Heatherdown Road) and Hwy. 16:14 & Range Road 20 which are the anticipated access routes to the proposed development. The scope of work in this case, is to assess the existing roadways and any required intersection treatment as a result of the increased traffic volumes and turning movements attributed to the development. This document will address the following:

- Retrieve any available traffic volume data, including conducting traffic counts.
- Determine present traffic volumes for 2016.
- Determine projected traffic volumes to 2016, 2021, 2026 and 2036.
- Determine traffic volumes generated from the development.
- Determine combined traffic volumes for 2016, 2021, 2026 and 2036.
- Complete intersectional analysis of the roadways if required.
- Identifies geometric deficiencies based on current 3R/4R guidelines.
- Review sight distance requirements.
- Complete illumination and signalization warrant analysis, if required.
- Provides a preliminary engineering cost estimate for recommended improvements.

This report is based on information provided by the developers of the 13 lots in SE 1-54-2-W5M, site observations from Mr. Darcy Paulichuk, P. Eng., traffic volume data from Alberta Transportation, intersectional analysis procedures and standards documented in Alberta Infrastructure and Transportation's "Highway Geometric Design Guide", 1999, and Alberta Infrastructure and Transportation's "Traffic Impact Assessment Guideline", 2005.

This report has been prepared for developers of the 13 lots in SE 1-54-2-W5M for the purposes of gaining approval from municipal and provincial governments for the development of this site.

2. DEVELOPMENT DETAILS

2.1 Development Site

The proposed development is for a 13 lot Residential rural subdivision within SE 1-54-2-W5M. The site will utilize Range Road 20 for access to the east and then south on Rge. Rd. 20 to the south to Hwy 16 or west on Twp. Rd. 540 (Heatherdown Road) to Hwy. 43. See below:

FIGURE 2.1
Proposed Development Site Plan



2.2 Development Trip Generation

The proposed development will comprise of 13 lots.

For the **Residential** development lots, ITE 210 – “Single-Family Detached Housing” Land Use from the Institute of Transportation Engineers (ITE) Trip Generation Manual, 9th Edition, can be used. The following data is available from the manual:

Time Period	Trips per Lot
Daily	9.52
Peak Hour of Adjacent Street Traffic One Hour Between 7 and 9 a.m.	0.75
Peak Hour of Adjacent Street Traffic One Hour Between 4 and 6 p.m.	1.00

Single-Family Detached Housing (210)

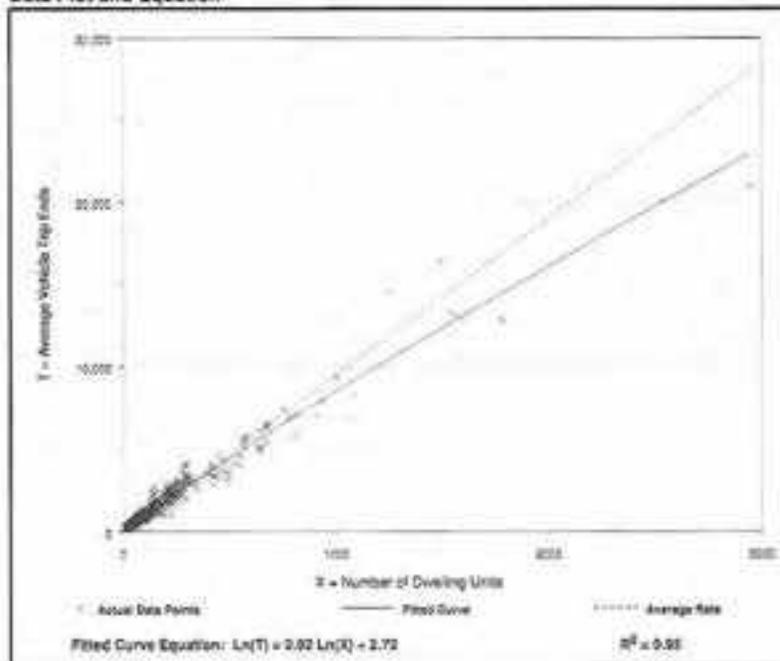
Average Vehicle Trip Ends vs: Dwelling Units
 On a: Weekday

Number of Studies: 355
 Avg. Number of Dwelling Units: 198
 Directional Distribution: 50% entering, 50% exiting

Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
9.52	4.21 - 21.85	3.70

Data Plot and Equation



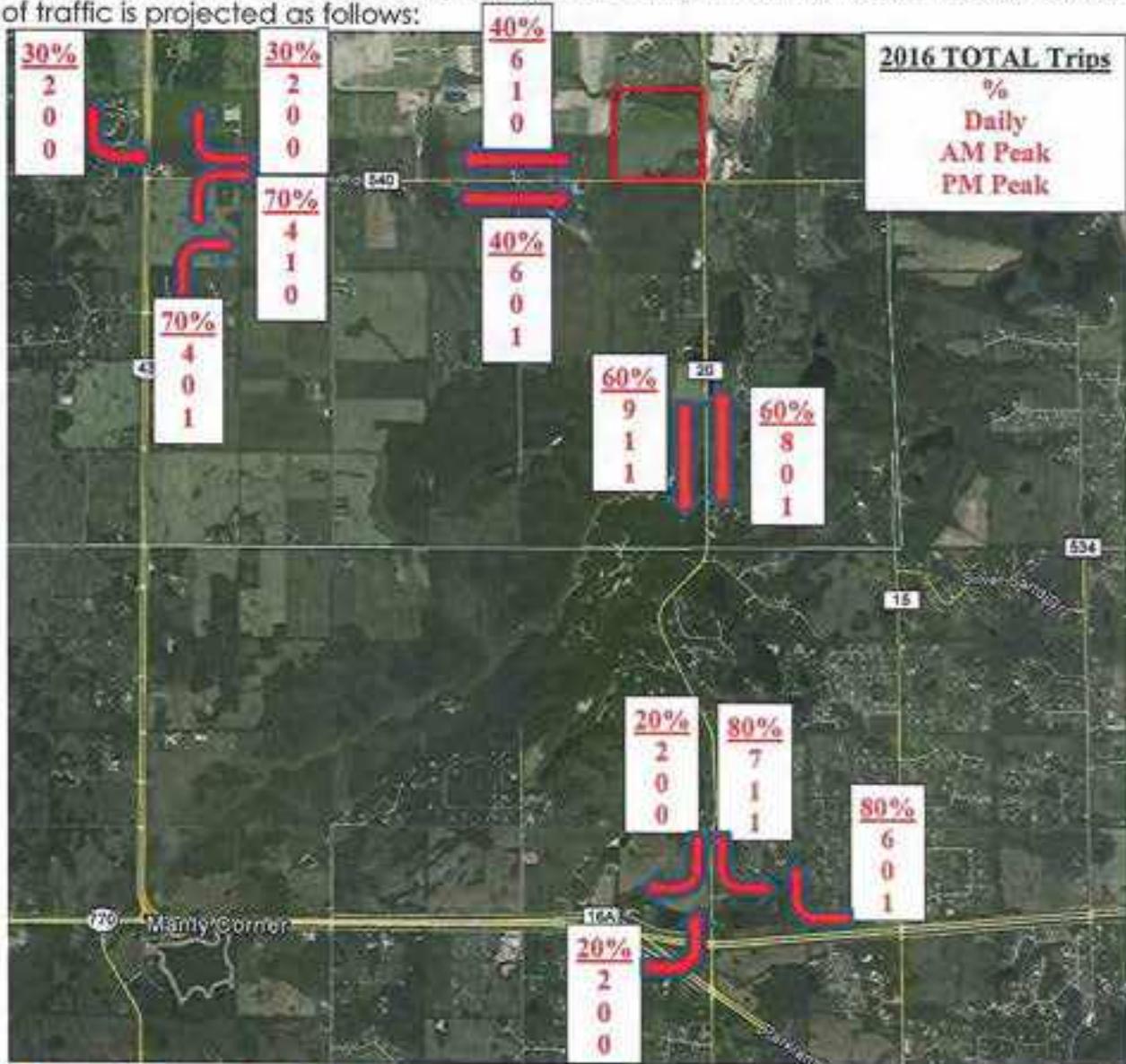
TRAFFIC IMPACT ASSESSMENT

LAUNCH VENTURES RESIDENTIAL
 SE 1-54-2-W5M
 Lac Ste. Anne County
 TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

Therefore, the estimated trip generation of the proposed development with 13 new lots is as follows:

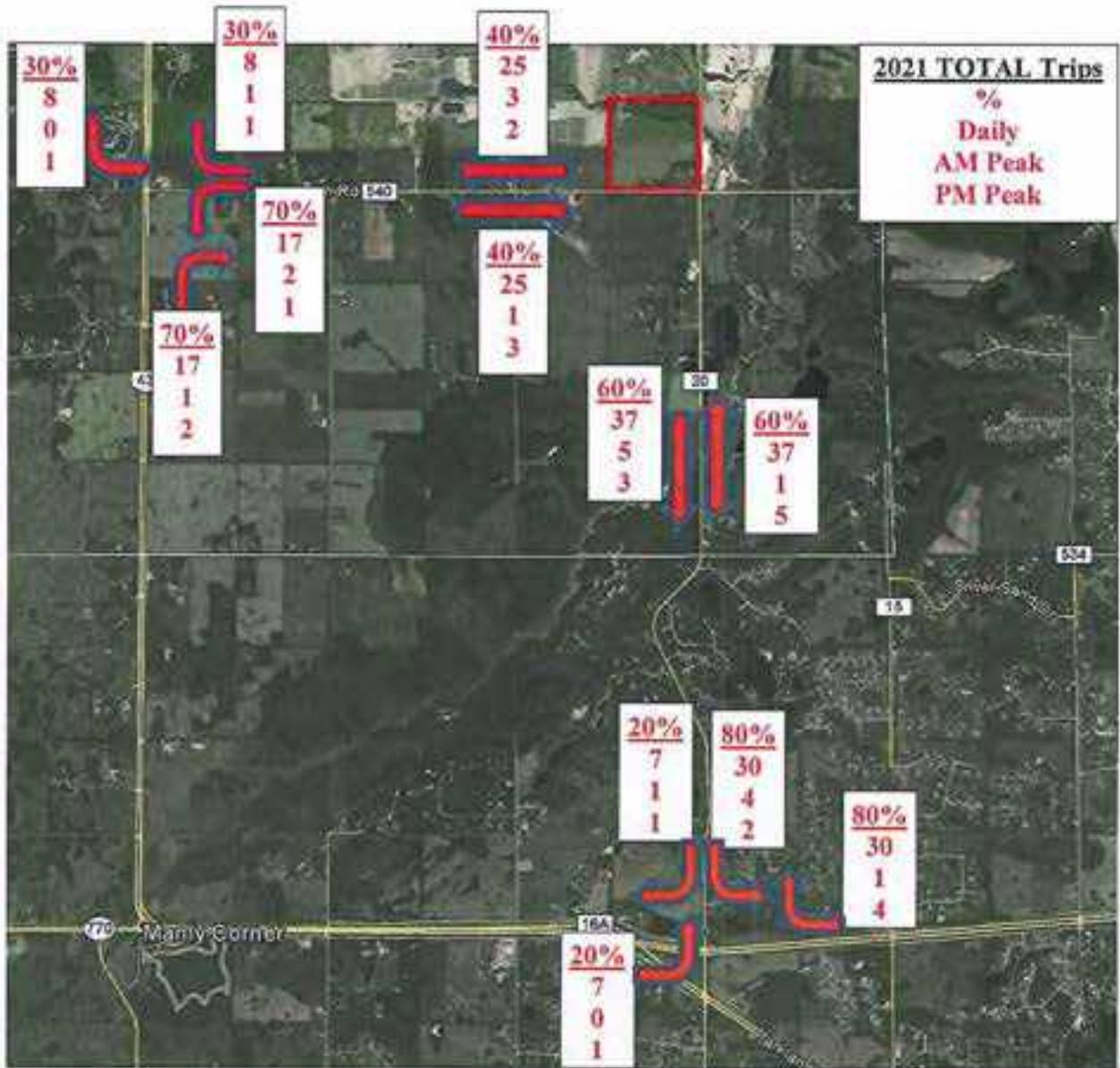
Daily Traffic:	13 lots x 9.52 trips/day	= 124 trips per day
AM Peak Hour:	13 lots x 0.75 trips/hour	= 10 trips per hour
PM Peak Hour:	13 lots x 1.00 trips/hour	= 13 trips per hour

These values above represent the traffic generation once all 13 lots are built, homes are constructed and then occupied. Since this number is fairly low, it is estimated that 3 lots will be occupied in 2016 and the remaining 10 lots will be occupied in 2021 (Year 5). The trip distribution is anticipated to be 40% to the west on Twp. Rd. 540 to Hwy. 43 and 60% to the south on Range Road 20 to Hwy. 16, with 25% entering/75% exiting in the AM Peak and 63% entering/37% exiting in the PM Peak. The distribution of traffic is projected as follows:



TRAFFIC IMPACT ASSESSMENT

LAUNCH VENTURES RESIDENTIAL
 SE 1-54-2-W5M
 Lac Ste. Anne County
 TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

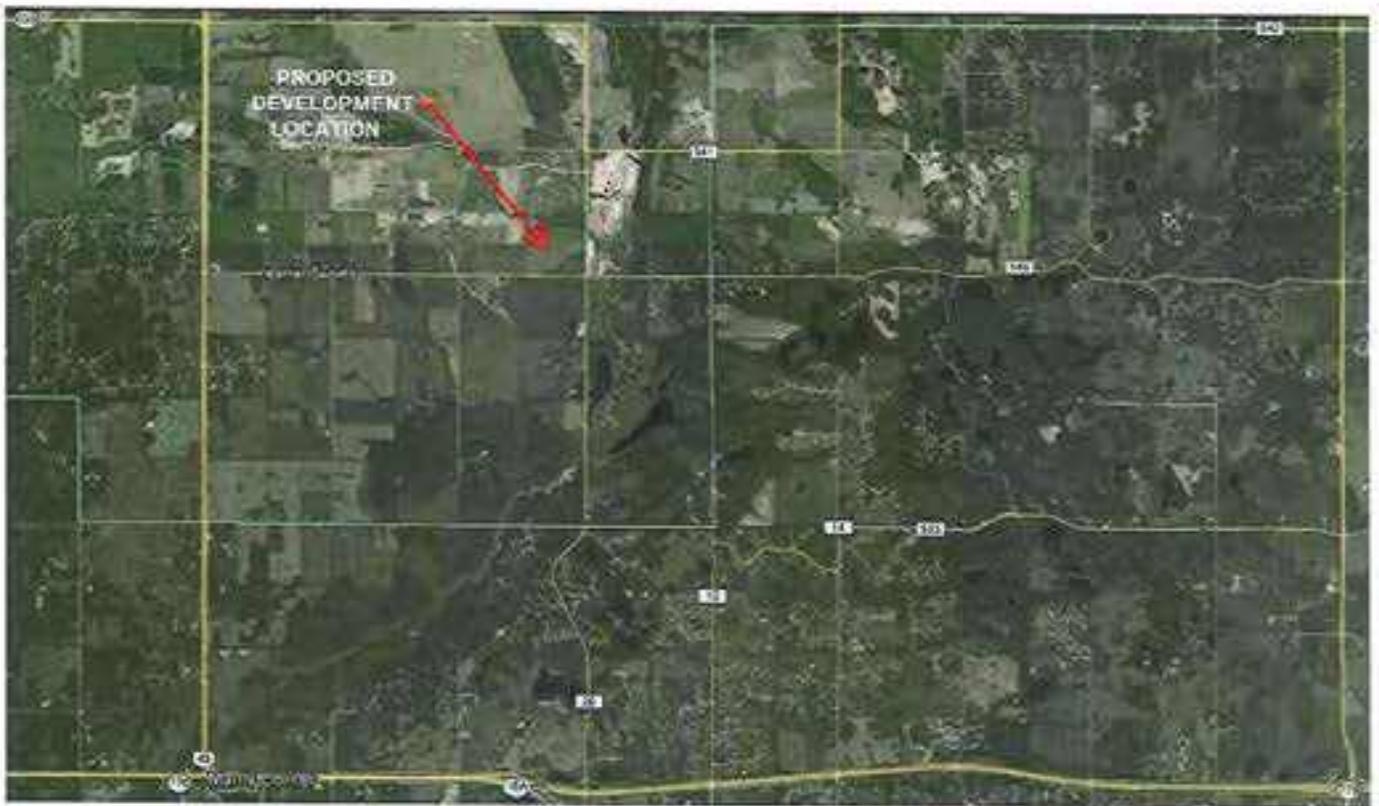


3. EXISTING CONDITIONS – LOCAL ROADS

The focus of this traffic impact assessment is mainly on Twp. Rd. 540 to Hwy. 43 and Rge. Rd. 20 to Hwy. 16 in which it is anticipated that the development will use.

3.1 Development Routes of Travel

Range 20 traverses between Highway 16 to the south and Highway 633 to the north. Twp. Rd. 540 traverses between Highway 43 and Hwy. 779 to the east.



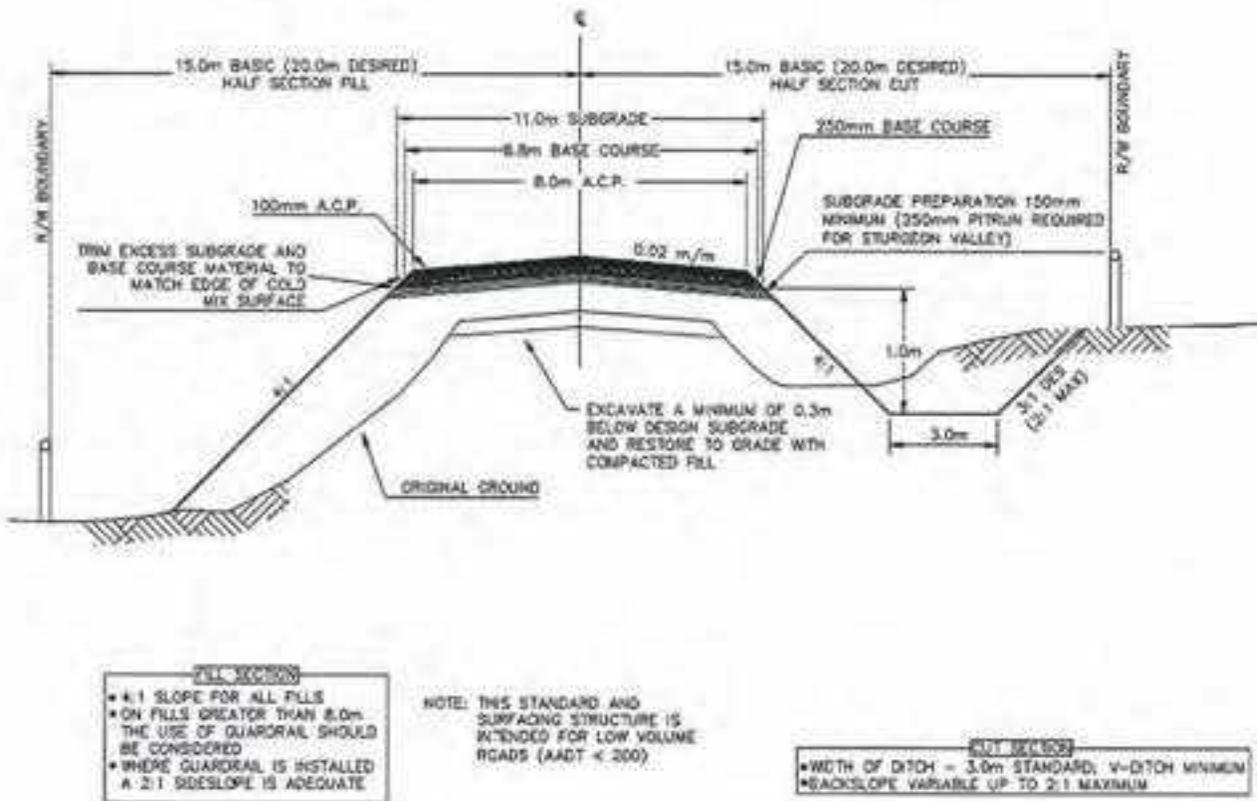
3.2 Range Road 20

Range Road 20 can be classified as a minor to major collector roadway. The existing width is approximately 8.2m from Hwy. 16 to Twp. Rd. 541, and 9.2m wide from Twp. Rd. 541 to Hwy. 633 (west leg)/Twp. Rd. 542. Range Road 20 is surfaced with a full surfacing structure comprised of granular base course and asphalt concrete pavement. The pavement thickness appears to be 100 – 150mm in thickness. The roadway also contains roadway paint lines for the centreline and shoulders. Range Road 20 provides collection of traffic from the local area and provides connections

to the provincial highway system. It also is a route constructed for the haul of natural resources from the area such as aggregate.

The section of Range Road 20 from Hwy. 16 to Hwy. 633 is posted at 80 km/hr. This usually implies a 90 km/hr. design speed.

The section of Range Road 20 from Hwy. 16 to Twp. Rd. 541 was constructed several years ago and likely to a RLU-208 standard (8m paved width standard) as shown below as Lac Ste. Anne County Standard Drawing G-07:



This standard is dated March 2008, however this section of Range Road 20 was constructed several years before this, likely with a lesser standard. This can be recognized with physical aspects such as a 3:1 fill slope in some locations.

This present County standard would be most representative for this section.

Below is an air photo view and ground photo of this section of Range Road 20.



The following table is from Lac Ste. Anne County's General Municipal Servicing Standards and represents the roadway design parameters for the various road standards. The two standards that are being used as comparable standards for Range Road 20 are noted below.

* AADT = Average Annual Daily Traffic
 † ROW = Right-of-Way
 ‡ SSD = Stopping Sight Distance
 Note: The recommended design speeds are for internal roadway systems only.

Designation	Surface	AADT*	Truck Traffic	Min. ROW** (m)	Preferred ROW** (m)	Design Speed (km/h)	Posted Speed (km/h)	SSD [‡] (m)	Crest k (m)	Sag k (m)	Min. Horiz. Radius (m)
RLU-207G	Gravel	< 25	None	20	30	40	30	65	7	11	
RLU-208G(a)	Gravel	< 100	Minimal	20	30	60	50	85	15	20	90
RLU-208G(b)	Gravel	< 200	Minimal	20	30	60	50	85	15	20	90
RLU-209G	Gravel	> 200		20	40	60	50	85	15	20	90
RLU-210G	Gravel	> 200	Significant	20	40	70	60	140	35	30	190
RLU-209(a)	Cold Mix	< 200	Minimal	20	40	60	50	85	15	20	90
RLU-209(b)	ACP	< 200		20	40	60	50	85	15	20	90
RLU-209(a)	Cold Mix	< 500	Minimal	20	40	60	50	85	15	20	90
RLU-209(b)	ACP	< 500		40	40	60	50	85	15	20	90
RLU-210	ACP	< 2000		40	40	70	60	140	35	30	190
RLU-211	ACP	> 2000	Significant	40	40	70	60	140	35	30	190
ULU-209	ACP	< 2000		20	30	60	50	85	15	20	90
ULU-211.5	ACP	> 2000	Significant	30	30	70	60	140	35	30	190

Table 1 – Geometric Design Requirements

For both road standards, the posted and design speeds appear to be lower than the existing posted speed on Range Road 20. The lower posted speed on the standards above mostly are reflected in the vertical and horizontal alignment.

Using the Transportation Association of Canada standards shown below, the following minimum vertical and horizontal parameters can be used for an 80 km/hr. posted speed (design speed = 90 km/hr.):

Minimum Horizontal Radius:	340m
Maximum Gradient:	5 – 7%
Minimum K value for Crest Curves:	53
Minimum K value for Sag Curves:	40

Alignment and Lane Configuration

Table 2.1.2.3 Minimum Radii for Limiting Values of e and f for Rural and High Speed Urban Roadways*

Design Speed (km/h)	Superelevation (e)	Design Value for f	e + f	Minimum Radius (m) (Rounded)	Minimum Radius for Design (m)
40	0.04	0.17	0.21	85	85
50	0.04	0.16	0.20	88	100
60	0.04	0.15	0.19	149	160
70	0.04	0.15	0.19	201	200
80	0.04	0.14	0.18	280	280
90	0.04	0.13	0.17	375	380
100	0.04	0.12	0.16	492	490
40	0.04	0.17	0.21	85	85
50	0.04	0.16	0.20	88	100
60	0.04	0.15	0.19	130	130
70	0.04	0.15	0.21	184	180
80	0.04	0.14	0.20	250	250
90	0.04	0.13	0.19	338	340
100	0.04	0.12	0.18	437	440
110	0.04	0.11	0.16	565	600
120	0.04	0.09	0.15	728	750
130	0.04	0.08	0.14	921	950
40	0.04	0.17	0.21	85	85
50	0.04	0.16	0.20	88	90
60	0.04	0.15	0.19	100	100
70	0.04	0.15	0.20	140	170
80	0.04	0.14	0.20	226	230
90	0.04	0.13	0.21	304	300
100	0.04	0.12	0.20	394	390
110	0.04	0.10	0.18	525	530
120	0.04	0.09	0.17	687	670
130	0.04	0.08	0.16	892	880

curvature or considerations influencing the horizontal alignment and superelevation.

Moreover, if the latest urban conditions, drivers are accustomed to a greater level of discomfort when driving curves, hence, the maximum lateral forces resulting from lower superelevation rates of the supervisor at all are permissible. In these cases, the maximum lateral forces before as defined by Table 2.1.2.2 are used in calculating minimum radii.

To allow for the fact that in low speed urban design radii are used for maximum permissible superelevation may exist, the minimum value is provided for a number of commonly used rates of superelevation.

Table 2.1.2.4 provides tunnel design values for minimum radii for low speed urban design.

Alignment and Lane Configuration

In determining K values for various sight distances, the height of object is 1.08 m, and the height of object is as outlined following and discussed in more detail in Chapter 1.3.

- For stopping sight distance the most common object a vehicle has to stop for is another vehicle ahead on the road, the height of all light is used. The typical minimum is 0.38 m and is applied for design. Other heights of objects can be used if necessary.
- For decision sight distance the more common height of object is 0.15 m, although other heights, such as stop for pavement markings, are not uncommon.
- For passing sight distance the height of object is 1.35 m, which represents the height of the oncoming vehicle.

Clear Vertical Curves: Design Domain

Based on the above most commonly used heights of object, and on sight distances from Tables 1.2.3.1 and 1.2.3.2, the K values for

stopping sight distance are provided in Table 2.1.3.2 and for passing sight distance the K values are provided in Table 2.1.3.3. The decision sight distance K values are not included because the vertical curve depends on the height of object when it is visible (depending on what the driver has to see).

The calculated K values are based on the angle of curve exceeding the sight distance and they can be used without significant error when the length of curve is less than the sight distance. Applicable differences occur only when it is a small angle and an additional curve is involved in obtaining longer vertical curves.

On undivided roads non-stopping sight distance is used to determine when no-passing pavement markings are required. It is desirable to provide passing sight distance whenever possible but non-stopping sight distance is generally adequate for safe passing manoeuvres.

Non-stopping sight distance is less than passing sight distance, so with design speed, Passing manoeuvres can be completed in less than the full passing sight distance because of the timing of oncoming vehicles.

Table 2.1.3.2 K Factors to Provide Stopping Sight Distance on Crest Vertical Curves

Design Speed (km/h)	Assumed Operating Speed (km/h)	Stopping Sight Distance (m)	View of Vertical Curvature (m)	
			Calculated	Required
30	30	29.8	1.6	2
40	40	44.4	3.7	4
50	47.50	57.4-62.8	6.1-7.2	6-7
60	55.00	74.3-84.8	10.0-11.3	10-13
70	63.70	95.1-113.6	16.4-22.3	16-23
80	72.80	112.6-135.4	23.6-36.1	24-36
90	79.90	127.2-158.7	32.5-52.8	33-53
100	85-100	137.8-205.0	43.9-74.0	44-90
110	91-110	145.3-246.4	58.8-110.7	59-110
120	98-120	152.3-285.6	76.4-151.4	76-150
130	105-130	157.3-327.9	96.8-199.8	97-200

Note: The above are minimum values, use higher K factors whenever possible.

Alignment and Lane Configuration

Maximum Grade Design Domain

Although the relationship between design speeds and maximum grade is relatively subjective, reasonable grades for maximum grade have been developed:

The guidelines for maximum grades are given in Table 2.1.3.1.

Maximum Grade: Design Domain

- The range of values shown in Table 2.1.3.1 recognizes that maximum grade selected for design varies with complexity and the general financial capability of the road

adhering to fund the capital work. For level classification urban roads, land use is an additional consideration and land use is incorporated into the guidelines for urban road and urban arterial collectors.

- The values shown may be adjusted to suit local and economic conditions. Maximum grades by classification and land use are often a matter of policy, and as a result, vary from jurisdiction to jurisdiction. However, the road policy is established at a senior engineering and planning level. In any event, if adjusting these figures, designers should ensure that they explicitly consider the impact of such alternative maximum grade values on safety.

Table 2.1.3.1 Maximum Grades**

Design Speed (km/h)	Category	30		40		50		60		70		80		90		100		110		120/130			
		M	S	M	S	M	S	M	S	M	S	M	S	M	S	M	S	M	S	M	S		
30	RLU	7	11	7	11	8	8	8	8	7	5	7	5	7	5	7	5	7	5	7	5	7	
40	RLU	-	-	8	10	8	8	8	8	7	5	7	5	7	5	7	5	7	5	7	5	7	
50	RLU	-	-	-	-	8	8	8	8	7	5	7	5	7	5	7	5	7	5	7	5	7	
60	RLU	-	-	-	-	-	-	8	8	7	5	7	5	7	5	7	5	7	5	7	5	7	
70	RLU	-	-	-	-	-	-	-	8	7	5	7	5	7	5	7	5	7	5	7	5	7	
80	RLU	-	-	-	-	-	-	-	-	7	5	7	5	7	5	7	5	7	5	7	5	7	
90	RLU	-	-	-	-	-	-	-	-	-	7	5	7	5	7	5	7	5	7	5	7	5	
100	RLU	-	-	-	-	-	-	-	-	-	-	7	5	7	5	7	5	7	5	7	5	7	
110	RLU	-	-	-	-	-	-	-	-	-	-	-	7	5	7	5	7	5	7	5	7	5	
120	RLU	-	-	-	-	-	-	-	-	-	-	-	-	7	5	7	5	7	5	7	5	7	
130	RLU	-	-	-	-	-	-	-	-	-	-	-	-	-	7	5	7	5	7	5	7	5	
40	RLU - Residential	8	10	7	11	7	10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
50	RLU - Industrial/Commercial	8	10	7	11	7	10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
60	RLU - Residential	8	10	8	11	8	10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
70	RLU - Industrial/Commercial	8	10	8	11	8	10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
80	RLU	8	10	8	11	8	10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
90	RLU	8	10	8	11	8	10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
100	RLU	-	-	8	10	8	10	8	10	8	10	8	10	8	10	8	10	8	10	8	10	8	10
110	RLU	-	-	-	-	8	10	8	10	8	10	8	10	8	10	8	10	8	10	8	10	8	10
120	RLU	-	-	-	-	-	-	8	10	8	10	8	10	8	10	8	10	8	10	8	10	8	10
130	RLU	-	-	-	-	-	-	-	-	8	10	8	10	8	10	8	10	8	10	8	10	8	10

- Short grades less than 100 m in length, and one-way down grades may be 1% higher on urban roads, and 2% higher on the return road roads.
- A refers to rising topography.
- M refers to mountainous topography.

Geometric Design Guide for Canadian Roads

Table 2.1.3.4 K Factors to Provide Minimum Stopping Sight Distance on Sag Vertical Curves*

Design Speed (km/h)	Assumed Operating Speed (km/h)	Stopping Sight Distance (m)	View of Vertical Curvature (m)	
			Calculated	Required
30	30	29.8	1.6	2
40	40	44.4	3.7	4
50	47.50	57.4-62.8	6.1-7.2	6-7
60	55.00	74.3-84.8	10.0-11.3	10-13
70	63.70	95.1-113.6	16.4-22.3	16-23
80	72.80	112.6-135.4	23.6-36.1	24-36
90	79.90	127.2-158.7	32.5-52.8	33-53
100	85-100	137.8-205.0	43.9-74.0	44-90
110	91-110	145.3-246.4	58.8-110.7	59-110
120	98-120	152.3-285.6	76.4-151.4	76-150
130	105-130	157.3-327.9	96.8-199.8	97-200

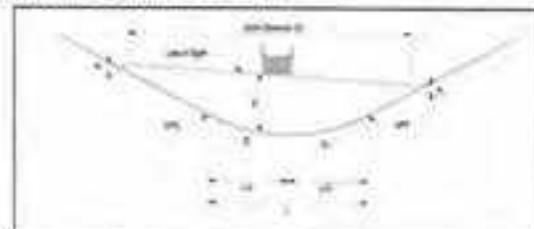
Values for sag curvature based on the vertical curve are shown in Table 2.1.3.4.

These K values for sag curves are useful in urban situations such as underpasses where it is often necessary for property and access reasons to clear from original ground elevations for a certain distance an overpass. Minimum values are normally exceeded where feasible, in consideration of possible power lines and other obstructions in the street lighting systems. Designing sag vertical curves along curved roadways for decision sight distance is normally not feasible due to the clearance for grades and resultant curve design problems.

2.1.3.4 Sight Distance at Underpasses

While not a frequent design problem, the sight distance at underpasses may be restricted due to the overpass structure or signs hanging below the bottom of the overpass structure restricting the line of sight. The sight distance through a grade separation should be equal to or greater than the minimum stopping sight distance. Figure 2.1.3.5 illustrates the sight distance from an eye height of h, is an object height of h, with an underpass clearance of C. Case 1 is for a sight distance greater than the length of the vertical curve (SVC) and Case 2 is for a sight

Figure 2.1.3.5 Sight Distance at Underpass



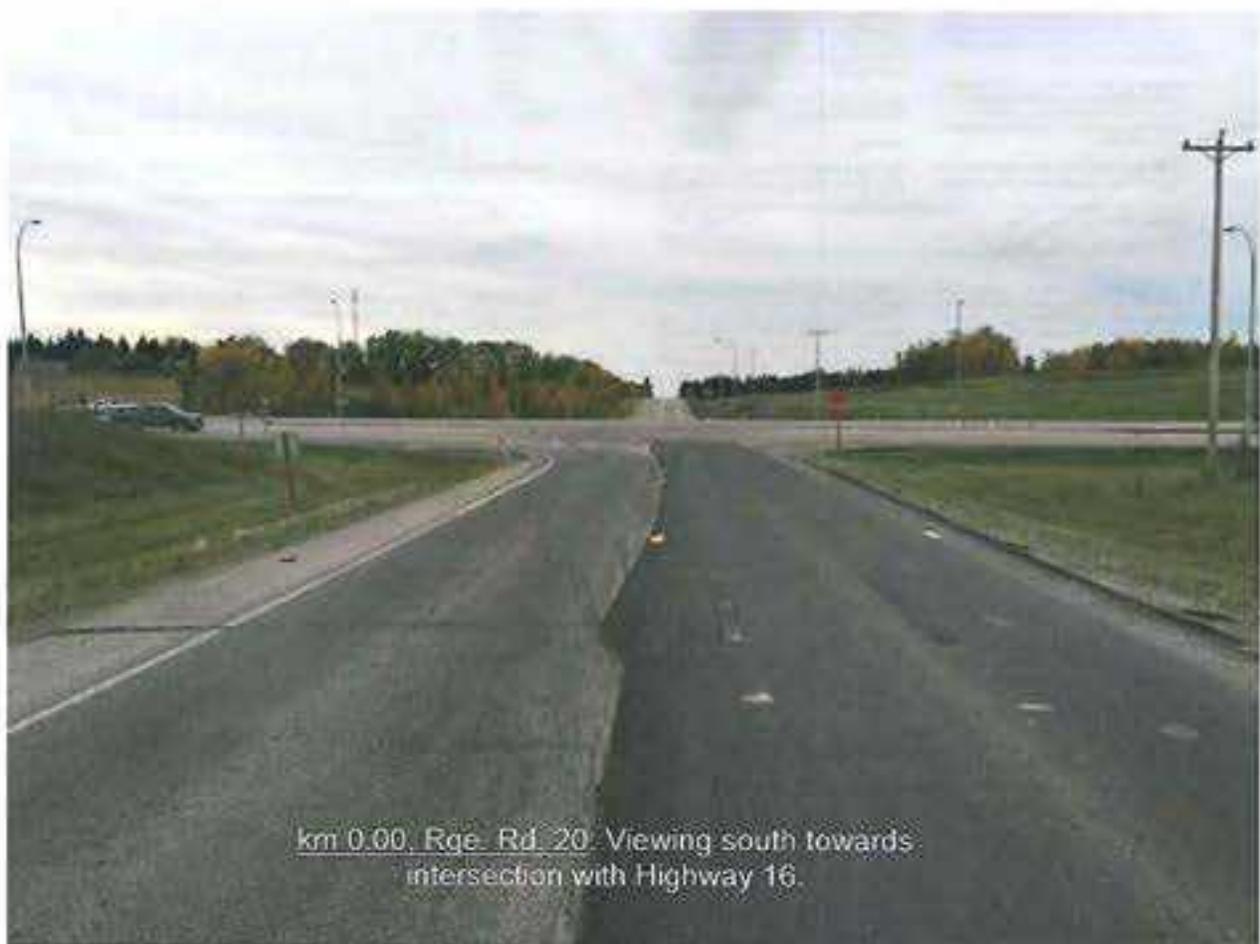
The existing horizontal curvatures on Range Road 20 appear to be between 300 to 350m presently, which is near the standard for an 80 km/hr. posted speed. These curves are within Parkland County. The horizontal alignment of Range Road 20 through Lac Ste. Anne County is straight and on tangent.

The existing vertical profile contains some crest and sag curves that appear to be within the 80km/hr. posted speed standard within Parkland County. Range Road 20 is fairly flat within Lac Ste. Anne County.

All grades appear to be less than 8%.

3.2.1 Site Observations

Site observations of Range Road 20 between September 10 to 14 are shown below:



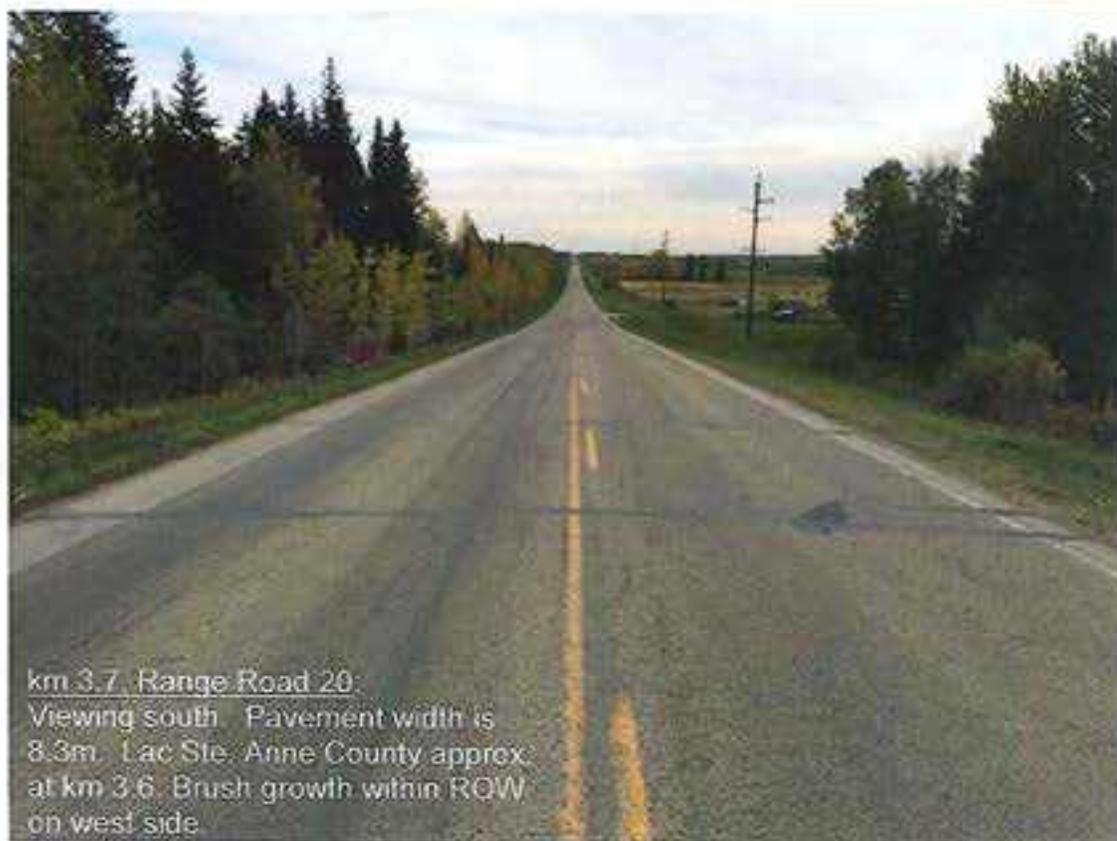


km 0.00, Highway 16:
Viewing northeast towards intersection with Range Road 20. Left Turn out acceleration lane exists in the Eastbound Lanes for SB to EB turns.

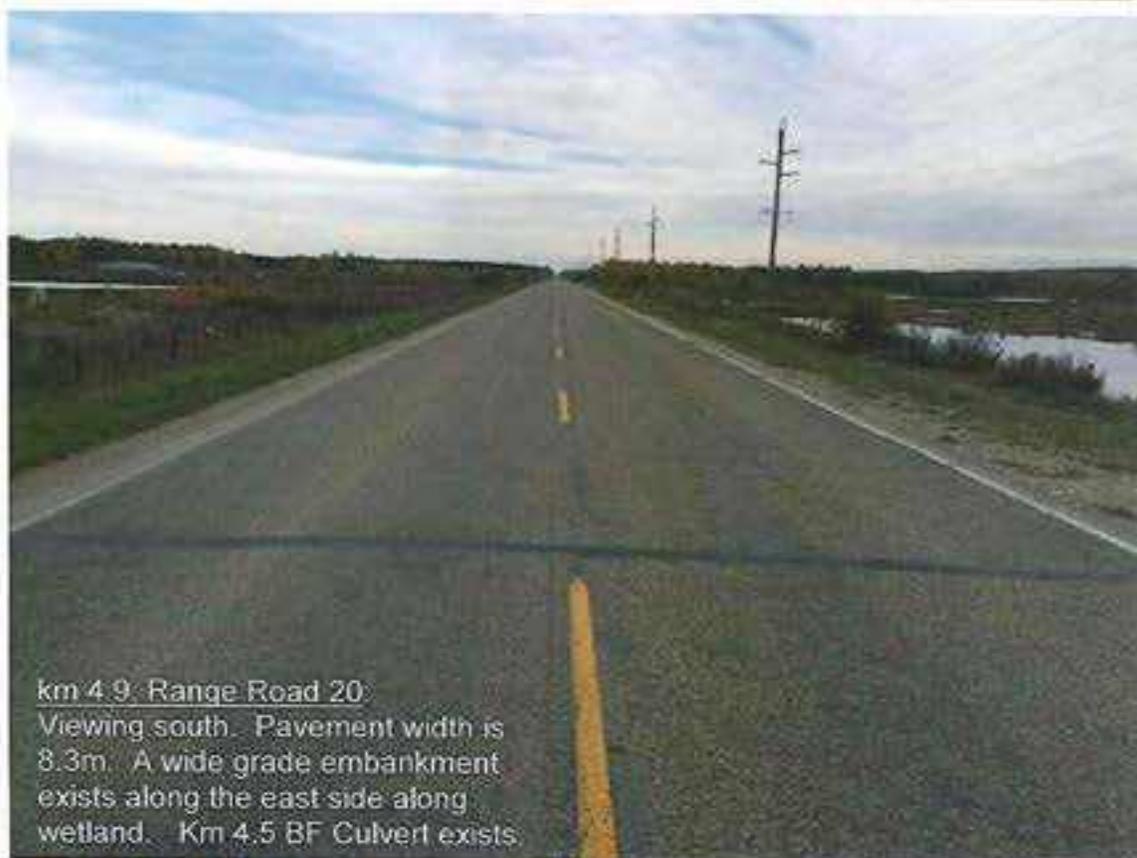


km 0.00, Highway 16:
Viewing west towards the intersection with Range Road 20. Left Turn out acceleration lane exists in the Eastbound Lanes shown on the right, approx. 240m in length (lane + taper).

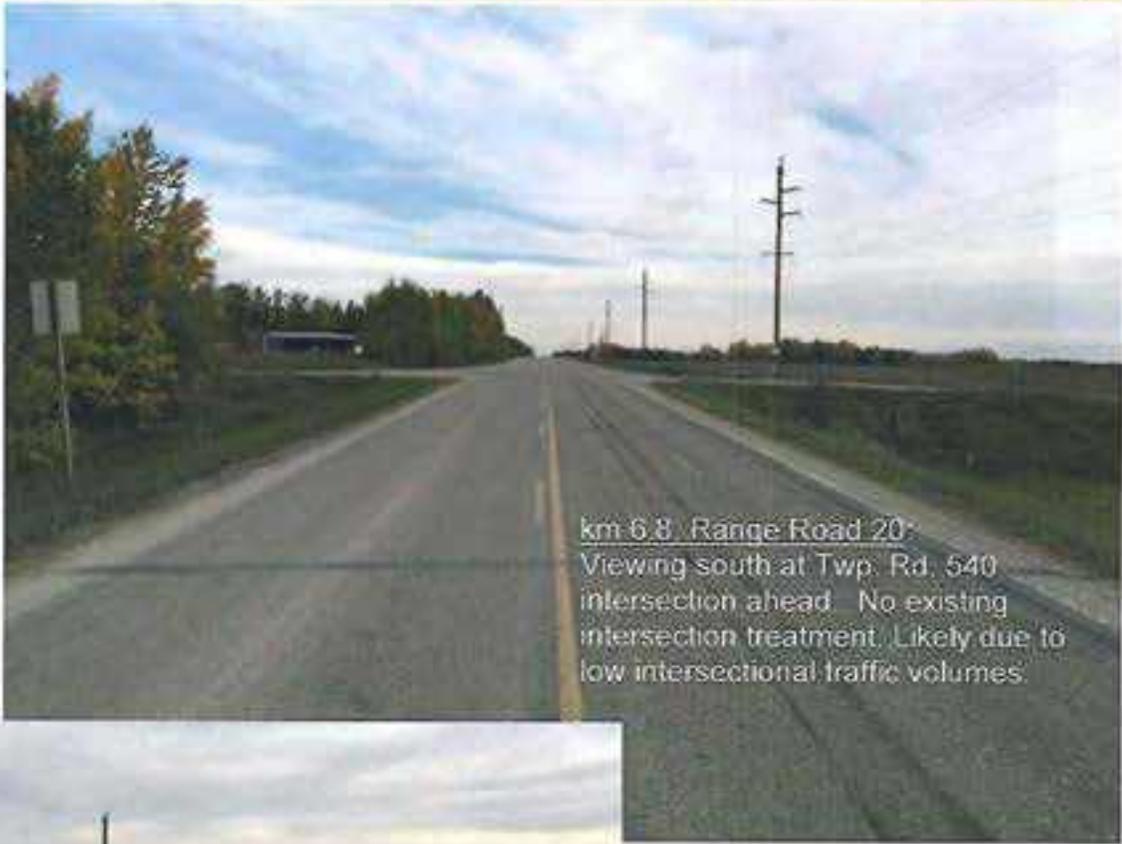




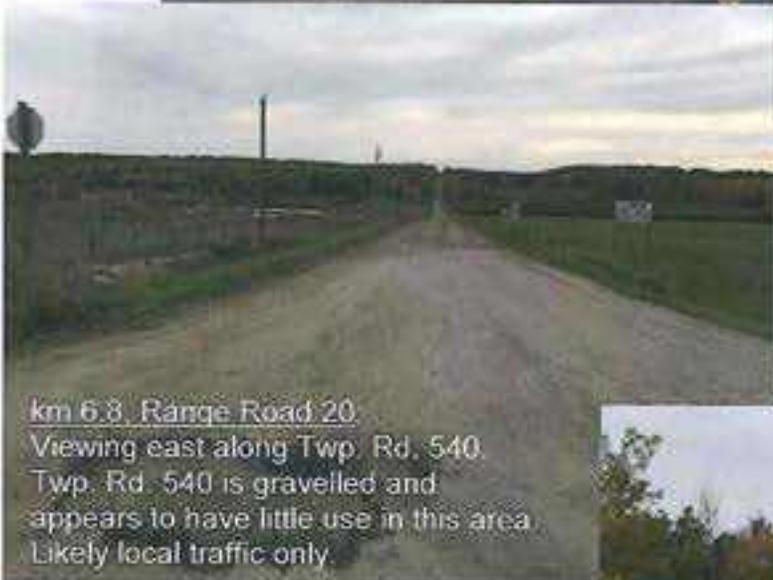
km 3.7 Range Road 20:
Viewing south. Pavement width is
8.3m. Lac Ste. Anne County approx.
at km 3.6. Brush growth within RQW
on west side.



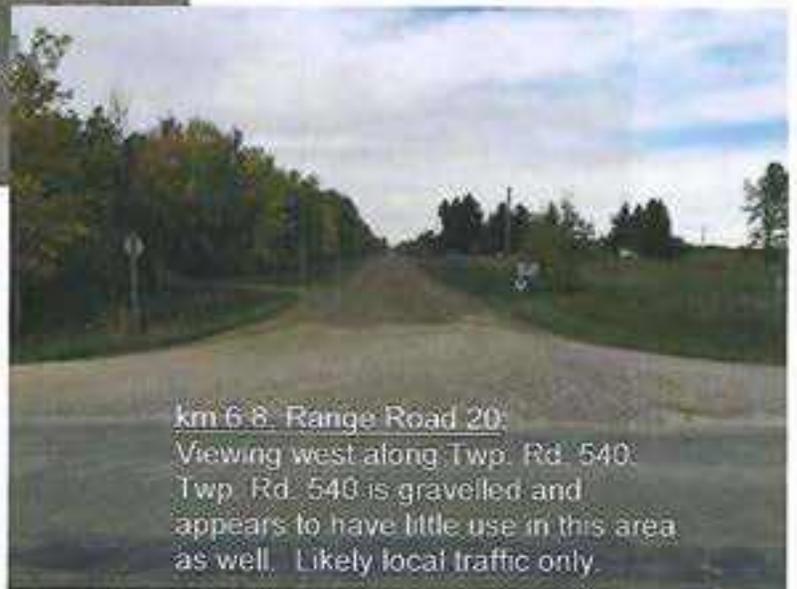
km 4.9 Range Road 20:
Viewing south. Pavement width is
8.3m. A wide grade embankment
exists along the east side along
wetland. Km 4.5 BF Culvert exists.



km 6.8, Range Road 20:
Viewing south at Twp. Rd. 540
intersection ahead. No existing
intersection treatment. Likely due to
low intersectional traffic volumes.



km 6.8, Range Road 20:
Viewing east along Twp. Rd. 540.
Twp. Rd. 540 is gravelled and
appears to have little use in this area.
Likely local traffic only.



km 6.8, Range Road 20:
Viewing west along Twp. Rd. 540.
Twp. Rd. 540 is gravelled and
appears to have little use in this area
as well. Likely local traffic only.

SUMMARY

Upon inspection, Range Road 20 appears to be consistent with a Rural Local Road County standard with widths from 8 to 9m. The existing roadway exhibits cross sectional attributes near or within the standard that includes sideslopes, ditch widths and backslopes. The horizontal and vertical alignments also appear satisfactory and consistent with standard.

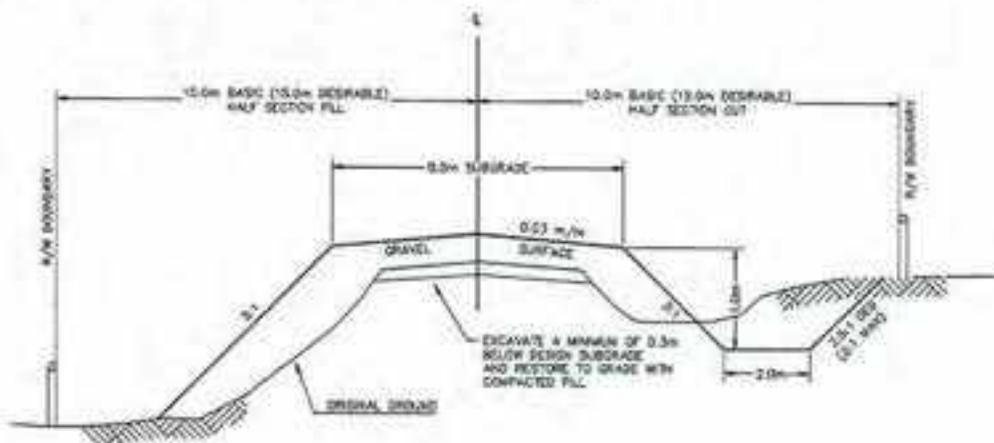
There are no concerns with the present geometric, sight distances and condition of Range Road 20.

3.3 Township Road 540 (Heatherdown Road)

Township Road 540 can be classified as a minor to major collector roadway. It presently collects traffic from the area and distributes it to other collectors or arterial roads such as highways. The existing width is approximately 7 - 9m from Hwy. 43 to Hwy. 779. Range Road 20 is gravel surfaced between Hwy. 43 to RR 20. Township Road 540 will be an important internal local road route once the at-grade intersections close onto Hwy. 43 and Hwy. 16.

The section of Township Road 540 from Hwy. 43 to RR 20 is posted at 80 km/hr. This usually implies a 90 km/hr. design speed.

The section of Range Road 20 from Hwy. 16 to Twp. Rd. 541 was constructed several years ago and likely to a RLU-208a standard (8m gravelled width standard) as shown below as Lac Ste. Anne County Standard Drawing G-02:



DESIGN NOTES:

- 3:1 SLOPE FOR ALL FILLS
- ON FILLS GREATER THAN 8.0m THE USE OF GUARDRAIL SHOULD BE CONSIDERED
- WHERE GUARDRAIL IS INSTALLED A 3:1 SLOPE IS ADEQUATE

NOTE: THIS STANDARD IS INTENDED FOR LOW VOLUME LOCAL ROADS WHICH WILL BE GRAVEL SURFACED ONLY. TYPICALLY THIS 8.0m ROADWAY IS FOR ROADS WITH AN AADTS < 100.

NOTES:

- WIDTH OF DITCH = 2.0m STANDARD; V-DITCH MINIMUM BACKSLOPE VARIABLE UP TO 3:1 MAXIMUM

This standard is dated March 2008, however this section of Township Road 20 was constructed several years before this, likely with a lesser standard. This can be recognized with physical aspects such as a 3:1 fill slope in some locations. This present County standard would be most representative for this section. Below is an air photo view and ground photo of this section of Range Road 20.



The following table is from Lac Ste. Anne County's General Municipal Servicing Standards and represents the roadway design parameters for the various road standards. The two standards that are being used as comparable standards for Township Road 540 are noted below.

* AADT - Average Annual Daily Traffic
 ** ROW - Right-of-Way
 *** SSD - Stopping Sight Distance
 Note: The recommended design speeds are for arterial roadway systems only.

Designation	Surface	AADT*	Truck Traffic	Min. ROW** (m)	Preferred ROW** (m)	Design Speed (km/h)	Posted Speed (km/h)	SSD* (m)	Crest k (m)	Sag k (m)	Min. Horiz. Radius (m)
RLU-207G	Gravel	< 25	None	20	30	40	30	65	7	11	
RLU-208G(a)	Gravel	< 100	Minimal	20	30	60	50	85	15	20	90
RLU-208G(b)	Gravel	< 200	Minimal	20	30	60	50	85	15	20	90
RLU-209G	Gravel	> 200		20	40	60	50	85	15	20	90
RLU-210G	Gravel	> 200	Significant	20	40	70	60	140	35	30	190
RLU-208(a)	Cold Mix	< 200	Minimal	20	40	60	60	85	15	20	90
RLU-208(b)	ACP	< 200		20	40	60	60	85	15	20	90
RLU-209(a)	Cold Mix	< 500	Minimal	20	40	60	60	85	15	20	90
RLU-209(b)	ACP	< 500		40	40	60	60	85	15	20	90
RLU-210	ACP	< 2000		40	40	70	60	140	35	30	190
RLU-211	ACP	> 2000	Significant	40	40	70	60	140	35	30	190
ULU-209	ACP	< 2000		20	30	60	60	85	15	20	90
ULU-211.5	ACP	> 2000	Significant	30	30	70	60	140	35	30	190

Table 1 - Geometric Design Requirements

For both road standards, the posted and design speeds appear to be lower than the existing posted speed on Range Road 20. The lower posted speed on the standards above mostly are reflected in the vertical and horizontal alignment.

Using the Transportation Association of Canada standards shown below, the following minimum vertical and horizontal parameters can be used for an 80 km/hr. posted speed (design speed = 90 km/hr.):

Minimum Horizontal Radius:	340m
Maximum Gradient:	5 - 7%
Minimum K value for Crest Curves:	53
Minimum K value for Sag Curves:	40

The horizontal alignment of Township Road 20 is straight and on tangent. The existing vertical profile contains some crest and sag curves that appear to be within the 80km/hr. posted speed standard and is fairly flat. All grades appear to be less than 8%.

3.3.1 Site Observations

Site observations of Township Road 540 between September 10 to 14 are shown below:



Twp. Rd. 540. Viewing west from Range Road 20

SUMMARY

Upon inspection, Township Road 540 appears to be consistent with a Rural Local Road County standard with widths from 8 to 9m. The existing roadway exhibits cross sectional attributes near or within the standard that includes sideslopes, ditch widths and backslopes. The horizontal and vertical alignments also appear satisfactory and consistent with standard.

There are no concerns with the present geometric, sight distances and condition of Township Road 540.

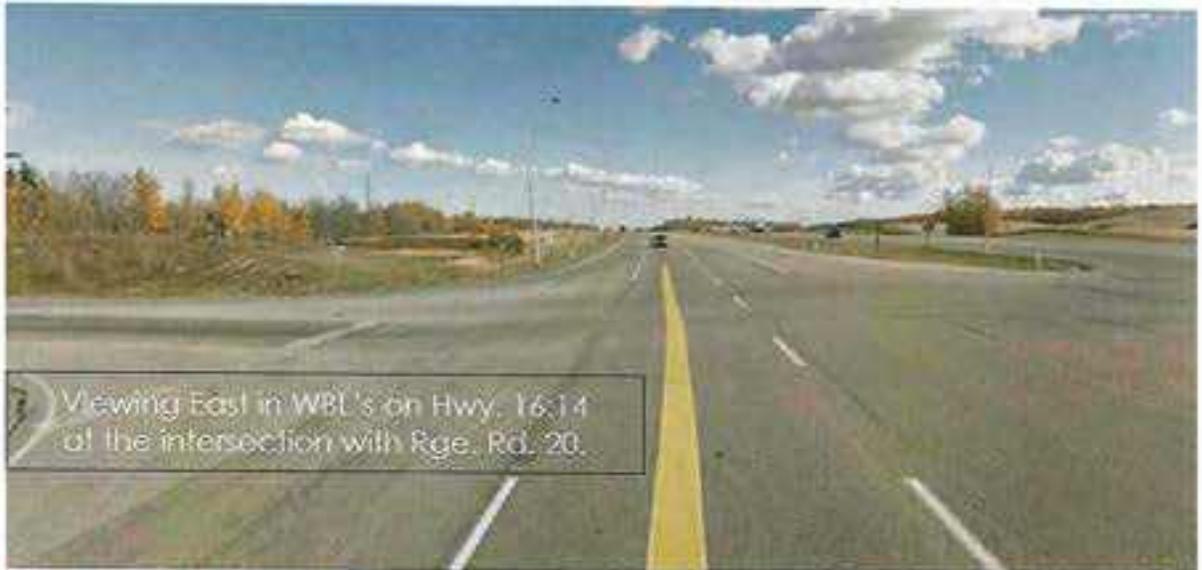
4. EXISTING CONDITIONS - HIGHWAYS

4.1 Existing Intersection Treatment – Hwy. 16:14 & RR 20

The details of the Hwy. 16:14 & RR 20 intersection site are as follows:

- Highway 16:14 is a four-laned roadway and is paved.
- The intersection has an existing treatment as follows:
 - EBL's: Right Turn
 - Deceleration: 80m 25:1 Taper with 100m parallel 3.5m right turn lane;
 - Acceleration: 90m parallel 3.5m right turn lane with a 110m 30:1 Taper;
 - EBL's Left Turn
 - Deceleration: 70m 20:1 Taper with 100m parallel 3.5m left turn lane;
 - **Acceleration: 120m 25:1 Taper with 120m parallel 3.5m left turn lane;**
 - WBL's Right Turn
 - Deceleration: 140m 40:1 Taper with 100m parallel 3.5m right turn lane;
 - Acceleration: 75m Taper
 - WBL's Left Turn
 - Deceleration: 120m 35:1 Taper with 90m parallel left turn lane.
 - Acceleration: None
- There is existing full intersection illumination.
- The side slopes seem to be 5:1 or better throughout.





4.2 Existing Intersection Treatment – Hwy. 43:22 & Twp. Rd. 540

The details of the Hwy. 43:22 & Twp. Rd. 540 intersection site are as follows:

- Highway 43:22 is a four-laned roadway and is paved.
- The intersection has an existing treatment as follows:
 - NBL's Right Turn
 - Deceleration: None;
 - Acceleration: None;
 - NBL's Left Turn
 - Deceleration: 90m 30:1 Taper with 120m parallel 3.0m left turn lane;
 - Acceleration: None
 - SBL's Right Turn
 - Deceleration: 140m 40:1 Taper with 50m parallel 3.0m right turn lane;
 - Acceleration: None
 - SBL's Left Turn
 - Deceleration: 80m 25:1 Taper with 110m parallel 3.0- 3.5m left turn lane.
 - Acceleration: None
- There is no existing illumination.
- The side slopes seem to be 5:1 or better throughout.



TRAFFIC IMPACT ASSESSMENT

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Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14



Viewing South in NBL's on Hwy. 43:22, at the intersection with Twp. Rd. 540.



Viewing North in NBL's on Hwy. 43:22, at the intersection with Twp. Rd. 540.



Viewing North in SBL's on Hwy. 43:22, at the intersection with Twp. Rd. 540.

4.3 Available Traffic Data

Existing Alberta Transportation intersectional traffic count locations are shown in the map below:



AT website <http://www.transportation.alberta.ca/3460.htm> has traffic counts available that are relevant for comparison purposes for this assessment.

There are two specific traffic counts available of interest for this assessment as shown below:

Table-4.1a: 2014 AADT and ASDT from Alberta Highways Traffic Volume History

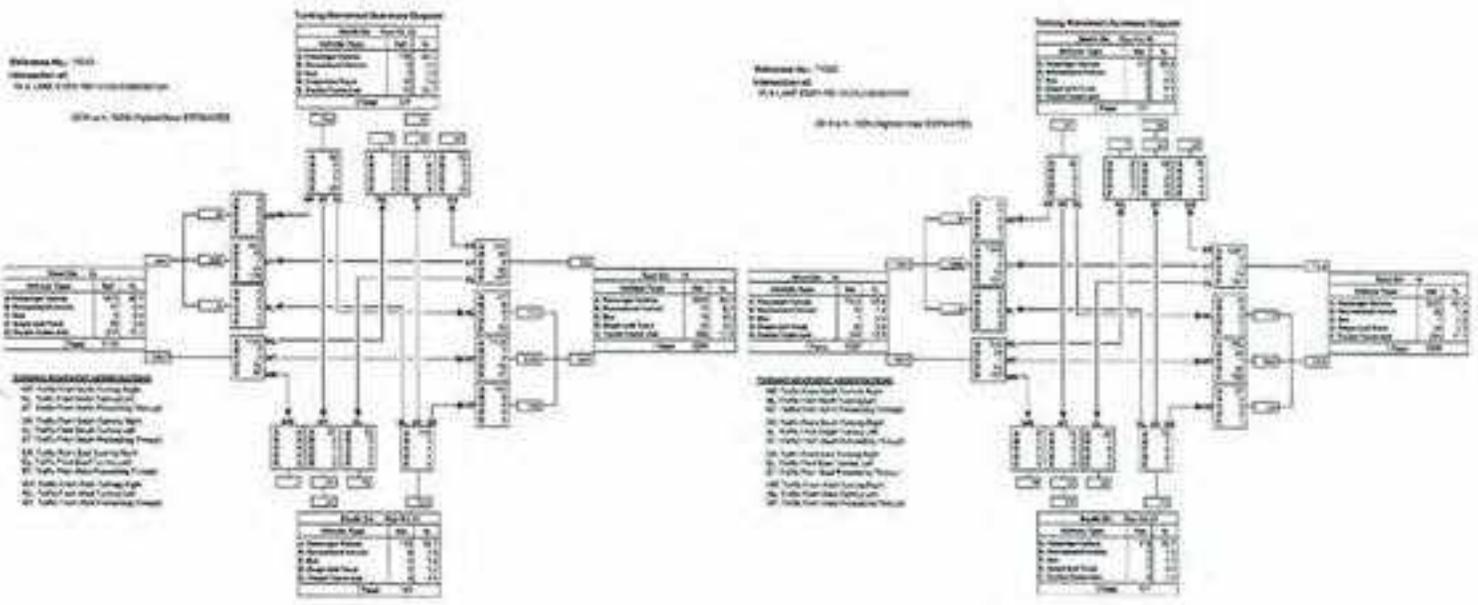
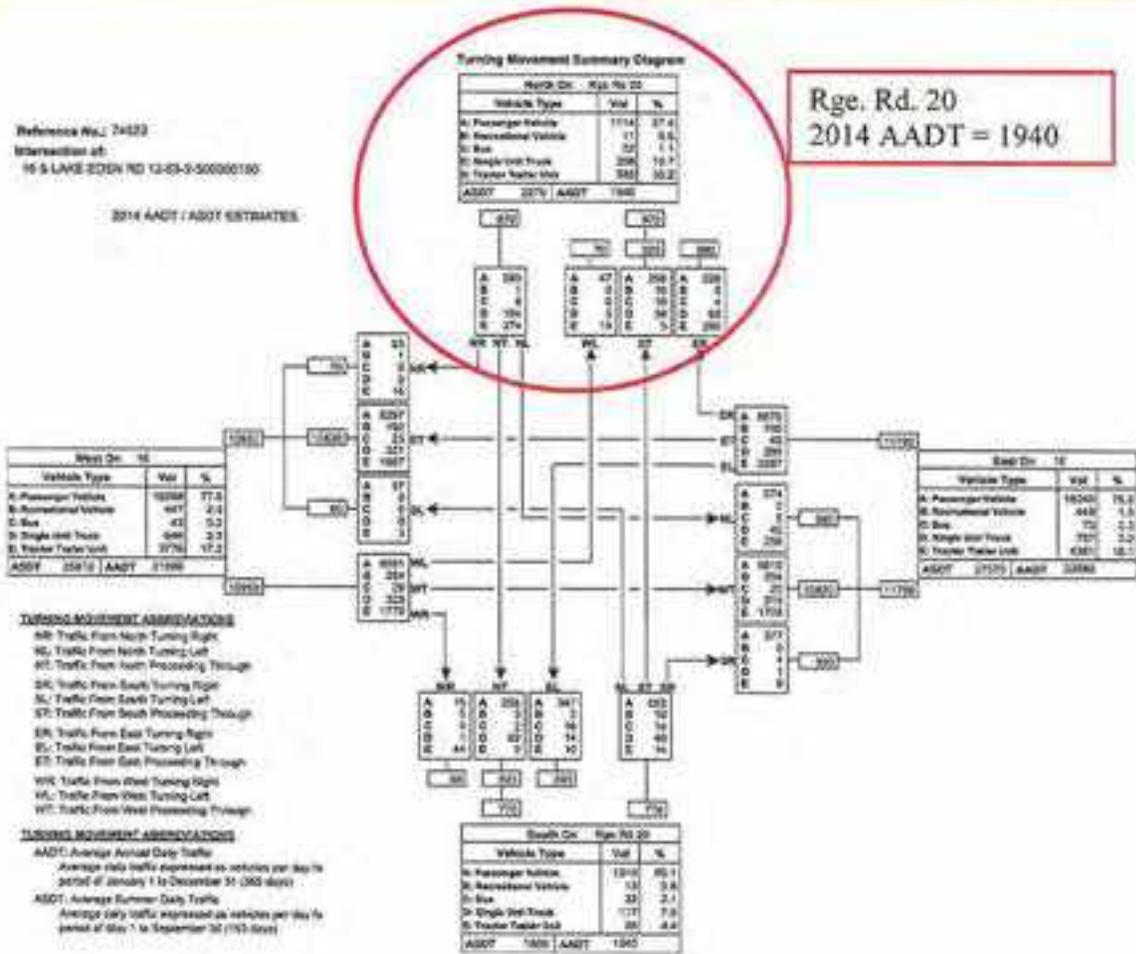
Intersection Leg	2014 AADT	
	16 & Lake Eden Rd (74523)	23580 - Hwy. 16
43 & 633 E of Alberta Beach (71530)	12960 - Hwy. 43	
16 & 43 at Manly Corner (72510)	13360 - Hwy. 43	

TRAFFIC IMPACT ASSESSMENT

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Reference No.: 2422
Intersection at:
16 & LAKE EDDEN RD 12-03-3-50000180

2014 AADT / AADT ESTIMATES



TRAFFIC IMPACT ASSESSMENT

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Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

Reference No.: 7100
Intersection of:
43 & 622 E OF ALBERTA BEACH

2014 AADT / AADT ESTIMATES

Turning Movement Summary Diagram

North On - 43			
Vehicle Type	Vol	%	
A. Passenger Vehicle	875	75.1	
B. Recreational Vehicle	151	4.8	
C. Bus	6	1.1	
D. Single Unit Truck	381	3.3	
E. Tractor Trailer Unit	145	12.7	
AADT	1460	AADT	1460

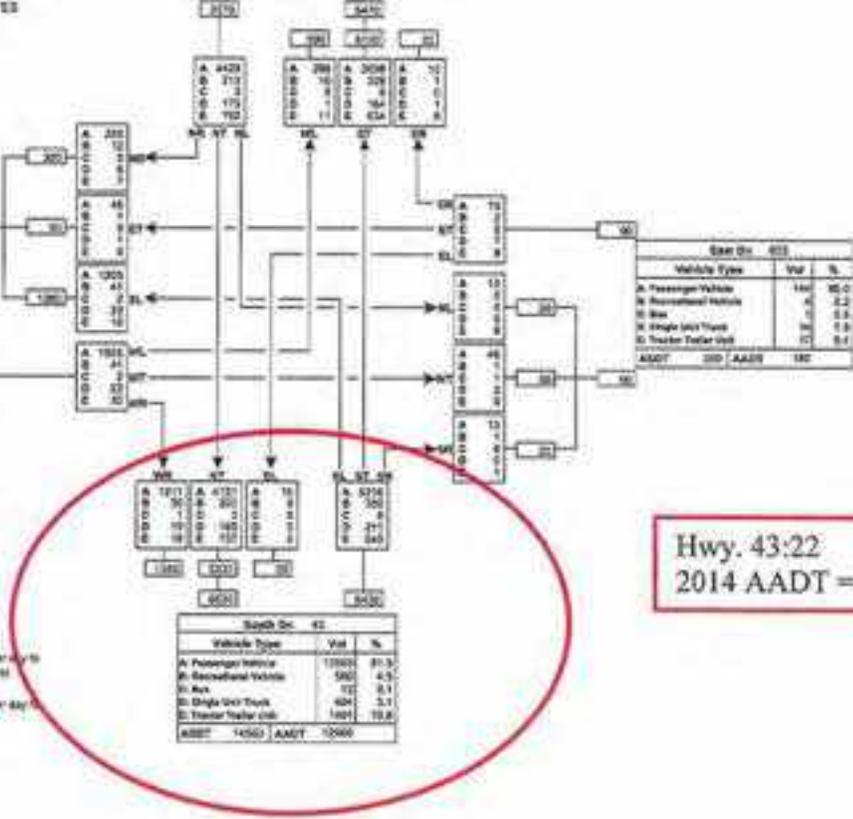
West On - 622			
Vehicle Type	Vol	%	
A. Passenger Vehicle	2127	84.2	
B. Recreational Vehicle	95	2.8	
C. Bus	4	0.1	
D. Single Unit Truck	37	1.8	
E. Tractor Trailer Unit	47	1.4	
AADT	2510	AADT	2510

TURNING MOVEMENT ABBREVIATIONS

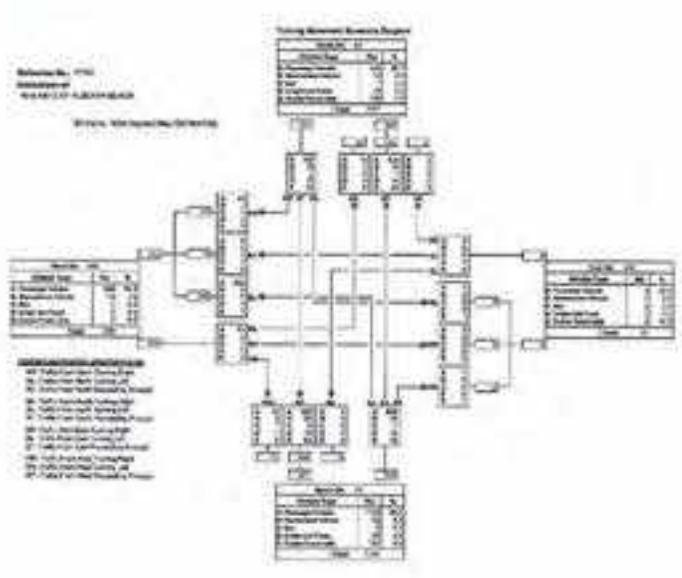
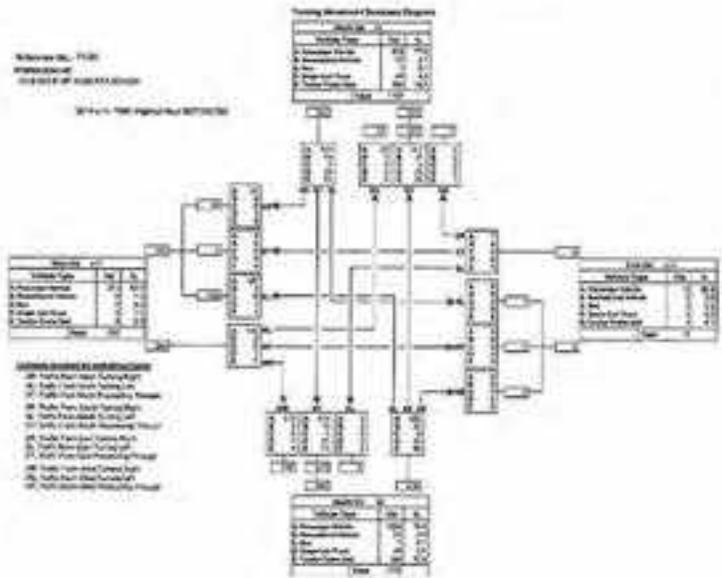
- NR Traffic From North Turning Right
- NL Traffic From North Turning Left
- NT Traffic From North Proceeding Through
- DR Traffic From South Turning Right
- DL Traffic From South Turning Left
- DT Traffic From South Proceeding Through
- ER Traffic From East Turning Right
- EL Traffic From East Turning Left
- ET Traffic From East Proceeding Through
- WR Traffic From West Turning Right
- WL Traffic From West Turning Left
- WT Traffic From West Proceeding Through

TURNING MOVEMENT ABBREVIATIONS

- AADT - Average Annual Daily Traffic
- Average daily traffic expressed as vehicles per day for period of January 1 to December 31 (365 days)
- ASDT - Average Summer Daily Traffic
- Average daily traffic expressed as vehicles per day for period of May 1 to September 30 (152 days)



Hwy. 43:22
2014 AADT = 12960



4.4 Traffic Counts

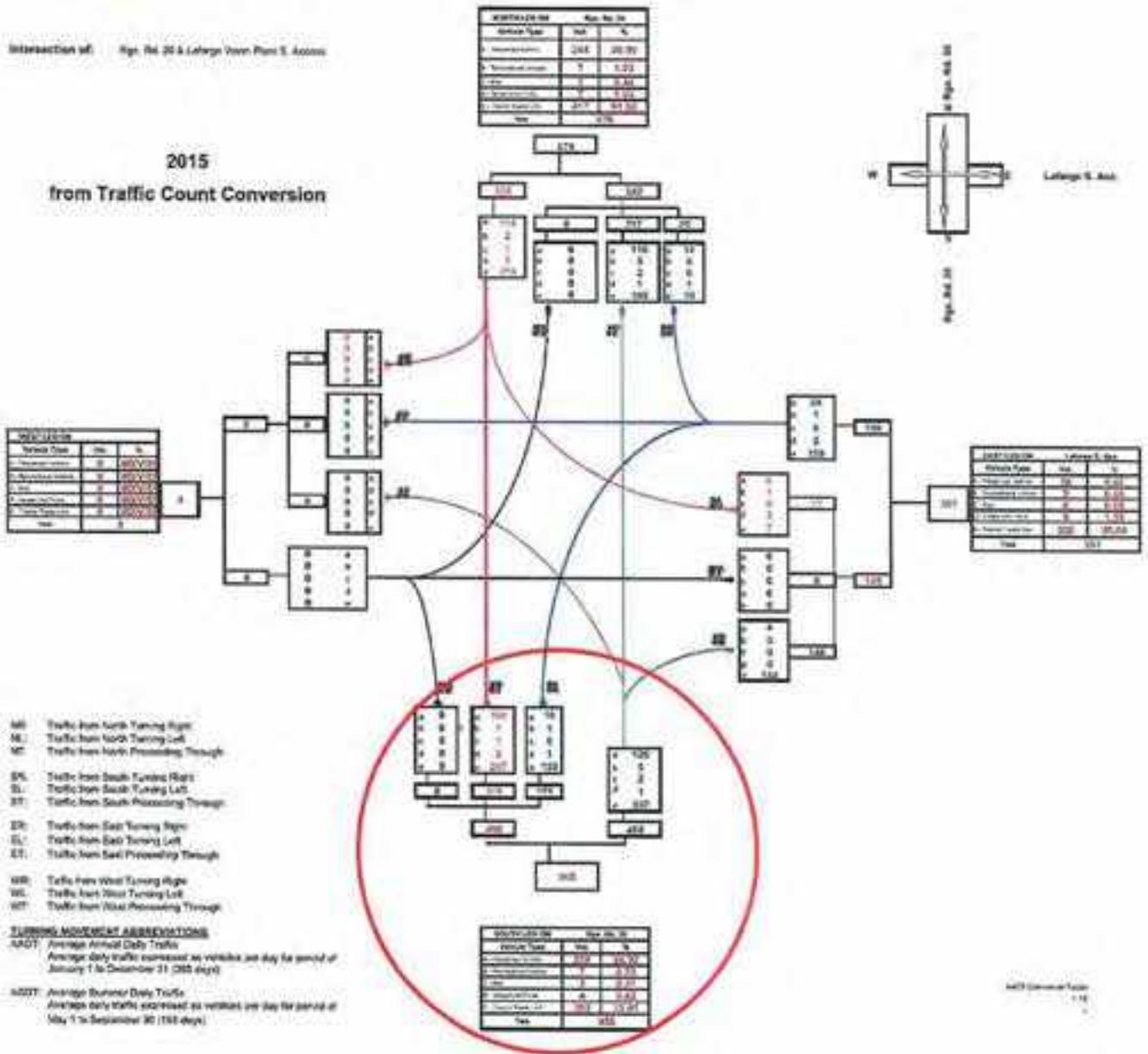
A traffic count was recently on Sept. 10, 2015, conducted by D&A Paulichuk Consulting Ltd. on Rge. Rd. 20, just north of the proposed development. The results indicate that the 2015 AADT for RR 20 at the development access is approximately 995.



Turning Movement Summary Diagram

Intersection of: Rge. Rd. 20 & Lehigh Town Plan 5 Access

2015
from Traffic Count Conversion



4.5 Traffic Growth - Hwy. 16:14

The following historical traffic data for Highway 16:14 is available from the Alberta Transportation's website, which indicates a growth of approximately 6.72% growth for the east leg of Hwy. 16 at RR 20, from 1989 to 2014 (25 years) and a 2.26% from 2007 to 2014 (7 years).

Historical Traffic Volumes – Hwy. 16:14

Year	2004	2005	2006	2007	2008	2009	2010	2011	2012	2013	2014
AADT	15630	16390	18760	20360	20480	21300	21660	20690	22400	23040	23580

Year	1993	1994	1995	1996	1997	1998	1999	2000	2001	2002	2003
AADT	9940	10130	10020	9760	10490	11280	11570	12830	13550	14290	14280

Year	1989	1990	1991	1992
AADT	8800	9060	9040	9250

Since there has been a noticeable growth rate change between the longer term and shorter term, 4.5% will be used for projecting the next 20 years.

4.6 Traffic Growth - Hwy. 43:22

The following historical traffic data for Highway 43:22 is available from the Alberta Transportation's website, which indicates a growth of 4.03% per annum non-compounded from 1986 to 2014 (28 years), 3.46% from 1994 to 2014 (20 years), 3.08% from 2004 to 2104 (10 years) and a 3.00% from 2009 to 2014 (5 years) for the south leg of Hwy. 43 from Hwy. 633.

Historical Traffic Volumes – Hwy. 43:22

Year	2004	2005	2006	2007	2008	2009	2010	2011	2012	2013	2014
AADT	9910	10230	11030	11770	11320	11280	11530	11510	12050	12270	12960

Year	1993	1994	1995	1996	1997	1998	1999	2000	2001	2002	2003
AADT	7390	7660	7700	7650	7910	8290	8290	8530	8920	9250	9600

Year	1984	1985	1986	1987	1988	1989	1990	1991	1992
AADT			6090	6490		6720	6850	6860	7000

A growth rate of 3.39% will be used for the next 20 years, which is the average of the four rates shown above.

4.7 Traffic Growth – Rge. Rd. 20 & Twp. Rd. 540

The following historical traffic data for Range Road 20 north of Hwy. 16 is available from the Alberta Transportation's website, which indicates a growth of approximately 0.80% growth for north leg off of Hwy. 16:14 intersection, from 2002 to 2014 (12 years).

Historical Traffic Volumes – Rge. Rd. 20

Year	2004	2005	2006	2007	2008	2009	2010	2011	2012	2013	2014
AADT	1810	1830	2280	2460	2540	2640	2680	1770	1840	1900	1940

Year	2002	2003
AADT	1770	1740

A growth rate of 1.0% per year will be used for Range Road 20 for analysis purposes.

For Twp. Rd. 540 between Hwy. 43 and RR 20, there is no historical data available. A growth rate of 1.0% per year will be used for Twp. Rd. 540 for analysis purposes.

4.8 Back Ground Traffic Turning Movement Diagrams

The background traffic turning movement diagrams are shown below for 2016.

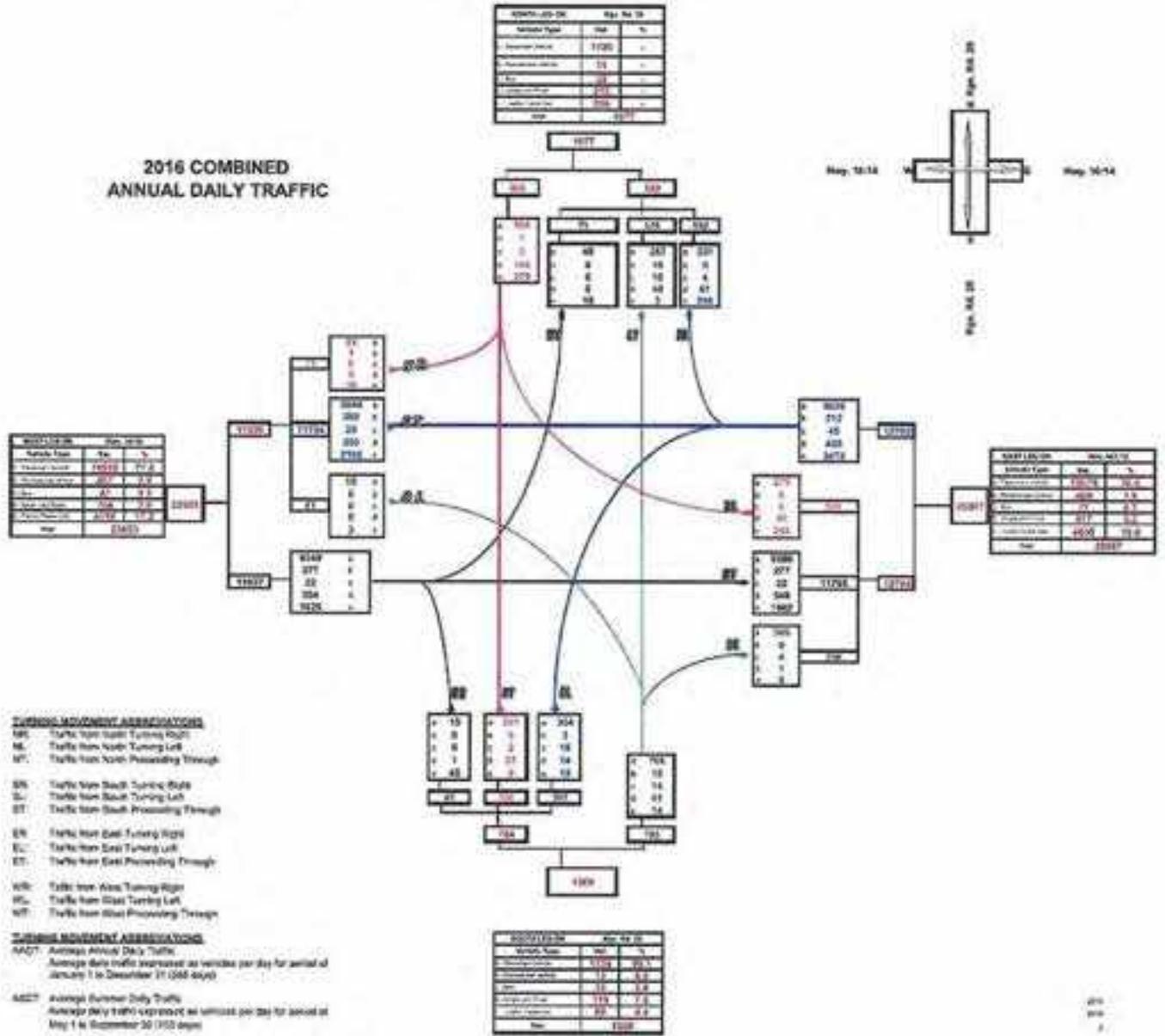
The 2014 AADT for Twp. Rd. 540 is estimated to be 600 to the west and 140 to the east based on existing development in the area.

TRAFFIC IMPACT ASSESSMENT

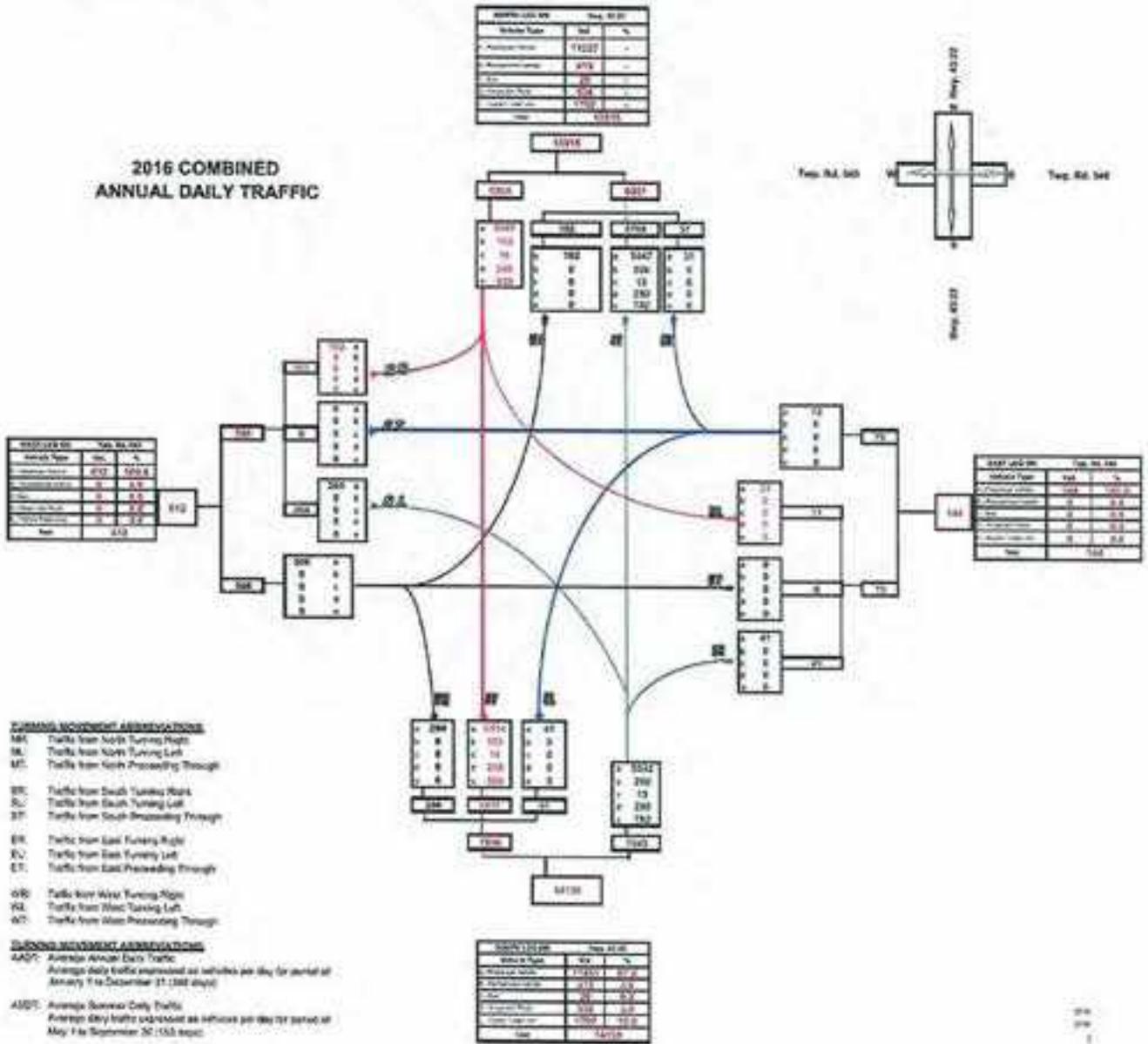
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TR 540, Hwy. 43:22, RR 20, Hwy. 16:14



Turning Movement Summary Diagram



Turning Movement Summary Diagram



5. ANALYSIS – DEVELOPMENT ACCESS & RGE. RD. 20

The proposed development is to access Range Road 20 to the east as shown below.



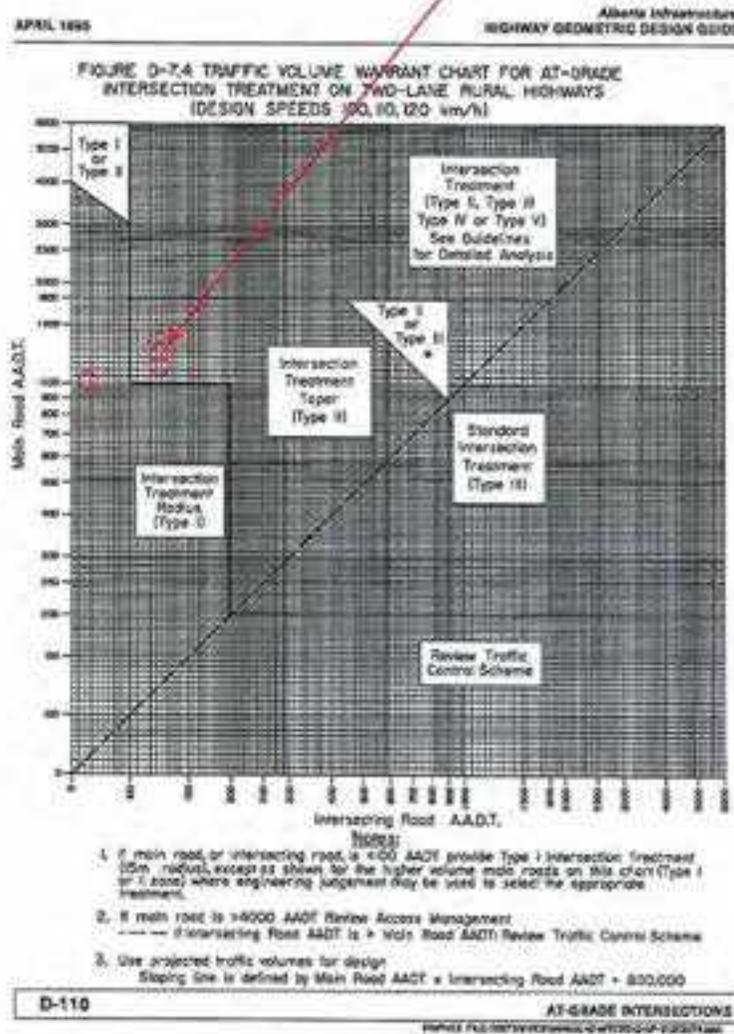
The projected traffic volumes for the next 20 years is as follows:

Existing Highway Traffic Forecast, Daily Volumes

Year	Range Road 20 South Leg	Dev. Ent. West Leg
2016	1043	29
2021 (5 Year)	1187	124
2026 (10 Year)	1239	124
2036 (20 Year)	1336	124

5.1 Initial Determination based on Traffic Volume Warrant Chart

The 2016, 2021, 2026 and 2036 AADT values for the Rge. Rd. 20 and Development Access intersection indicate from referencing Figure D-7.4, "Traffic Volume Warrant Chart for At-Grade Intersection Treatment on Two-Lane Rural Highways (Design Speeds 100/110/120 km/h)", that a Type II intersection maybe warranted for 2021, 2026 and 2036.



Due to the 80 km/hr. posted speed limit, a detailed analysis is required to further determine the appropriate intersection treatment type. It is important to note, that the above chart is for design speeds of 110 km/h and greater.

5.2 Design Speed

The posted speed limit on Range Road 20 is 80 km/hr. It is therefore reasonable to conclude that a design speed of 90 km/h is suitable.

5.3 Detailed Analysis

Right Turn

In accordance with Alberta Transportation's "Highway Geometric Design Guide" (Section D.7.7), an exclusive right turn lane is warranted on an undivided highway when all three of the following conditions are met:

- Main (or though) road AADT \geq 1,800
- Intersecting road AADT \geq 900
- Right turn daily traffic volume \geq 360 for the movement in question.

The following table indicates the status of these requirements for right turns from Range Road 20.

Table 5.3a - Right Turn Warrant

Condition	Base Year (2016)	5 Year (2021)	10 Year (2026)	20 Year (2036)
	(Condition Met)	(Condition Met)	(Condition Met)	(Condition Met)
Main Road (Rge. Rd. 20)	1043	1187	1239	1336
AADT \geq 1800	(No)	(No)	(No)	(No)
Intersecting Road (Dev. Ent.)	29	124	124	124
AADT \geq 900	(No)	(No)	(No)	(No)
Right turn daily traffic \geq 360	0	0	0	0
	(No)	(No)	(No)	(No)
For movement in question	(No)	(No)	(No)	(No)

Based on the projected volumes, an exclusive right turn lane is **not** required for the next 20 years with the added development traffic.

Left Turn

The Highway Geometric Design Guide Section D.7.6 gives graphical guidelines for determining left turn warrant. The graphs use peak (100th highest) hour volumes and factor in percent turning and design speed to identify the required treatment for the intersection. The following table shows the treatments needed for current and

projected traffic volumes.

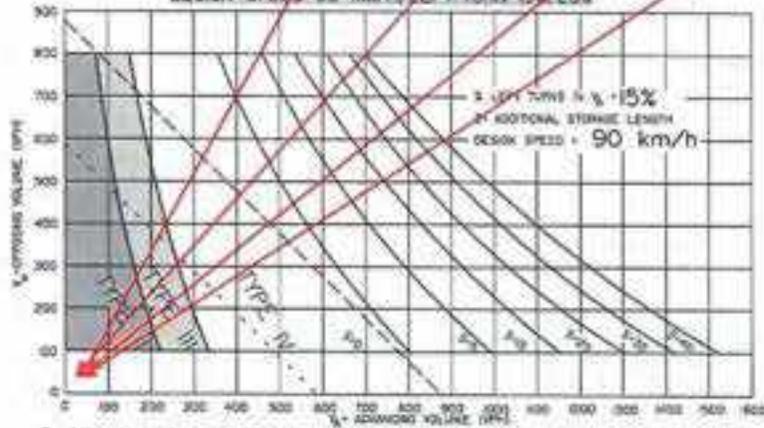
Table 5.3b - Required Treatment Type PM PEAK

	Base Year (2016)	5 Year (2021)	10 Year (2026)	20 Year (2036)
Peak 100th Hour - p.m.				
% Left Turns	5.9%	18.6%	18.2%	17.0%
V_a = Advancing Volume (VPH)	34	43	44	47
V₀ = Opposing Volume (VPH)	34	36	37	41
V_l = Left turning Volume (VPH)	2	8	8	8
Design Speed	90 km/hr	90 km/hr	90 km/hr	90 km/hr
Required Treatment Type	Type I	Type I	Type I	Type I

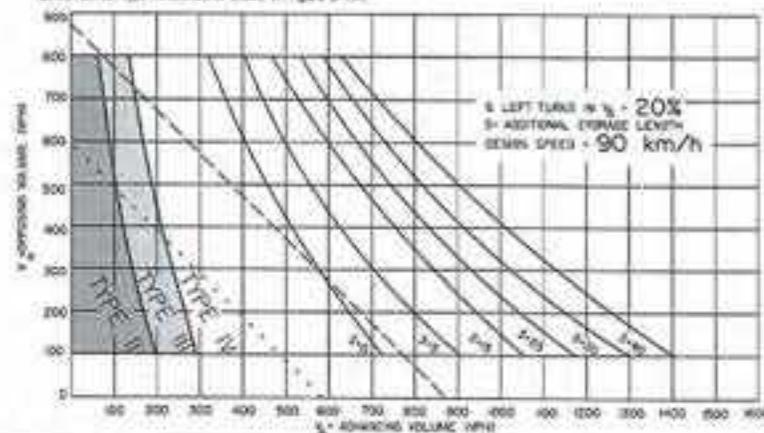
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AUGUST 1999

FIGURE D-7.6-5b WARRANTS FOR LEFT TURN TREATMENT AND STORAGE REQUIREMENTS FOR TWO-LANE HIGHWAYS
DESIGN SPEED: 90 KM/H LEFT TURN: 15% 2CS



S = additional storage length required, not L, is shown as the appropriate Type V storage showing. Designer should check additional storage requirements for trucks, see also Table D.7.6a.
 - - - Traffic signals may be warranted in rural areas or urban areas with restricted flow.
 --- Traffic signals may be warranted in "free flow" urban areas.
 Note:
 1. The traffic signal warrant lines are provided for reference only. For detailed analysis of the requirements for signals, consult Roadway Engineering Branch.
 2. Normal for Type I treatment is shown in Figure D-7.6a.



AT-GRADE INTERSECTIONS

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Intersection Analysis

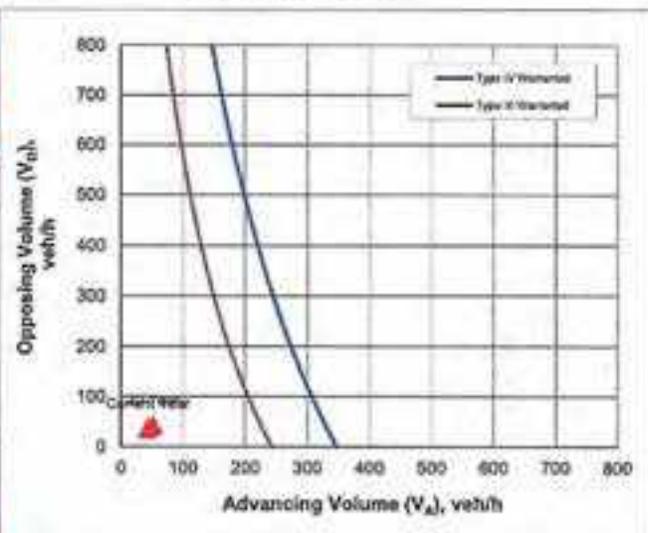
Main Rd: Range Road 20
Minor Rd: Development Entrance

Direction: NB
Count date or year: 2021

INPUT	Value
85 th percentile speed, km/h:	50
Left-turn volume (V_L), veh/h:	5
Advancing volume (V_A), veh/h:	43
Opposing volume (V_O), veh/h:	35
Yearly Linear Growth Rate, %:	1.0%

OUTPUT	Value
Percent of left-turns in advancing volume (V_L/V_A), %:	11.6%
20 year advancing volume, veh/h:	52
20 year opposing volume, veh/h:	43
Confidence:	98.80%
Conflicts per hour:	
% of advancing volume:	0.02%
veh/h:	0.01
Refer to FIGURE D-7.4 of the Highway Geometric Design Guide	

CALIBRATION CONSTANTS	
Variable	Value
Average time for making left-turn, s:	3.0
Critical headway, s:	5.0
Average time to clear, s:	1.5



The Alberta Transportation hourly analysis charts are both indicating a result to the far bottom, against the x-axis. This is indication that an intersection treatment above a Type I is not required since the opposing traffic and advancing traffic are minimal and provide significant opportunity to make a left turn. Based on this, a Type I intersection treatment is warranted between 2016 – 2036.

5.4 Intersectional Sight Distance

In accordance with section D.4, "Sight Distances at Intersections", the sight distance for left turning vehicles from the approach, without interfering with vehicles nearing the intersection, is used for determination of minimum sight distance requirements. Using Fig. D-4.2.2.2 below, the required sight distances for various vehicle types with a 110 km/hr design speed are as follows:

Vehicle Type	Required Sight Distance – 90 kph
Passenger Vehicle (P)	175 m
Single Unit or Bus (SU)	265 m
Semi-Trailer Combination (WB15)	350 m
Semi-Trailer Combination (WB21, WB23, WB28, WB33)	462 m

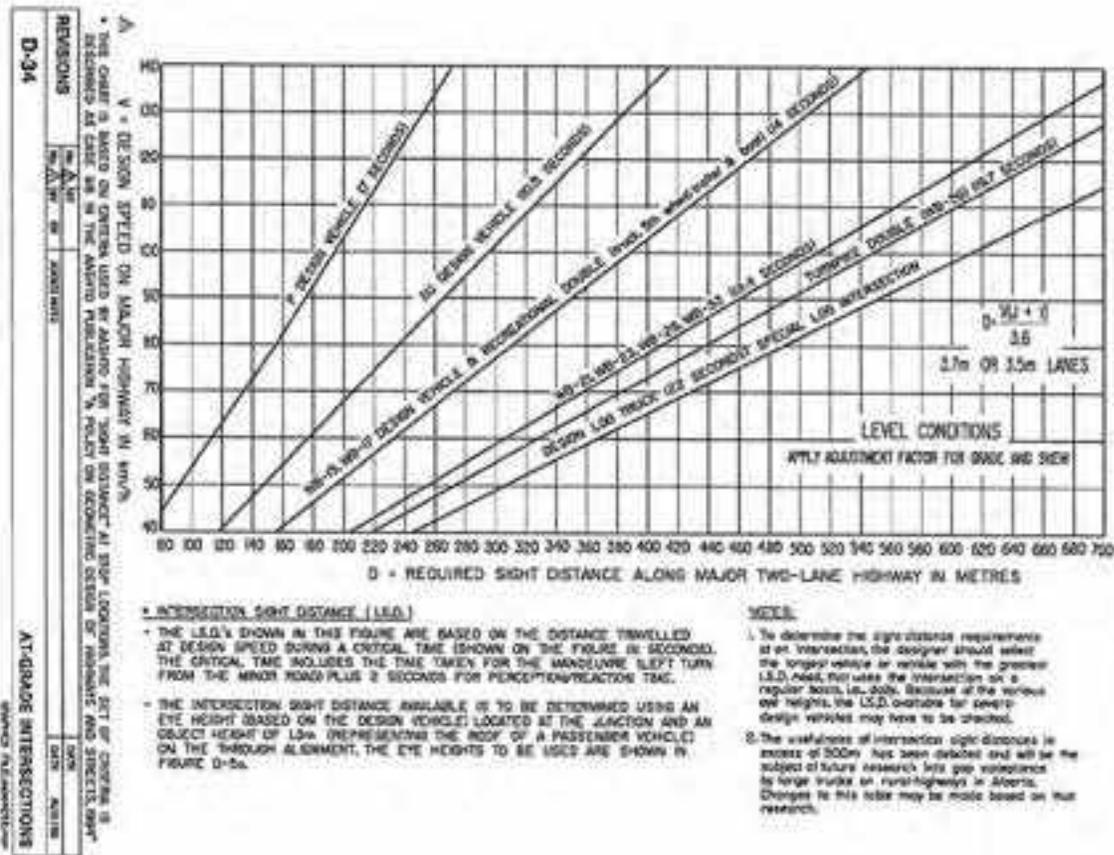


FIGURE D-4.2.2: SIGHT DISTANCES FOR LEFT TURN ONTO HIGHWAY

It is anticipated that the largest vehicle type to use the intersection will be WB-15 which requires 350m of sight distance. Presently there is more than 350m of sight distance available to the south and north of this intersection.

6. ANALYSIS – Hwy. 16:14 & RGE. Rd. 20

The following tables show the estimated combined traffic volumes at the intersection.

Combined Traffic Forecast, Daily Volumes

Year	Hwy. 16:14 East Leg Combined	Range Road 20 North Leg Combined
2016	25580	1994
2021	30590	2149
2026	35556	2246
2036	45668	2618

Projected peak hour traffic loading (100th highest hour) is shown below for am/pm:

Combined Traffic Forecast, Peak Hour Volumes

Year	Hwy. 16:14 East Leg Combined am/pm	Range Road 20 North Leg Combined am/pm
2016	2590 / 2601	229 / 135
2021	3080 / 3120	245 / 147
2026	3565 / 3638	256 / 153
2036	4542 / 4675	281 / 166

6.1 Design Speed

The posted speed on Highway 16:14 at this location is 110 km/hr. It is therefore reasonable to conclude that a design speed of 120 km/h is suitable.

6.2 Intersecting Road Classification

Intersection treatment on divided highways are characterized into three general formats:

- No Treatment; very low intersecting volumes (approx. less than 200 AADT)
- MINOR Roadway (i.e. Road Allowances)
- MAJOR Roadway (i.e. Highways, Town Access Roads, Park Access Roads)

However it is best to verify the proper treatment by completing a more detail analysis of each separate movement using the projected turning movement traffic volumes as per the following sections.

6.3 Detailed Analysis

6.3.1 Right Turn Lane

In accordance with Section D.8.7, "Warrants for Right Turn Lanes on Four-Lane Divided Highways", the following is apparent:

- The Right Turn Lane Volume must be at least 360 vehicles per day to warrant a Right Turn Lane. This is summarized in the following Table:

Table: Right Turn Lane Warrant

Condition	Base Year (2016)	5 Year (2021)	10 Year (2026)	20 Year (2036)
Range Road 20	(Condition Met)	(Condition Met)	(Condition Met)	(Condition Met)
Right turn daily traffic ≥ 360	598	651	679	915
For movement in question	(Yes)	(Yes)	(Yes)	(Yes)

Based on the projected volumes, a Right Turn Lane is presently warranted. A 100m right turn lane and 140m taper presently exists which satisfies this warrant.

6.3.2 Left Turn Lane

The Highway Geometric Design Guide Section D.8.6, "Warrants for Left Turn Lanes on Four-Lane Divided Highways" gives graphical guidelines for determining left turn warrant. The graphs use peak (100th highest) hour volumes and factor in percent turning and design speed to identify the required treatment for the intersection. The following table shows the treatments needed for current and projected traffic volumes.

Table: Required Treatment Type

	Base Year (2016)	5 Year (2021)	10 Year (2026)	20 Year (2036)
Range Road 20				
Peak am 100th Hour				
$V_t =$ Turning Volume (VPH)	7	8	8	10
$V_o =$ Opposing Volume (VPH)	1474	1768	2061	2649
Treatment Required	No	Yes	Yes	Yes

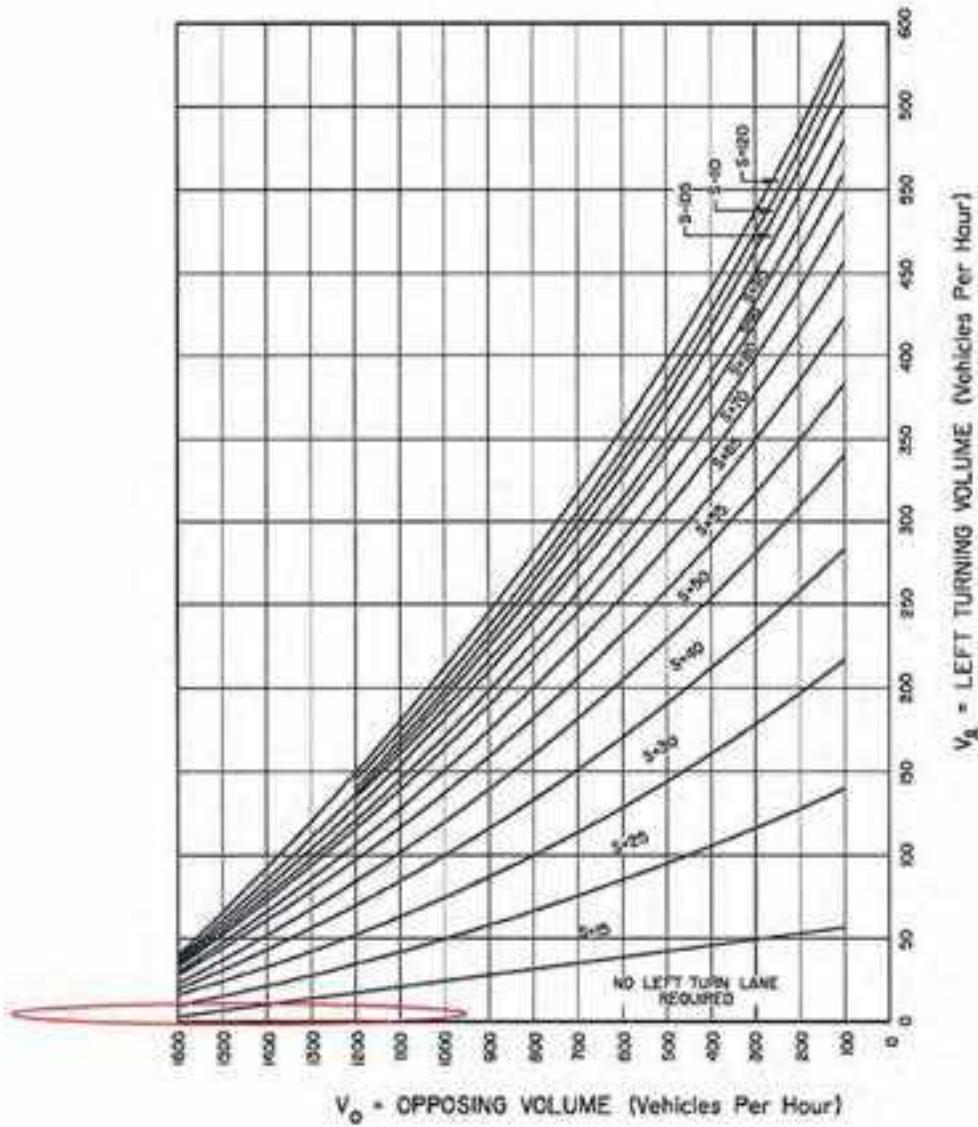
The above values are then used on Figure D-8.6c, "Warrants for Left Turn Lanes and Storage Requirements for Four-Lane Divided Highways", shown below.

Based on the projected volumes, a Left Turn Lane is not warranted until 2020 for the EB to NB left turning movement. A full left turn lane and taper already exists at this location and meets the requirements.

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FIGURE D-8.6c WARRANTS FOR LEFT TURN LANES AND STORAGE REQUIREMENTS FOR FOUR-LANE DIVIDED HIGHWAYS



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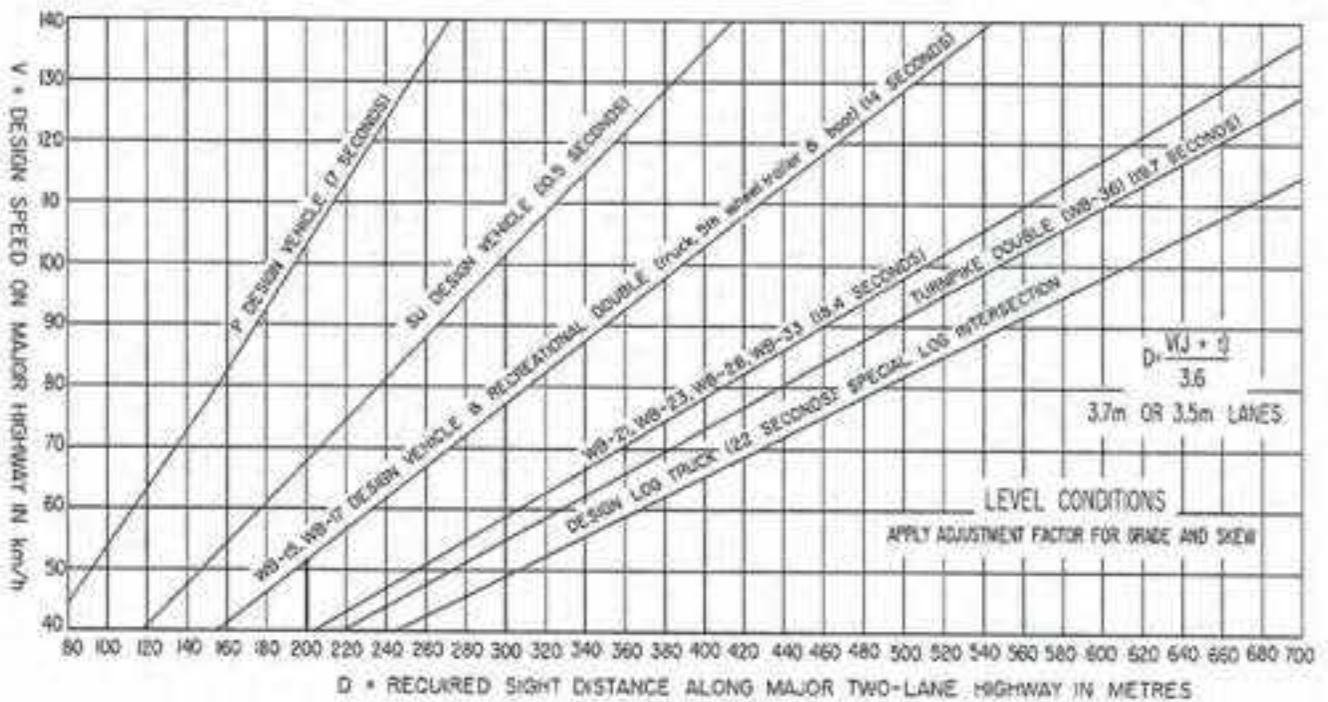
SUMMARY:

Presently, the intersection already contains a "Major Road Intersection on Four-Lane Divided Highway" intersection treatment and therefore no improvements are required for the next 20 years.

6.4 Intersectional Sight Distance

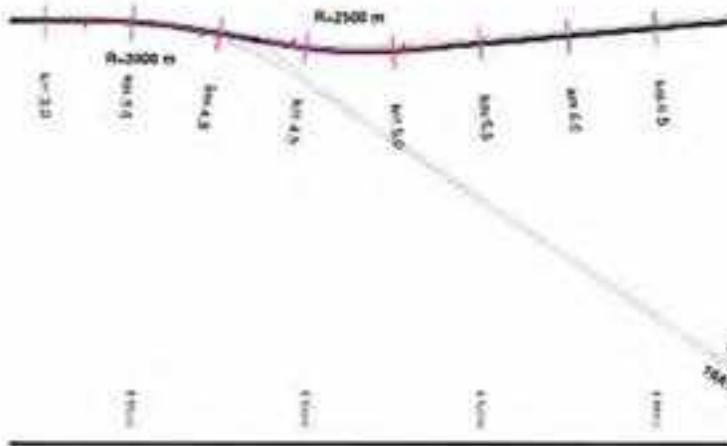
In accordance with section D.4, "Sight Distances at Intersections", the sight distance for left turning vehicles from the approach, without interfering with vehicles nearing the intersection, is used for determination of minimum sight distance requirements. Using Fig. D-4.2.2.2, shown in Appendix C, the required sight distances for various vehicle types with a 120 km/hr design speed are as follows:

Vehicle Type	Required Sight Distance – 120 km/hr.
Passenger Vehicle (P)	233 m
Single Unit or Bus (SU)	355 m
Semi-Trailer Combination (WB15)	470 m
Semi-Trailer Combination (WB21, WB23, WB28, WB33)	613 m



Hwy. 16:14 EBL's

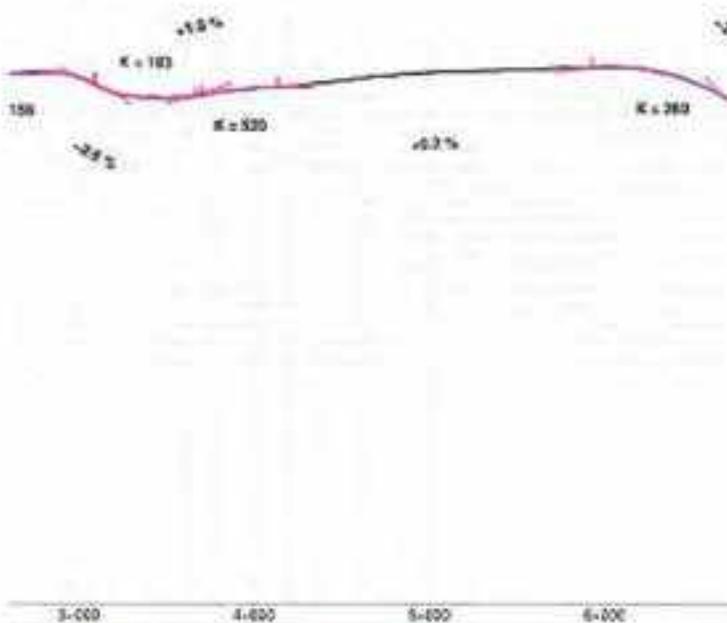
The intersection of Hwy. 16:14 and Range Road 20 is approximately at km 4.8.



The intersection is on a horizontal curve of 2500m radius alignment in an east west direction.

The intersection is on a grade of +0.2%.

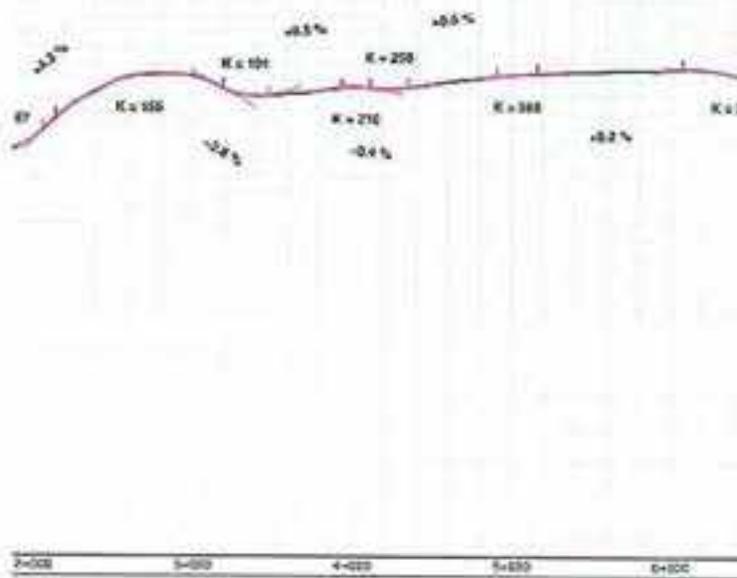
The sight distance from RR 20 to the west is >800m to the east.



Hwy. 16:14 WBL's



The sight distance from RR 20 to the east is >800m.



The site distance is greater than 650 metres in both directions on Hwy. 16 at the intersection of Range Road 20.

6.5 Highway Capacity Analysis

The capacity analysis was performed for the intersection for the 100th highest hour for the AM Peak for Years 2016, 2021, 2026 and 2036. The traffic analysis was completed using **Synchro 9** software based on HCM 2010 methodology. A saturation flow of 1700 vpl was used in this analysis. See the tables below for the results.



It is important to note that Synchro does not take into account acceleration lanes. As seen in the above schematic, deceleration lanes are shown but not acceleration lanes. Acceleration lanes exist for right turns coming out of Range Road 20 as well as a left turn acceleration for the SB to EB movement onto Hwy. 16.

For this reason, some of the results below will be incorrect.

TRAFFIC IMPACT ASSESSMENT

LAUNCH VENTURES RESIDENTIAL
 SE 1-54-2-W5M
 Lac Ste. Anne County
 TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

YEAR			Highway 16:14						Rge. Rd. 20	
TIME PERIOD	PARAMETERS	EB Left	EB Thru	EB Right	WB Left	WB Thru	WB Right	NB All Turns	SB All Turns	
Year 2016	AM Peak	LOS	B	A	A	C	A	A	D	F*
		Delay (s)	11.7	0.0	0.0	15.6	0.0	0.0	26.3	165.3
		v/c Ratio	0.01	0.44	0.00	0.06	0.29	0.04	0.45	1.10
	PM Peak	95 th Que	0.4	0.0	0.0	1.4	0.0	0.0	17.9	69.7
		LOS	C	A	A	B	A	A	E	E*
		Delay (s)	16.1	0.0	0.0	12.9	0.0	0.0	42.6	47.1
		v/c Ratio	0.02	0.35	0.01	0.06	0.45	0.03	0.48	0.35
Year 2021	AM Peak	95 th Que	0.6	0.0	0.0	1.5	0.0	0.0	18.2	11.5
		LOS	B	A	A	C	A	A	E	F*
		Delay (s)	13.2	0.0	0.0	19.8	0.0	0.0	42.7	548.6
	PM Peak	v/c Ratio	0.02	0.53	0.00	0.08	0.35	0.04	0.62	1.96
		95 th Que	0.4	0.0	0.0	2.1	0.0	0.0	29.1	119.4
		LOS	C	A	A	C	A	A	F	F*
		Delay (s)	20.4	0.0	0.0	15.8	0.0	0.0	93.5	98.6
Year 2026	AM Peak	v/c Ratio	0.04	0.43	0.01	0.08	0.54	0.03	0.74	0.60
		95 th Que	0.9	0.0	0.0	2.1	0.0	0.0	32.8	21.8
		LOS	C	A	A	D	A	A	F	F*
	PM Peak	Delay (s)	15.1	0.0	0.0	25.8	0.0	0.0	84.0	>500
		v/c Ratio	0.02	0.62	0.00	0.11	0.41	0.04	0.84	4.71
		95 th Que	0.6	0.0	0.0	3.0	0.0	0.0	47.5	>250
		LOS	D	A	A	C	A	A	F	F*
Year 2036	AM Peak	Delay (s)	26.5	0.0	0.0	18.4	0.0	0.0	241.2	247.1
		v/c Ratio	0.05	0.49	0.01	0.11	0.63	0.15	1.15	1.00
		95 th Que	1.0	0.0	0.0	2.6	0.0	0.0	47.8	30.2
Year 2036	AM Peak	LOS	C	A	A	E	A	A	F	F*
		Delay (s)	20.5	0.0	0.0	49.4	0.0	0.0	409.2	>500
		v/c Ratio	0.04	0.80	0.00	0.23	0.53	0.05	1.65	
		95 th Que	1.0	0.0	0.0	6.6	0.0	0.0	103.1	>250

TRAFFIC IMPACT ASSESSMENT

LAUNCH VENTURES RESIDENTIAL
SE 1-54-2-W5M
Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

	LOS	E	A	A	D	A	A	F	F*
PM Peak	Delay (s)	49.2	0.0	0.0	28.8	0.0	0.0	>500	>500
	v/c Ratio	0.12	0.63	0.01	0.19	0.82	0.03	4.31	
	95 th Que	3.1	0.0	0.0	5.4	0.0	0.0	>250	>250

*Note: The SB turns are showing a LOS of F and high delay times which is inaccurate since Synchro does not provide calculation for efficiencies due to acceleration lanes.

As noted above, the above results indicate that the southbound turns are all at a Level of Service (LOS) of F. This does not represent the actual operation on site since a 120m left turn acceleration and 120m taper exists for the southbound to eastbound movement. In order to validate this, a site inspection was conducted on Sept. 8 to 10, 2015, and the wait times were measured. The results indicated an average wait time of 20 seconds to 55 seconds during the AM Peak (average approximately 35 seconds). This is significantly lower than the Synchro result of 126.7 seconds shown above. Again, this is likely due to the fact that Synchro does not use calculations that account for acceleration lanes. Therefore in extrapolating this analysis, the NB and SB turns will both likely be at a LOS of E in 2020. In 2025, the NB and SB turns will both likely be at a LOS of F. The proposed development fractionally increased the results above, since it only adds 4 left turns in the AM Peak Hour and 2 left turns in the PM Peak Hour.

Therefore, in summary, the intersection is performing near capacity presently. Full capacity may be reached within the next 10 – 15 years, mostly depending on the growth of Highway 16. The intersection should be monitored in about 5 - 10 years to further evaluate the operation and safety.

LEVEL OF SERVICE (LOS) CRITERIA

Control Delay Per Vehicle (s)	LOS by Volume to Capacity Ratio	
	≤1	>1
≤10	A	F
>10 and ≤15	B	F
>15 and ≤25	C	F
>25 and ≤35	D	F
>35 and ≤50	E	F
>50	F	F



Level of Service "A"



Level of Service "B"



Level of Service "C"



Level of Service "D"



Level of Service "E"



Level of Service "F"

6.6 Future Freeway Planning

It is important to complete a review on how the traffic in the area will be integrated into the future freeway system. Highway 16 is designated as a future freeway in Alberta. Access to this future freeway will only be made via a grade separation or interchange. Presently, Alberta Transportation's long range plan is to construct an interchange at the Highway 16 and Range Road 20 intersection location. See below:



Therefore, the present Range Road 20 will be directly connected to the future interchange. No re-routing of Range Road 20 will be required. The timing of this improvement is not known at this time. Traffic volume on Range Road 20 is likely to increase due to the consolidation of traffic in the area occurs with the implementation of the interchange.

7. ANALYSIS – Hwy. 43:22 & Twp. Rd. 540

The following tables show the estimated combined traffic volumes at the intersection.

Combined Traffic Forecast, Daily Volumes

Year	Hwy. 43:22 South Leg Combined	Twp. Rd. 540 East Leg Combined
2016	14147	156
2021	16364	200
2026	18554	208
2036	22935	222

Projected peak hour traffic loading (100th highest hour) is shown below for am/pm:

Combined Traffic Forecast, Peak Hour Volumes

Year	Hwy. 43:22 South Leg Combined am/pm	Twp. Rd. 540 East Leg Combined am/pm
2016	1437 / 1442	15 / 15
2021	1661 / 1667	18 / 19
2026	1882 / 1890	18 / 19
2036	2329 / 2338	22 / 23

7.1 Design Speed

The posted speed on Highway 43:22 at this location is 110 km/hr. It is therefore reasonable to conclude that a design speed of 120 km/h is suitable.

7.2 Intersecting Road Classification

Intersection treatment on divided highways are characterized into three general formats:

- No Treatment; very low intersecting volumes (approx. less than 200 AADT)
- MINOR Roadway (i.e. Road Allowances)
- MAJOR Roadway (i.e. Highways, Town Access Roads, Park Access Roads)

However it is best to verify the proper treatment by completing a more detail analysis of each separate movement using the projected turning movement traffic volumes as per the following sections.

7.3 Detailed Analysis

7.3.1 Right Turn Lane

In accordance with Section D.8.7, "Warrants for Right Turn Lanes on Four-Lane Divided Highways", the following is apparent:

- The Right Turn Lane Volume must be at least 360 vehicles per day to warrant a Right Turn Lane. This is summarized in the following Table:

Table: Right Turn Lane Warrant

Condition	Base Year (2016)	5 Year (2021)	10 Year (2026)	20 Year (2036)
Twp. Rd. 540	(Condition Met)	(Condition Met)	(Condition Met)	(Condition Met)
Right turn daily traffic ≥ 360	45	60	62	66
For movement in question	(No)	(No)	(No)	(No)

Based on the projected volumes, a Right Turn Lane is not warranted for the next 20 years.

7.3.2 Left Turn Lane

The Highway Geometric Design Guide Section D.8.6, "Warrants for Left Turn Lanes on Four-Lane Divided Highways" gives graphical guidelines for determining left turn warrant. The graphs use peak (100th highest) hour volumes and factor in percent turning and design speed to identify the required treatment for the intersection. The following table shows the treatments needed for current and projected traffic volumes.

Table: Required Treatment Type

	Base Year (2016)	5 Year (2021)	10 Year (2026)	20 Year (2036)
Twp. Rd. 540				
Peak PM 100th Hour				
V_t = Turning Volume (VPH)	3	4	4	5
V_o = Opposing Volume (VPH)	718	830	941	1165
Treatment Required	No	No	No	Yes

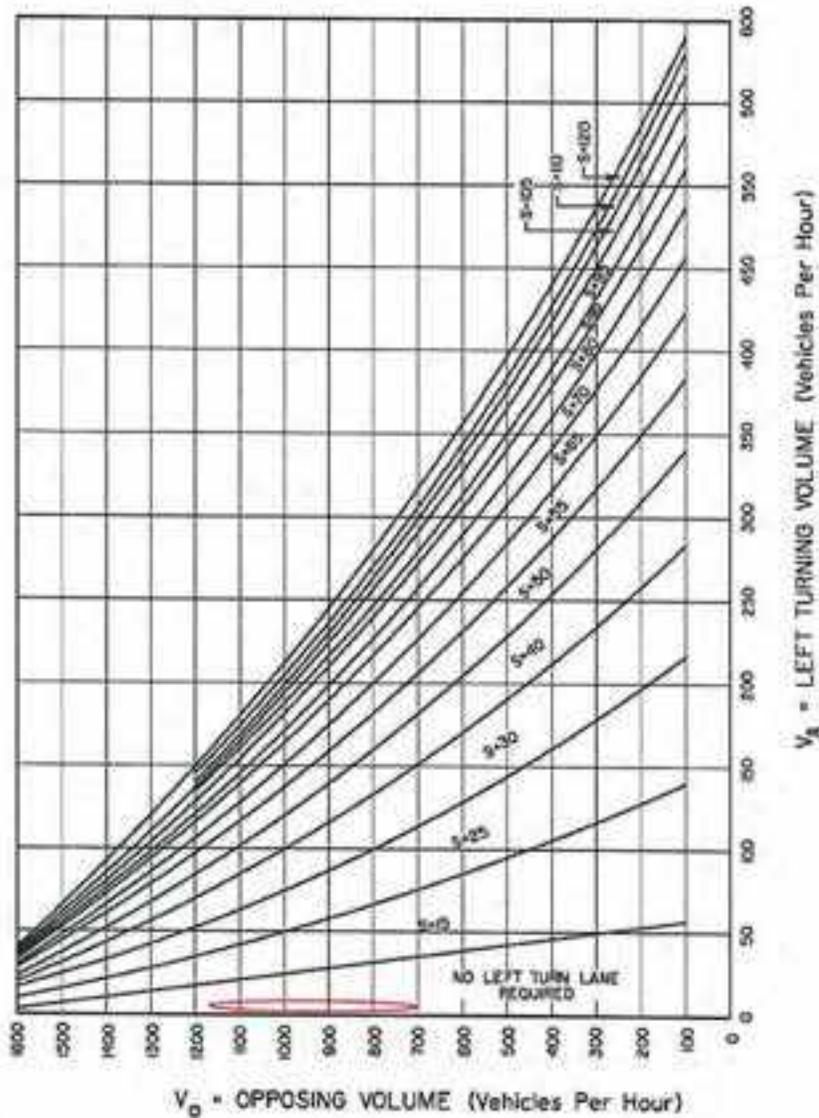
The above values are then used on Figure D-8.6c, "Warrants for Left Turn Lanes and Storage Requirements for Four-Lane Divided Highways", shown below.

Based on the projected volumes, a Left Turn Lane is not warranted for the next 20 years for the BB to EB left turning movement. A full left turn lane and taper already exists at this location and exceeds this requirement.

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FIGURE D-8.6c WARRANTS FOR LEFT TURN LANES AND STORAGE REQUIREMENTS FOR FOUR-LANE DIVIDED HIGHWAYS



D-180

AT-GRADE INTERSECTIONS

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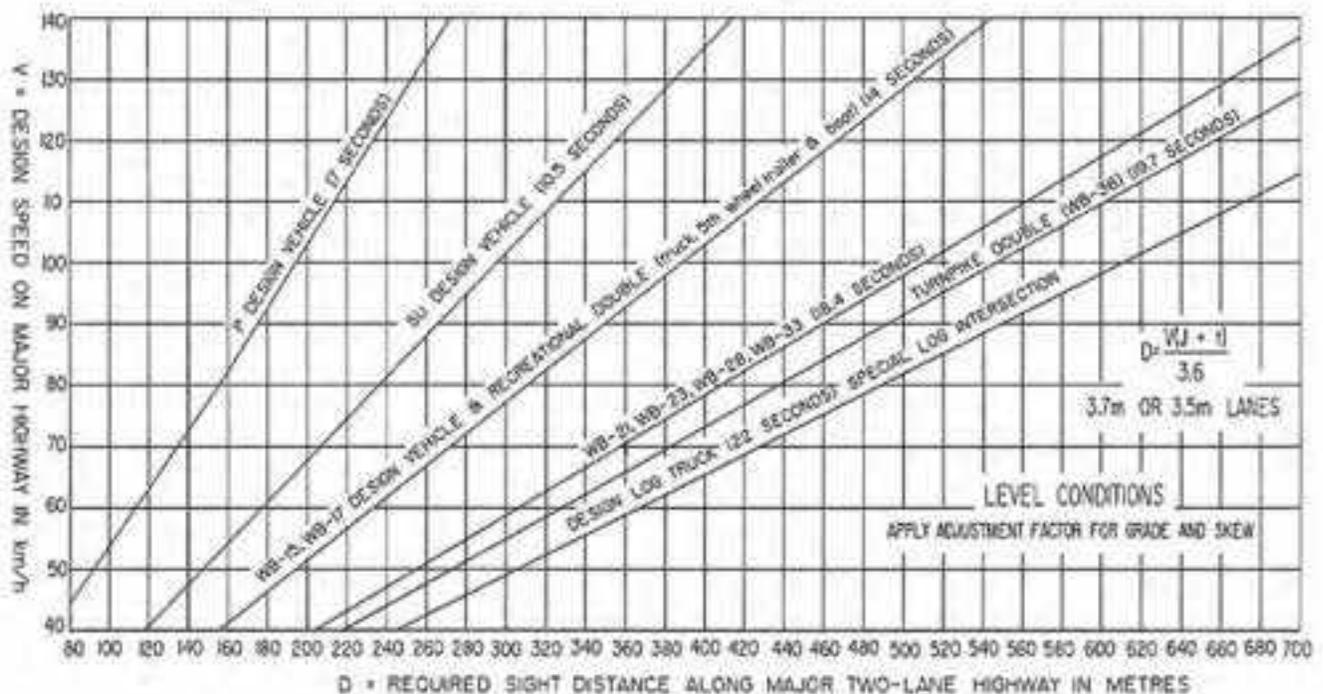
SUMMARY:

Presently, the intersection already contains a left turn lane for SB to EB left turns. There is no right turn lane and tapers for NB to EB, however this not warranted due to the low turning volumes. Therefore no improvements are required for the next 20 years.

7.4 Intersectional Sight Distance

In accordance with section D.4, "Sight Distances at Intersections", the sight distance for left turning vehicles from the approach, without interfering with vehicles nearing the intersection, is used for determination of minimum sight distance requirements. Using Fig. D-4.2.2.2, shown in Appendix C, the required sight distances for various vehicle types with a 120 km/hr design speed are as follows:

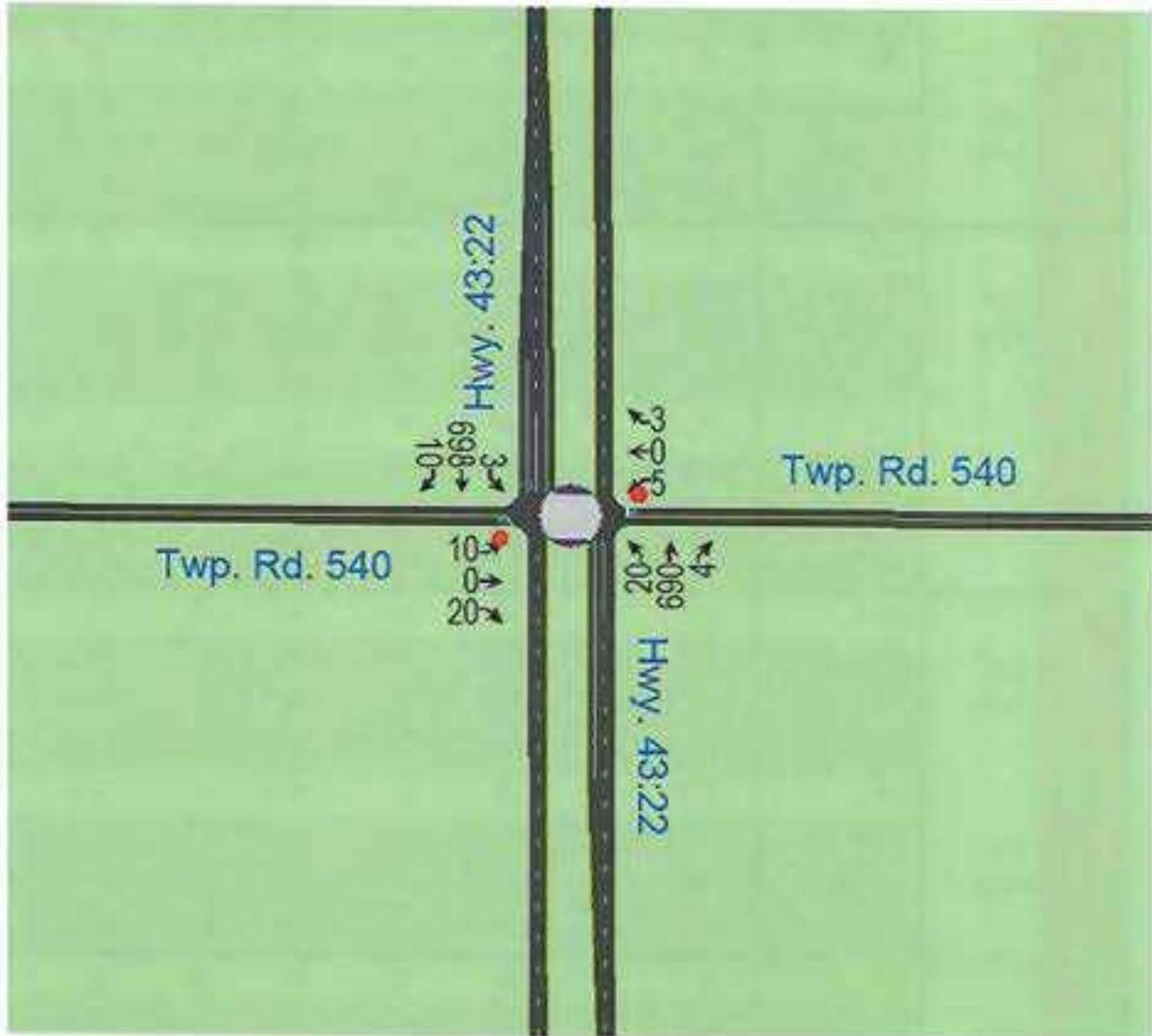
Vehicle Type	Required Sight Distance – 120 km/hr.
Passenger Vehicle (P)	233 m
Single Unit or Bus (SU)	355 m
Semi-Trailer Combination (WB15)	470 m
Semi-Trailer Combination (WB21, WB23, WB28, WB33)	613 m



The sight distance from Twp. Rd. 540 to the south and north is >650m.

7.5 Highway Capacity Analysis

The capacity analysis was performed for the intersection for the 100th highest hour for the AM Peak for Years 2016, 2021, 2026 and 2036. The traffic analysis was completed using **Synchro 9** software based on HCM 2010 methodology. A saturation flow of 1700 vpl was used in this analysis. See the tables below for the results.



TRAFFIC IMPACT ASSESSMENT

LAUNCH VENTURES RESIDENTIAL
SE 1-54-2-W5M
Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

YEAR			Highway 43:22						Twp. Rd. 540	
TIME PERIOD	PARAMETERS	NB Left	NB Thru	NB Right	SB Left	SB Thru	SB Right	EB All Turns	WB All Turns	
Year 2016	AM Peak	LOS	A	A	A	A	A	A	B	B
		Delay (s)	9.5	0.0	0.0	9.3	0.0	0.0	13.4	15.0
		v/c Ratio	0.03	0.29	0.15	0.00	0.22	0.01	0.07	0.02
		95 th Que	0.7	0.0	0.0	0.1	0.0	0.0	1.8	0.5
	PM Peak	LOS	A	A	A	A	A	A	B	B
		Delay (s)	9.5	0.0	0.0	9.4	0.0	0.0	13.4	14.6
		v/c Ratio	0.03	0.30	0.15	0.00	0.22	0.01	0.07	0.02
		95 th Que	0.7	0.0	0.0	0.1	0.0	0.0	1.8	0.4
		LOS	B	A	A	A	A	A	B	C
Year 2021	AM Peak	Delay (s)	10.0	0.0	0.0	9.8	0.0	0.0	14.7	16.9
		v/c Ratio	0.03	0.34	0.17	0.00	0.26	0.01	0.09	0.04
		95 th Que	0.8	0.0	0.0	0.1	0.0	0.0	2.3	0.9
		LOS	B	A	A	A	A	A	B	C
	PM Peak	Delay (s)	10.0	0.0	0.0	9.9	0.0	0.0	14.8	16.2
		v/c Ratio	0.03	0.34	0.18	0.01	0.26	0.01	0.09	0.03
		95 th Que	0.8	0.0	0.0	0.1	0.0	0.0	2.3	0.7
		LOS	B	A	A	B	A	A	C	C
		Delay (s)	10.6	0.0	0.0	10.4	0.0	0.0	16.2	19.0
Year 2026	AM Peak	v/c Ratio	0.04	0.39	0.20	0.00	0.29	0.01	0.10	0.04
		95 th Que	0.9	0.0	0.0	0.1	0.0	0.0	2.7	1.0
		LOS	B	A	A	B	A	A	C	C
		Delay (s)	10.6	0.0	0.0	10.4	0.0	0.0	16.3	18.2
	PM Peak	v/c Ratio	0.04	0.39	0.20	0.01	0.29	0.01	0.10	0.03
		95 th Que	0.9	0.0	0.0	0.1	0.0	0.0	2.7	0.8
		LOS	B	A	A	B	A	A	C	C
		Delay (s)	12.1	0.0	0.0	11.7	0.0	0.0	20.4	24.6
		v/c Ratio	0.05	0.48	0.24	0.01	0.36	0.01	0.14	0.07
Year 2036	AM Peak	95 th Que	1.2	0.0	0.0	0.2	0.0	0.0	3.9	1.7

TRAFFIC IMPACT ASSESSMENT

LAUNCH VENTURES RESIDENTIAL
SE 1-54-2-W5M
Lac Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

	LOS	B	A	A	B	A	A	C	C
PM Peak	Delay (s)	12.1	0.0	0.0	11.7	0.0	0.0	20.5	24.1
	v/c Ratio	0.05	0.48	0.25	0.01	0.37	0.01	0.14	0.06
	95 th Que	1.2	0.0	0.0	0.2	0.0	0.0	4.0	1.5

The above results indicate that the intersection will operate with sufficient capacity for the next 20 years.

7.6 Future Freeway Planning

It is important to complete a review on how the traffic in the area will be integrated into the future freeway system. Highway 43 is designated as a future freeway in Alberta. Access to this future freeway will only be made via a grade separation or interchange. Presently, Alberta Transportation's long range plan is to construct an interchange at the Highway 43 and Hwy. 633 intersection location. The at-grade Hwy. 43:22 and Twp. Rd. 540 intersection will be closed. This will require the re-direction of traffic via service roads and local roads in order to access the nearest interchange. Twp. Rd. 540 will likely be one of these collector roads that will increase in traffic due to these future changes.



8. ILLUMINATION & SIGNALIZATION WARRANTS

Street lighting presently exists at the intersection of Hwy. 16:14 and Rge. Rd. 20. Signalization is not allowed on Hwy. 16 as it is designated as a future freeway.

Street lighting does not exist at the intersection of Hwy. 43:22 and Twp. Rd. 540. Illumination warrants were reviewed. The results are summarized in the Table below:

Location	Year	Illumination Warrant Score	Illumination Warrant Met? (Min. 120)	Signalization Warrant Score	Signalization Warrant Met?
Hwy. 43:22 & Twp. Rd. 540	2016	96	No	Not Allowed	N/A
Hwy. 43:22 & Twp. Rd. 540	2021	96	No	Not Allowed	N/A
Hwy. 43:22 & Twp. Rd. 540	2026	96	No	Not Allowed	N/A
Hwy. 43:22 & Twp. Rd. 540	2036	96	No	Not Allowed	N/A

Illumination is not warranted at the Hwy. 43:22 & Twp. Rd. 540 intersection. Signalization is not allowed on Hwy. 43 as it is designated as a future freeway.

For the other intersections along Range Road 20 and Township Road 540, there is no requirement for illumination or signalization since the traffic volumes are too low, as demonstrated by the warranted intersection treatments discussed earlier.

9. CONCLUSIONS & RECOMMENDATIONS

9.1 Conclusions & Recommendations

The proposed development is for a 13 lot Residential rural subdivision within SE 1-54-2-W5M. The site will utilize Range Road 20 for access to the east and then south on Rge. Rd. 20 to the south to Hwy 16 or west on Twp. Rd. 540 (Heatherdown Road) to Hwy. 43. See below:



For the **Residential** development lots, ITE 210 – "Single-Family Detached Housing" Land Use from the Institute of Transportation Engineers (ITE) Trip Generation Manual, 9th Edition, can be used. The following data is available from the manual:

Time Period	Trips per Lot
Daily	9.52
Peak Hour of Adjacent Street Traffic One Hour Between 7 and 9 a.m.	0.75
Peak Hour of Adjacent Street Traffic One Hour Between 4 and 6 p.m.	1.00

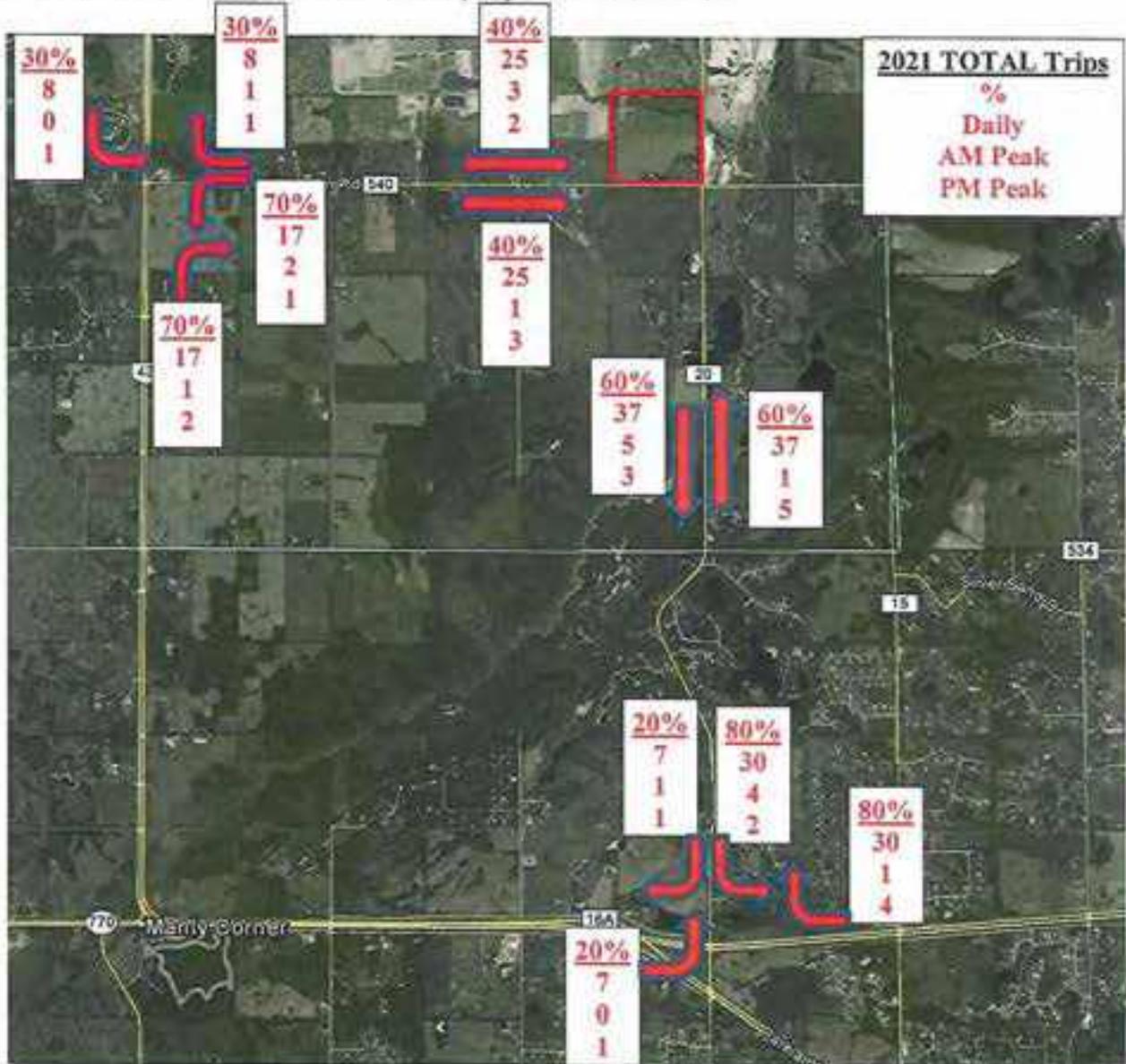
TRAFFIC IMPACT ASSESSMENT

LAUNCH VENTURES RESIDENTIAL
 SE 1-54-2-W5M
 Lac Ste. Anne County
 TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

Therefore, the estimated trip generation of the proposed development with 13 new lots is as follows:

Daily Traffic:	13 lots x 9.52 trips/day	= 124 trips per day
AM Peak Hour:	13 lots x 0.75 trips/hour	= 10 trips per hour
PM Peak Hour:	13 lots x 1.00 trips/hour	= 13 trips per hour

It is estimated that 3 lots will be occupied in 2016 and the remaining 10 lots will be occupied in 2021 (Year 5). The trip distribution is anticipated to be 40% to the west on Twp. Rd. 540 to Hwy. 43 and 60% to the south on Range Road 20 to Hwy. 16, with 25% entering/75% exiting in the AM Peak and 63% entering/37% exiting in the PM Peak. The distribution of traffic for 2012 is projected as follows:



A detailed intersection analysis was completed on three potentially impacted intersections. The analysis concluded the following:

Development Access and Range Road 20 intersection

- A Type I intersection treatment is warranted between 2016 – 2036.
- It is anticipated that the largest vehicle type to use the intersection will be WB-15 which requires 350m of sight distance. Presently there is more than 350m of sight distance available to the south and north of this intersection.
- A highway capacity analysis was not performed at this location as the traffic volumes are low and the intersection should have sufficient capacity above a LOS of D for the next 20 years.
- Illumination or signalization is not required for this location as the traffic volumes are too low.

Range Road 20 and Highway 16:14 intersection

- Presently, the intersection already contains a "Major Road Intersection on Four-Lane Divided Highway" intersection treatment with left turn acceleration lane for SB to EB left turns. No improvements are required for the next 20 years.
- The site distance is greater than 650 metres in both directions on Hwy. 16 at the intersection of Range Road 20.
- Upon completing a highway capacity analysis, the intersection is performing near capacity presently. Full capacity maybe reached within the next 10 – 15 years, mostly depending on the growth of Highway 16. The intersection should be monitored in about 5 - 10 years to further evaluate the operation and safety.
- Street lighting presently exists at the intersection of Hwy. 16:14 and Rge. Rd. 20. Signalization is not allowed on Hwy. 16 as it is designated as a future freeway.

Township Road 540 and Highway 43:22 intersection

- No improvements are required for the next 20 years.
- The sight distance from Twp. Rd. 540 to the south and north is >650m.
- Upon completing a highway capacity analysis, the intersection will operate with sufficient capacity for the next 20 years.

- Illumination is not warranted at the Hwy. 43:22 & Twp. Rd. 540 intersection. Signalization is not allowed on Hwy. 43 as it is designated as a future freeway.

The internal locals roads, Range Road 20 and Township Road 540 appear to be acceptable for transporting the proposed development traffic and are within acceptable parameters of the road standards for rural collector roadways.

9.2 Closure

We trust the information provided meets your present requirements. Should any questions arise, please contact our office at your convenience.



Darcy O. Paulichuk, P. Eng.



Dec. 9, 2015

APEGGA Permit to Practice Number: P12132

APPENDIX A

**ALBERTA HIGHWAYS TRAFFIC VOLUME HISTORY
TRAFFIC DATA – TURNING MOVEMENT DIAGRAMS**

Turning Movement Summary Diagram

Reference No.: 72510
 Intersection of:
 16 & 43 AT MANLY CORNER

2014 AADT / ASDT ESTIMATES

North On 43	
Vehicle Type	Vol
A: Passenger Vehicle	10725
B: Recreational Vehicle	204
C: Bus	41
D: Single Unit Truck	605
E: Tractor Trailer Unit	1785
ASDT	15000
AADT	13360

West On 16	
Vehicle Type	Vol
A: Passenger Vehicle	14056
B: Recreational Vehicle	250
C: Bus	94
D: Single Unit Truck	824
E: Tractor Trailer Unit	2487
ASDT	21730
AADT	17710

West On 16	
Vehicle Type	Vol
A: Passenger Vehicle	14056
B: Recreational Vehicle	250
C: Bus	94
D: Single Unit Truck	824
E: Tractor Trailer Unit	2487
ASDT	21730
AADT	17710

East On 16	
Vehicle Type	Vol
A: Passenger Vehicle	23298
B: Recreational Vehicle	438
C: Bus	115
D: Single Unit Truck	1301
E: Tractor Trailer Unit	4088
ASDT	32840
AADT	29250

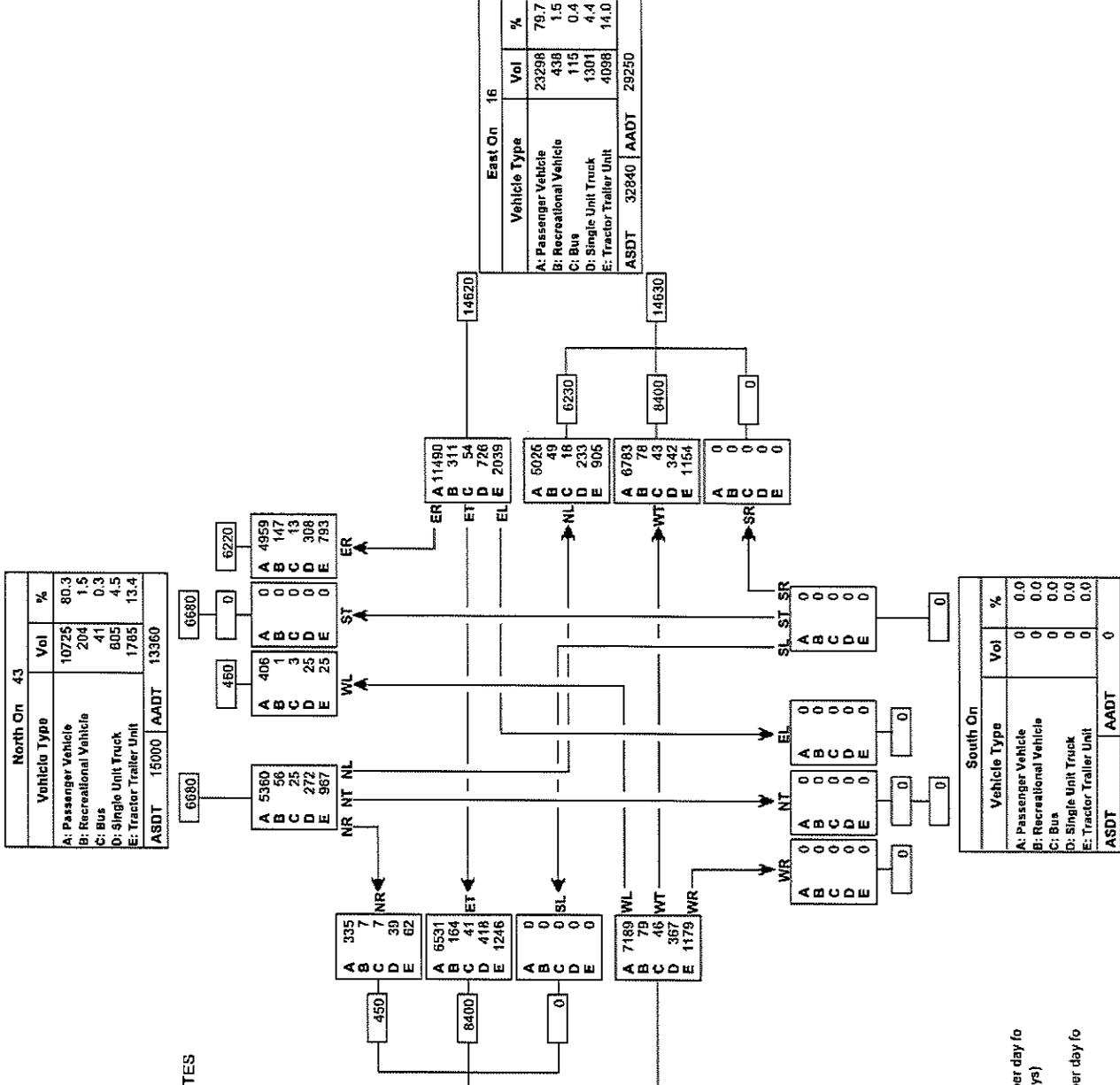
South On	
Vehicle Type	Vol
A: Passenger Vehicle	0
B: Recreational Vehicle	0
C: Bus	0
D: Single Unit Truck	0
E: Tractor Trailer Unit	0
ASDT	0
AADT	0

TURNING MOVEMENT ABBREVIATIONS

- NR: Traffic From North Turning Right
- NL: Traffic From North Turning Left
- NT: Traffic From North Proceeding Through
- SR: Traffic From South Turning Right
- SL: Traffic From South Turning Left
- ST: Traffic From South Proceeding Through
- ER: Traffic From East Turning Right
- EL: Traffic From East Turning Left
- ET: Traffic From East Proceeding Through
- WR: Traffic From West Turning Right
- WL: Traffic From West Turning Left
- WT: Traffic From West Proceeding Through

TURNING MOVEMENT ABBREVIATIONS

- AADT: Average Annual Daily Traffic
 Average daily traffic expressed as vehicles per day for period of January 1 to December 31 (365 days)
- ASDT: Average Summer Daily Traffic
 Average daily traffic expressed as vehicles per day for period of May 1 to September 30 (153 days)



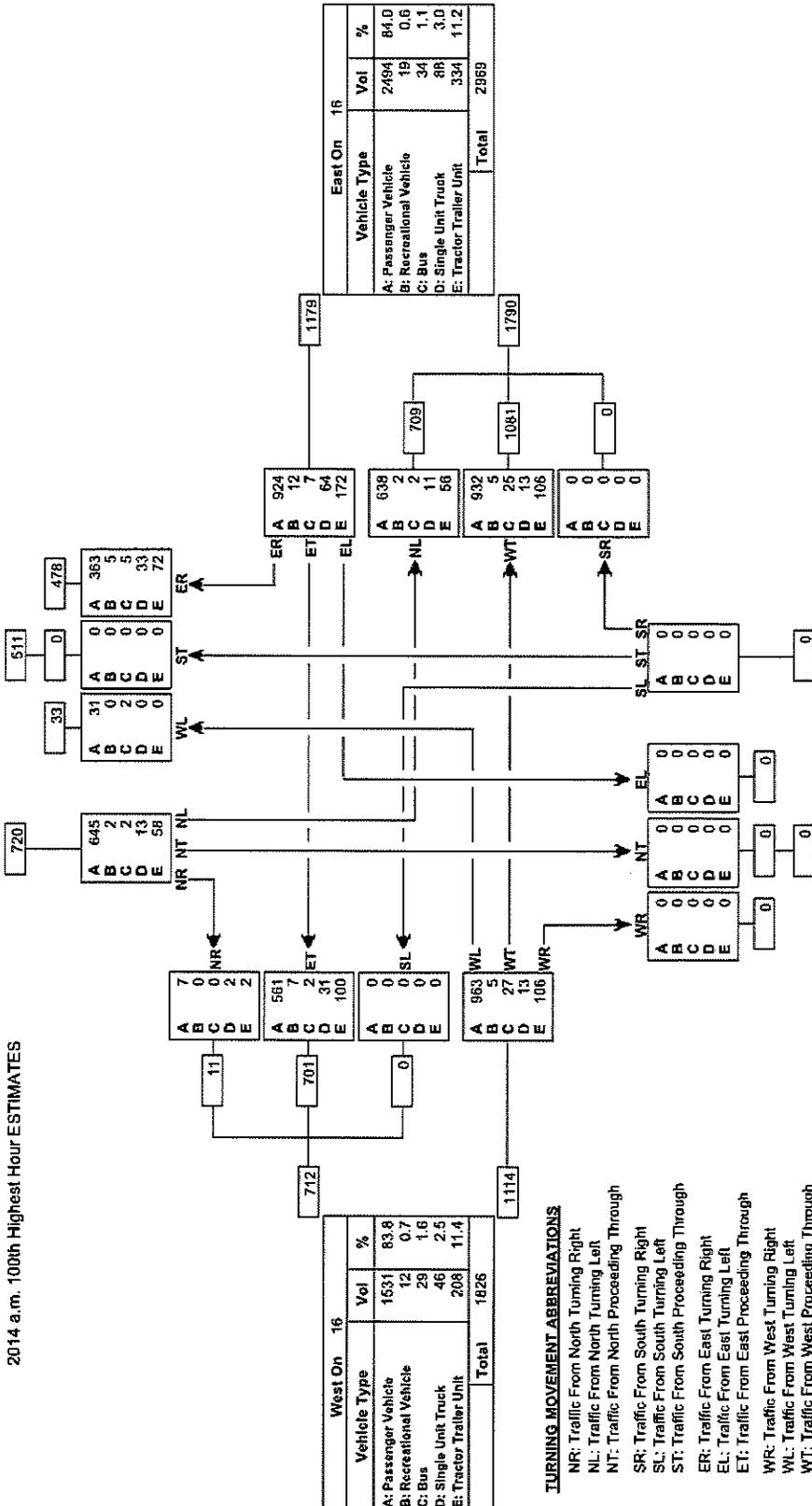
Turning Movement Summary Diagram

North On 43		
Vehicle Type	Vol	%
A: Passenger Vehicle	1039	84.4
B: Recreational Vehicle	7	0.6
C: Bus	9	0.7
D: Single Unit Truck	46	3.7
E: Tractor Trailer Unit	130	10.6
Total	1231	

Reference No.: 72510

Intersection of:
16 & 43 AT MANLY CORNER

2014 a.m. 100th Highest Hour ESTIMATES



West On 16		
Vehicle Type	Vol	%
A: Passenger Vehicle	1531	83.8
B: Recreational Vehicle	12	0.7
C: Bus	29	1.6
D: Single Unit Truck	46	2.5
E: Tractor Trailer Unit	208	11.4
Total	1826	

East On 16		
Vehicle Type	Vol	%
A: Passenger Vehicle	2494	84.0
B: Recreational Vehicle	19	0.6
C: Bus	34	1.1
D: Single Unit Truck	88	3.0
E: Tractor Trailer Unit	334	11.2
Total	2969	

- TURNING MOVEMENT ABBREVIATIONS**
- NR: Traffic From North Turning Right
 - NL: Traffic From North Turning Left
 - NT: Traffic From North Proceeding Through
 - SR: Traffic From South Turning Right
 - SL: Traffic From South Turning Left
 - ST: Traffic From South Proceeding Through
 - ER: Traffic From East Turning Right
 - EL: Traffic From East Turning Left
 - ET: Traffic From East Proceeding Through
 - WR: Traffic From West Turning Right
 - WL: Traffic From West Turning Left
 - WT: Traffic From West Proceeding Through

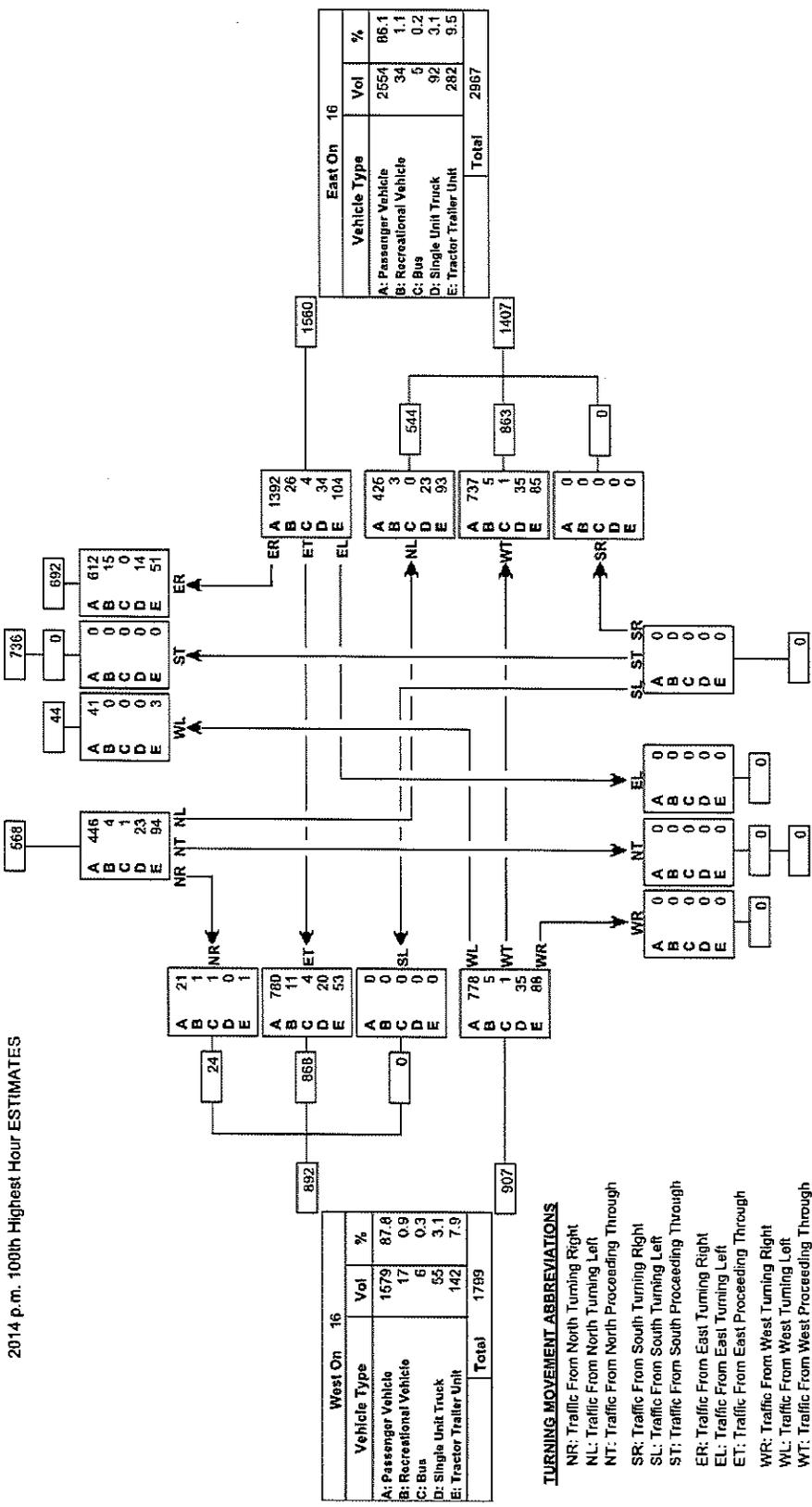
South On		
Vehicle Type	Vol	%
A: Passenger Vehicle	0	0.0
B: Recreational Vehicle	0	0.0
C: Bus	0	0.0
D: Single Unit Truck	0	0.0
E: Tractor Trailer Unit	0	0.0
Total	0	

Turning Movement Summary Diagram

North On 43		
Vehicle Type	Vol	%
A: Passenger Vehicle	1099	84.3
B: Recreational Vehicle	19	1.5
C: Bus	1	0.1
D: Single Unit Truck	37	2.8
E: Tractor Trailer Unit	148	11.3
Total	1304	

Reference No.: 72510
 Intersection of:
 16 & 43 AT MANLY CORNER

2014 p.m. 100th Highest Hour ESTIMATES



TURNING MOVEMENT ABBREVIATIONS

- NR: Traffic From North Turning Right
- NL: Traffic From North Turning Left
- NT: Traffic From North Proceeding Through
- SR: Traffic From South Turning Right
- SL: Traffic From South Turning Left
- ST: Traffic From South Proceeding Through
- ER: Traffic From East Turning Right
- EL: Traffic From East Turning Left
- ET: Traffic From East Proceeding Through
- WR: Traffic From West Turning Right
- WL: Traffic From West Turning Left
- WT: Traffic From West Proceeding Through

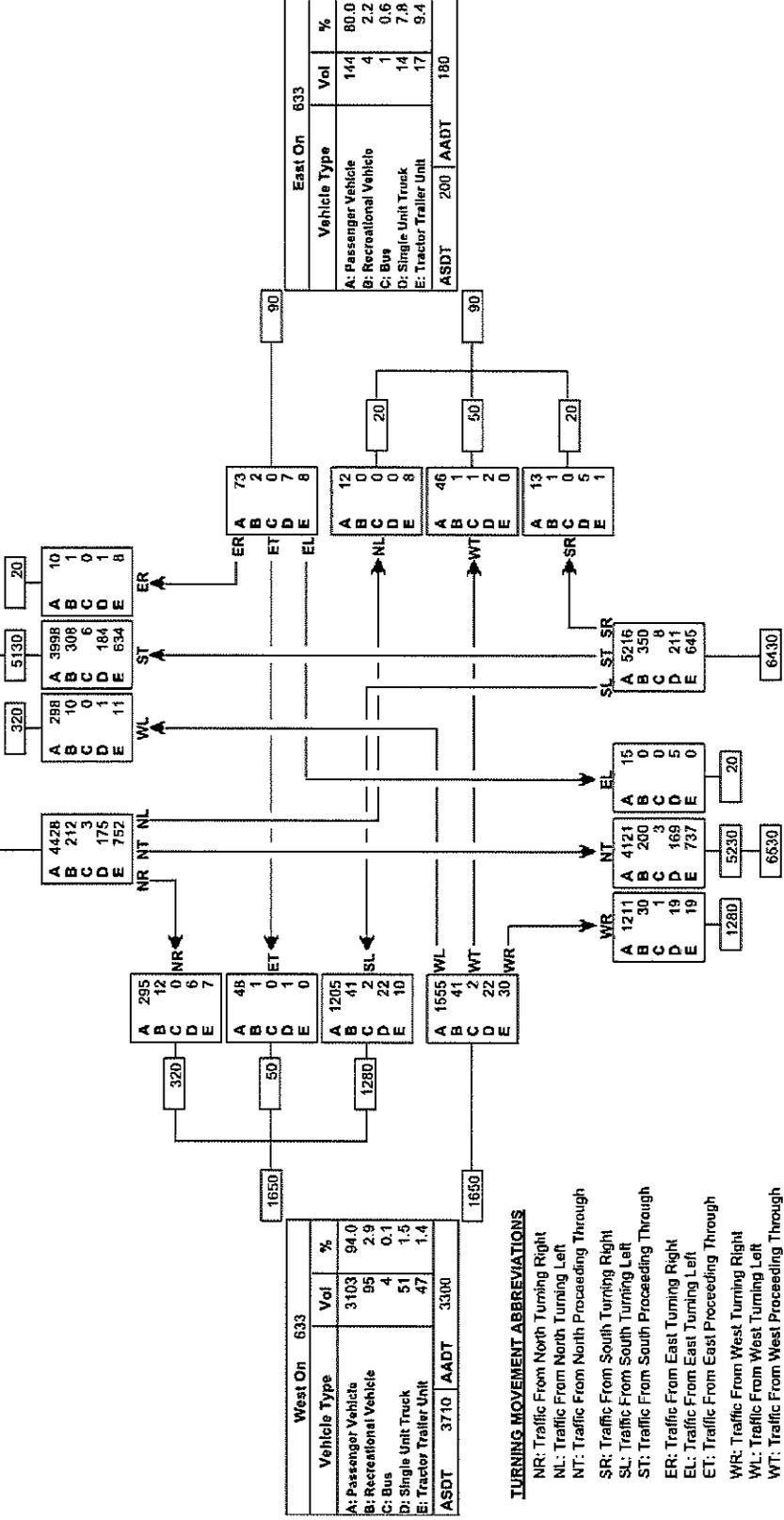
South On		
Vehicle Type	Vol	%
A: Passenger Vehicle	0	0.0
B: Recreational Vehicle	0	0.0
C: Bus	0	0.0
D: Single Unit Truck	0	0.0
E: Tractor Trailer Unit	0	0.0
Total	0	

Turning Movement Summary Diagram

North On 43		
Vehicle Type	Vol	%
A: Passenger Vehicle	8734	79.1
B: Recreational Vehicle	531	4.8
C: Bus	9	0.1
D: Single Unit Truck	361	3.3
E: Tractor Trailer Unit	1405	12.7
ASDT	12400	AAADT 11040

Reference No.: 71630
 Intersection of:
 43 & 633 E OF ALBERTA BEACH

2014 AADT / ASDT ESTIMATES



West On 633		
Vehicle Type	Vol	%
A: Passenger Vehicle	3103	94.0
B: Recreational Vehicle	95	2.9
C: Bus	4	0.1
D: Single Unit Truck	51	1.3
E: Tractor Trailer Unit	47	1.4
ASDT	3710	AAADT 3300

East On 633		
Vehicle Type	Vol	%
A: Passenger Vehicle	144	80.0
B: Recreational Vehicle	4	2.2
C: Bus	1	0.6
D: Single Unit Truck	14	7.8
E: Tractor Trailer Unit	17	9.4
ASDT	200	AAADT 180

South On 43		
Vehicle Type	Vol	%
A: Passenger Vehicle	10563	81.5
B: Recreational Vehicle	580	4.5
C: Bus	12	0.1
D: Single Unit Truck	404	3.1
E: Tractor Trailer Unit	1401	10.8
ASDT	14550	AAADT 12960

TURNING MOVEMENT ABBREVIATIONS

- NR: Traffic From North Turning Right
- NL: Traffic From North Turning Left
- NT: Traffic From North Proceeding Through
- SR: Traffic From South Turning Right
- SL: Traffic From South Turning Left
- ST: Traffic From South Proceeding Through
- ER: Traffic From East Turning Right
- EL: Traffic From East Turning Left
- ET: Traffic From East Proceeding Through
- WR: Traffic From West Turning Right
- WL: Traffic From West Turning Left
- WT: Traffic From West Proceeding Through

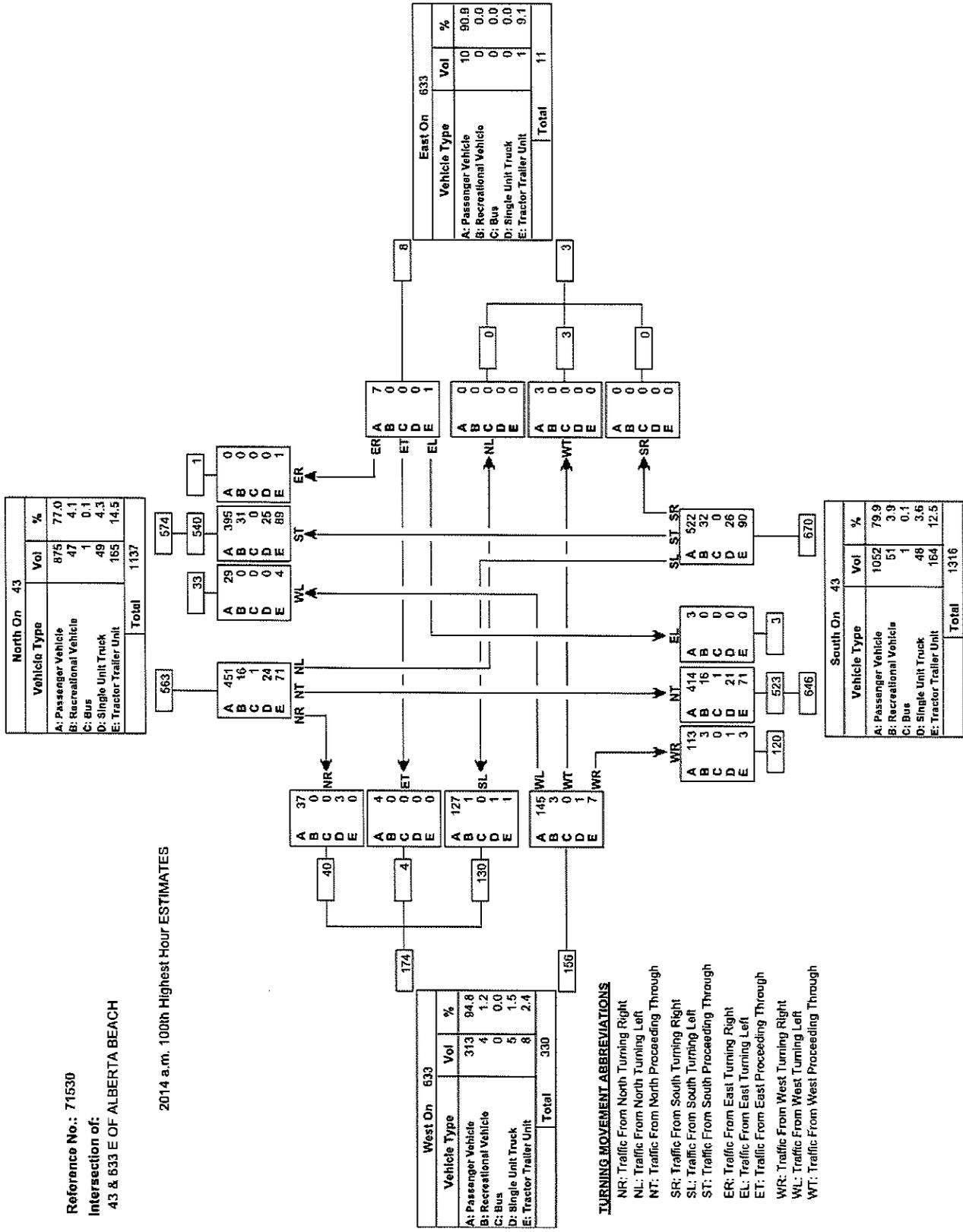
TURNING MOVEMENT ABBREVIATIONS

- AAADT: Average Annual Daily Traffic
- Average daily traffic expressed as vehicles per day for period of January 1 to December 31 (365 days)
- ASDT: Average Summer Daily Traffic
- Average daily traffic expressed as vehicles per day for period of May 1 to September 30 (153 days)

Turning Movement Summary Diagram

Reference No.: 71530
 Intersection of:
 43 & 633 E OF ALBERTA BEACH

2014 a.m. 100th Highest Hour ESTIMATES



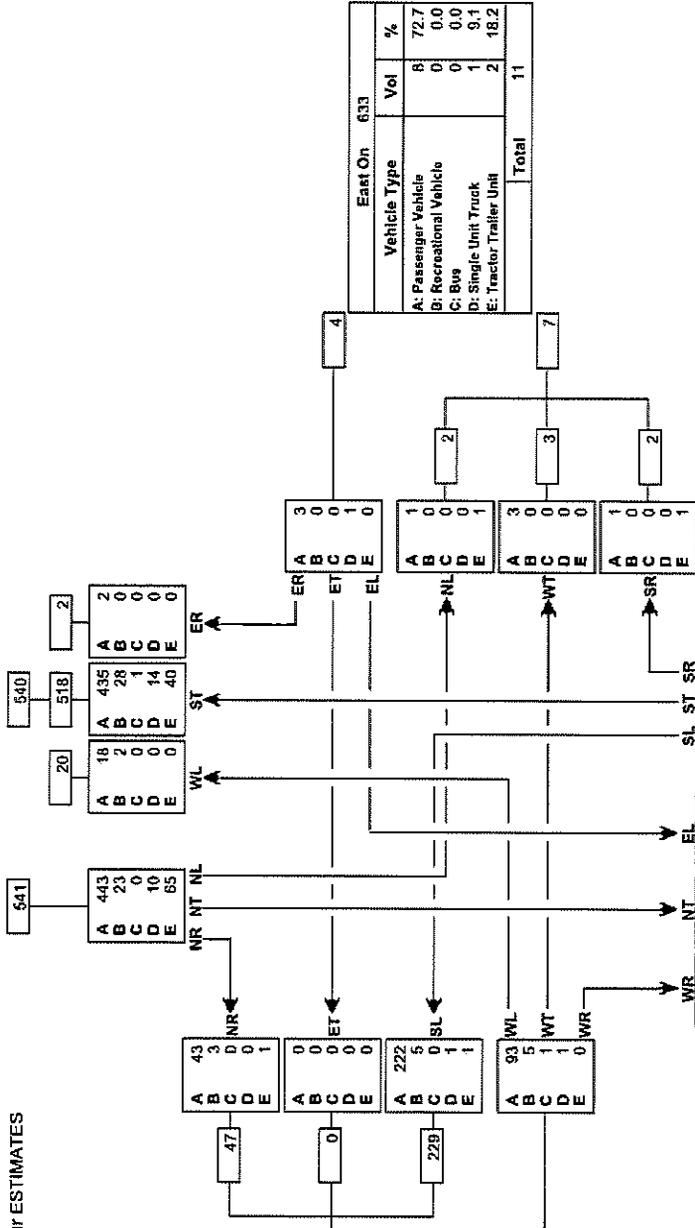
Turning Movement Summary Diagram

Reference No.: 71530

Intersection of:
43 & 633 E OF ALBERTA BEACH

2014 p.m. 100th Highest Hour ESTIMATES

North On 43		
Vehicle Type	Vol	%
A: Passenger Vehicle	898	83.1
B: Recreational Vehicle	53	4.9
C: Bus	1	0.1
D: Single Unit Truck	24	2.2
E: Tractor Trailer Unit	105	9.7
Total	1081	



West On 633		
Vehicle Type	Vol	%
A: Passenger Vehicle	358	95.2
B: Recreational Vehicle	13	3.5
C: Bus	1	0.3
D: Single Unit Truck	2	0.5
E: Tractor Trailer Unit	2	0.5
Total	376	

TURNING MOVEMENT ABBREVIATIONS

- NR: Traffic From North Turning Right
- NL: Traffic From North Turning Left
- NT: Traffic From North Proceeding Through
- SR: Traffic From South Turning Right
- SL: Traffic From South Turning Left
- ST: Traffic From South Proceeding Through
- ER: Traffic From East Turning Right
- EL: Traffic From East Turning Left
- ET: Traffic From East Proceeding Through
- WR: Traffic From West Turning Right
- WL: Traffic From West Turning Left
- WT: Traffic From West Proceeding Through

South On 43		
Vehicle Type	Vol	%
A: Passenger Vehicle	1130	85.6
B: Recreational Vehicle	56	4.2
C: Bus	2	0.2
D: Single Unit Truck	27	2.0
E: Tractor Trailer Unit	105	8.0
Total	1320	

East On 633		
Vehicle Type	Vol	%
A: Passenger Vehicle	8	72.7
B: Recreational Vehicle	0	0.0
C: Bus	0	0.0
D: Single Unit Truck	1	9.1
E: Tractor Trailer Unit	2	18.2
Total	11	

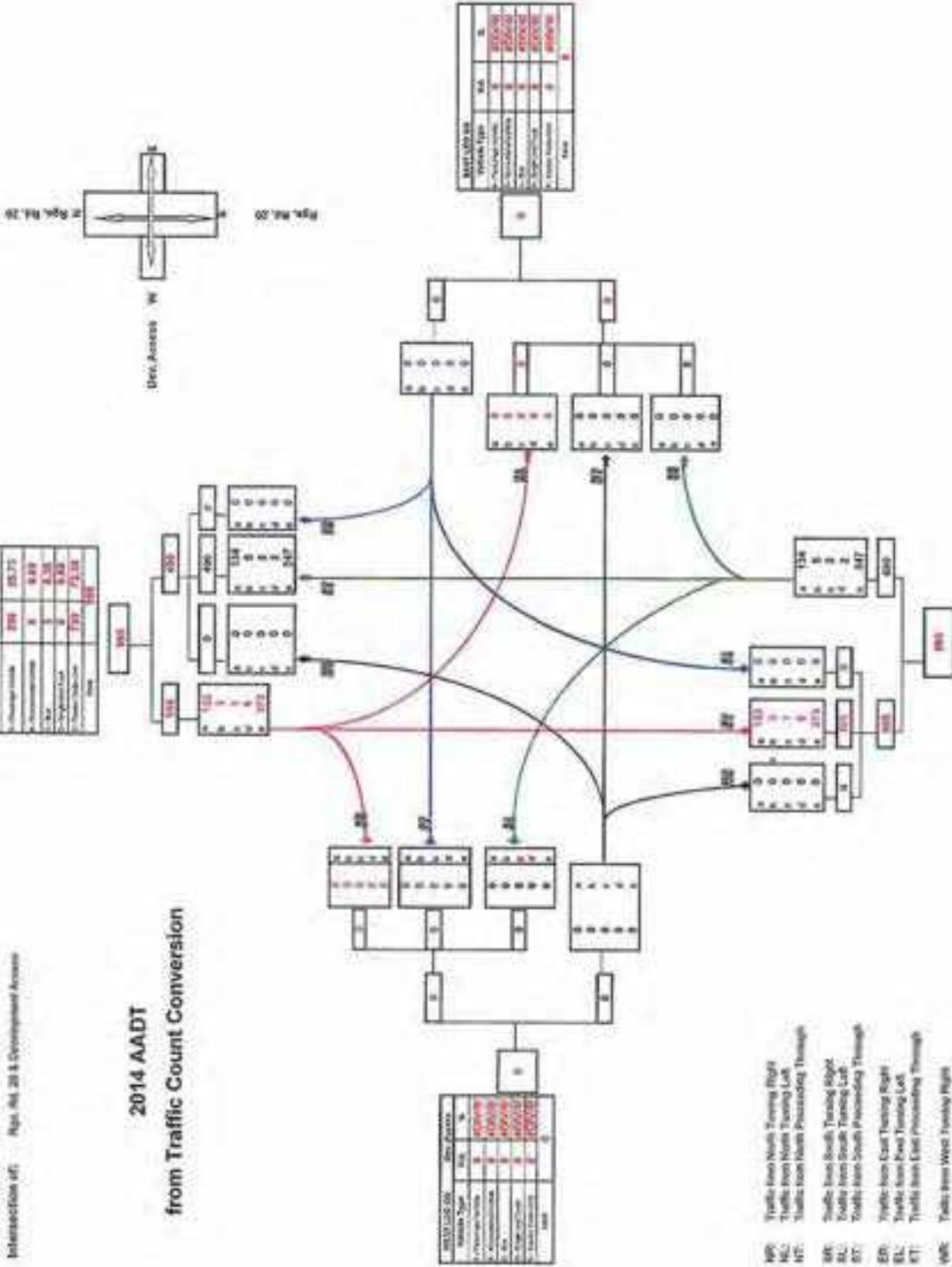
APPENDIX B
Development Access & Rge. Rd. 20

TURNING MOVEMENT DIAGRAMS

Intersections of: Rgn. Rd. 28 & Development Access

Vehicle Type	Vol.	%
Through Traffic	298	25.75
Left Turn	8	0.69
Right Turn	15	1.30
Other	177	15.38
Total	498	42.72

2014 AADT from Traffic Count Conversion



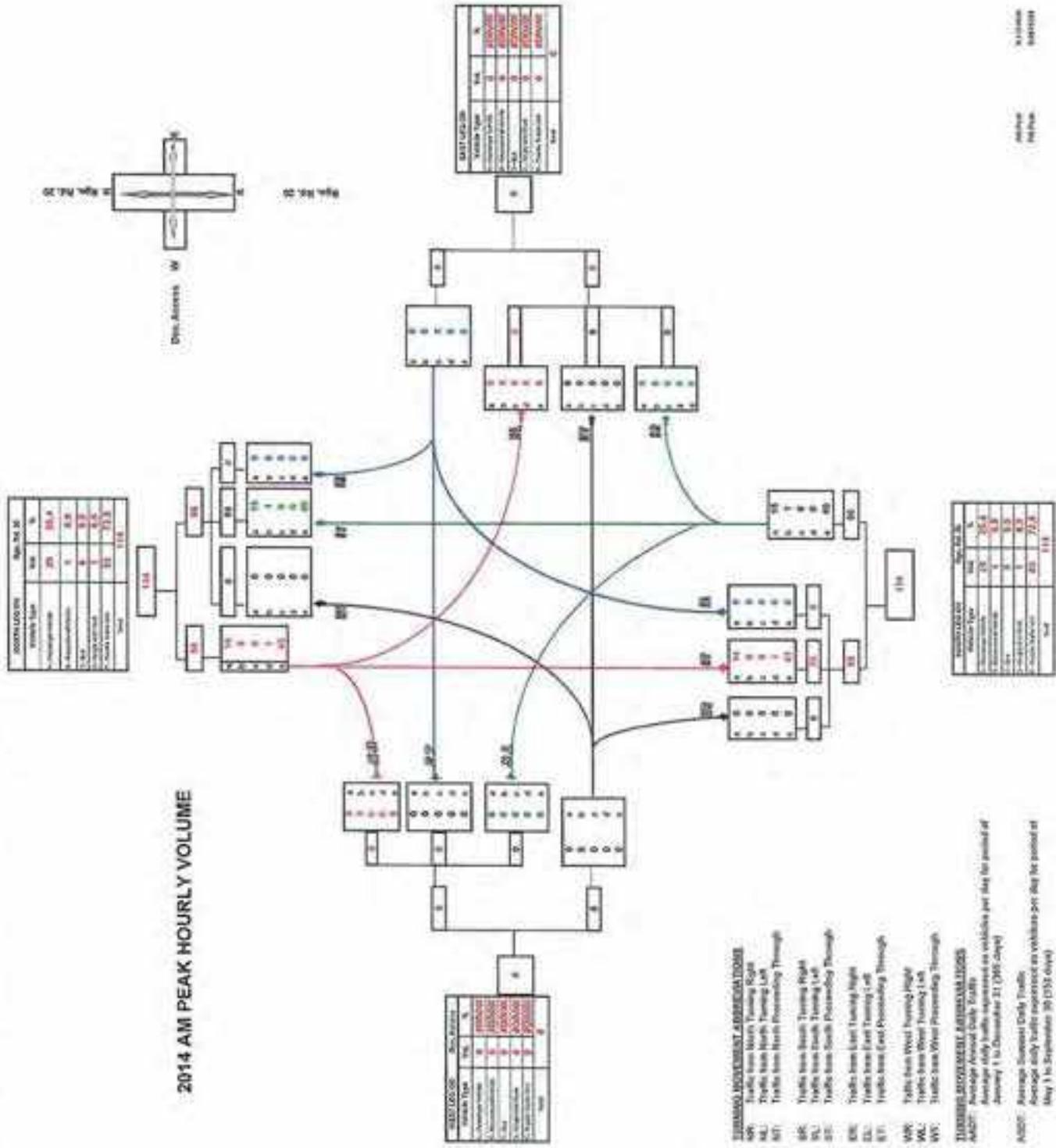
Vehicle Type	Vol.	%
Through Traffic	400	34.38
Left Turn	10	0.85
Right Turn	20	1.70
Other	170	14.25
Total	400	33.73

- NR: Traffic from North Turning Right
- NL: Traffic from North Turning Left
- NT: Traffic from North Proceeding Through
- SR: Traffic from South Turning Right
- SL: Traffic from South Turning Left
- ST: Traffic from South Proceeding Through
- ER: Traffic from East Turning Right
- EL: Traffic from East Turning Left
- ET: Traffic from East Proceeding Through
- WR: Traffic from West Turning Right
- WL: Traffic from West Turning Left
- WT: Traffic from West Proceeding Through

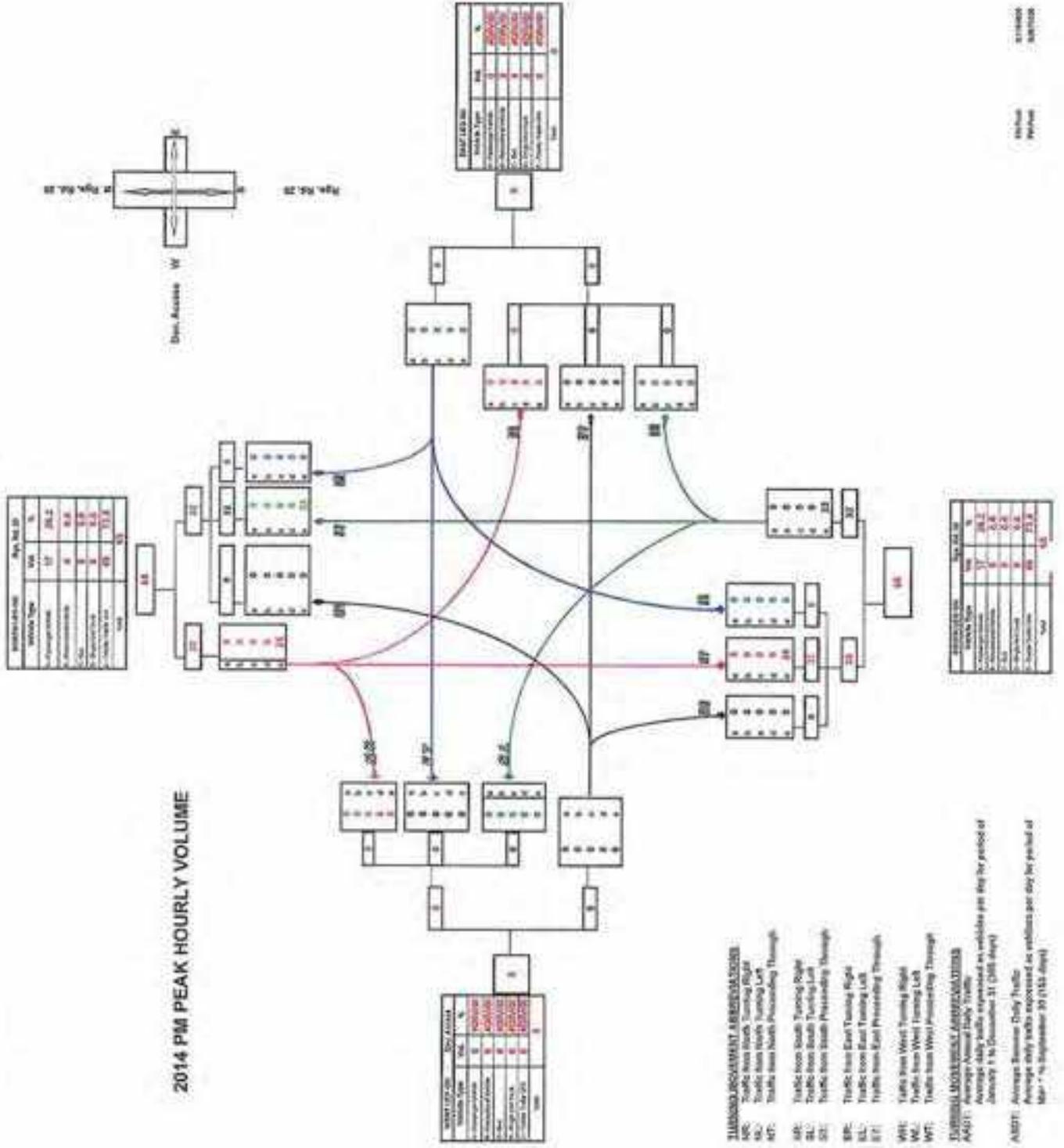
TURNING MOVEMENT ABBREVIATIONS
 AADT: Average Annual Daily Traffic
 Average daily traffic expressed as vehicles per day for period of January 1 to December 31 (365 days)
 ADU: Average Summer Daily Traffic
 Average daily traffic expressed as vehicles per day for period of May 1 to September 30 (153 days)

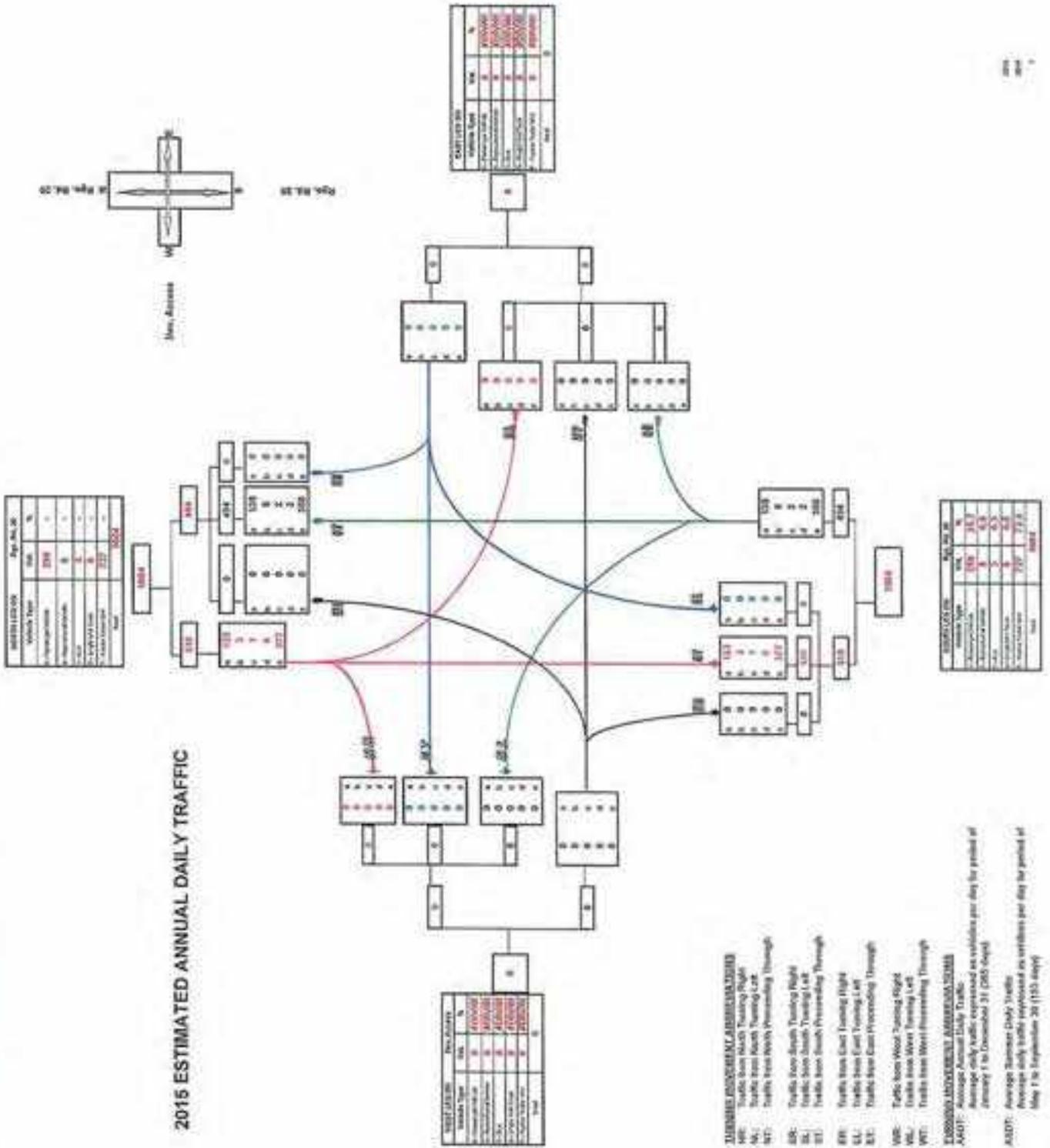
Vehicle Type	Vol.	%
Through Traffic	200	23.71
Left Turn	1	0.12
Right Turn	8	0.94
Other	71	8.23
Total	79	9.20

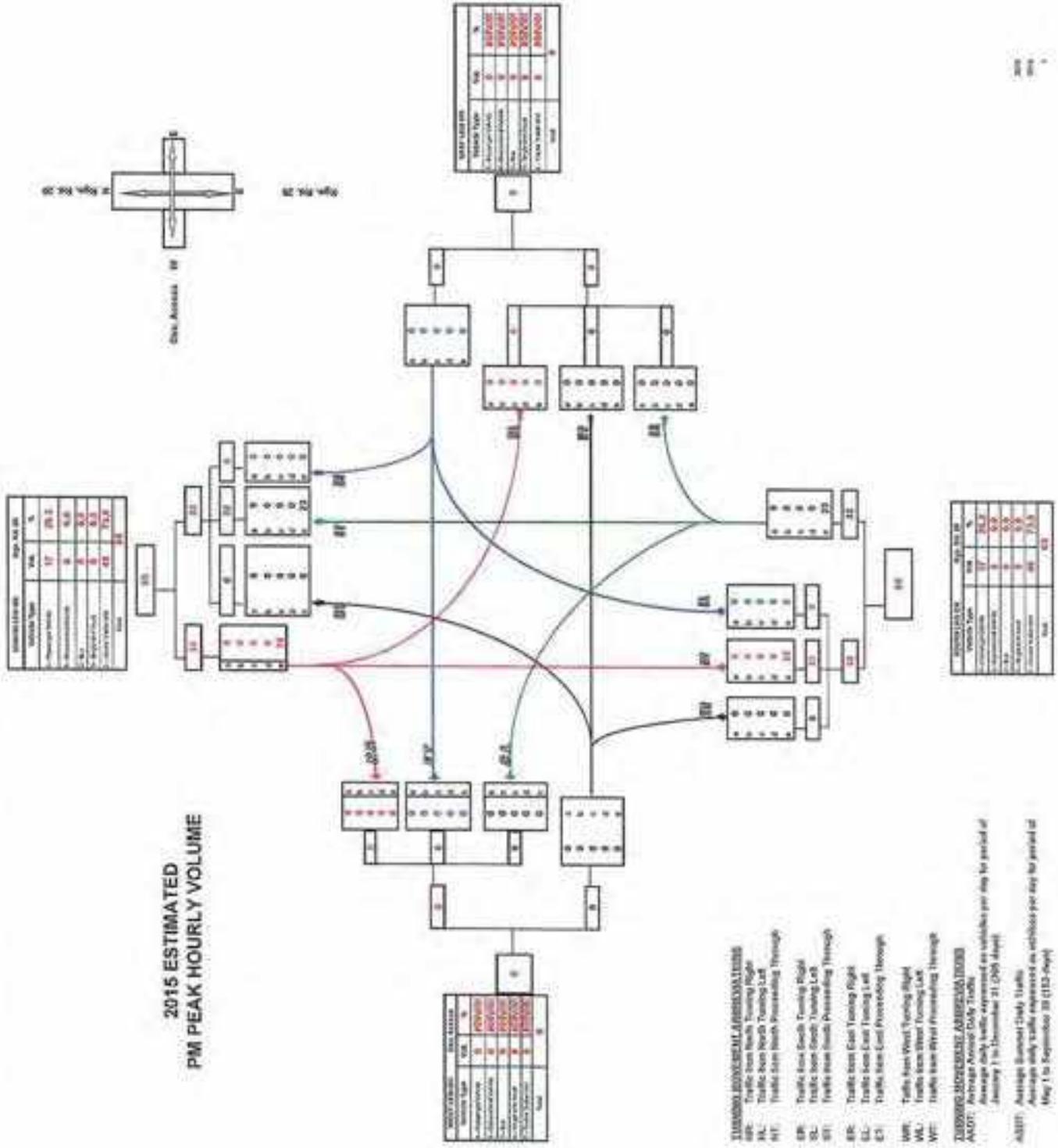
1:1000
 North
 East

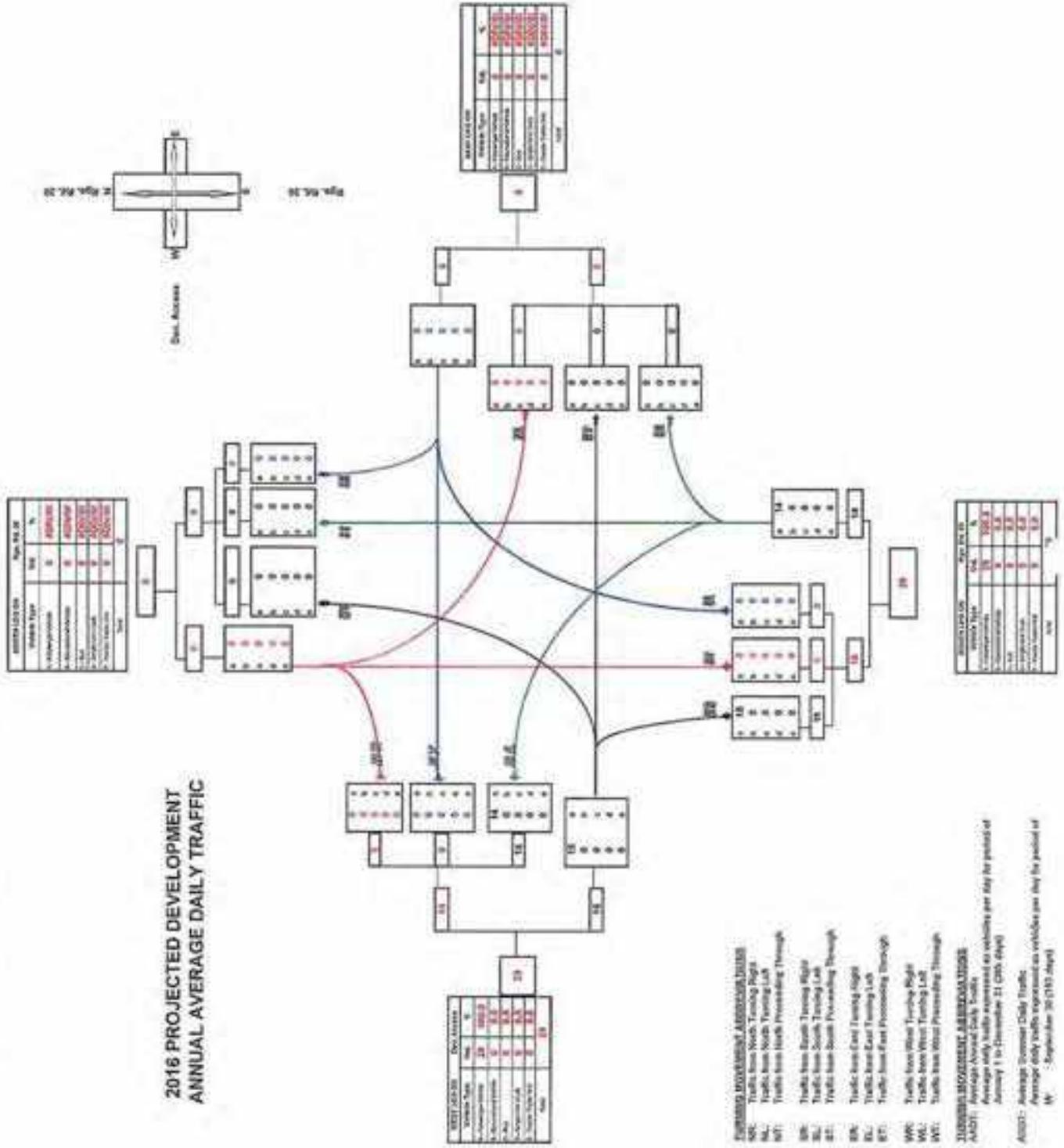


Turning Movement Summary Diagram

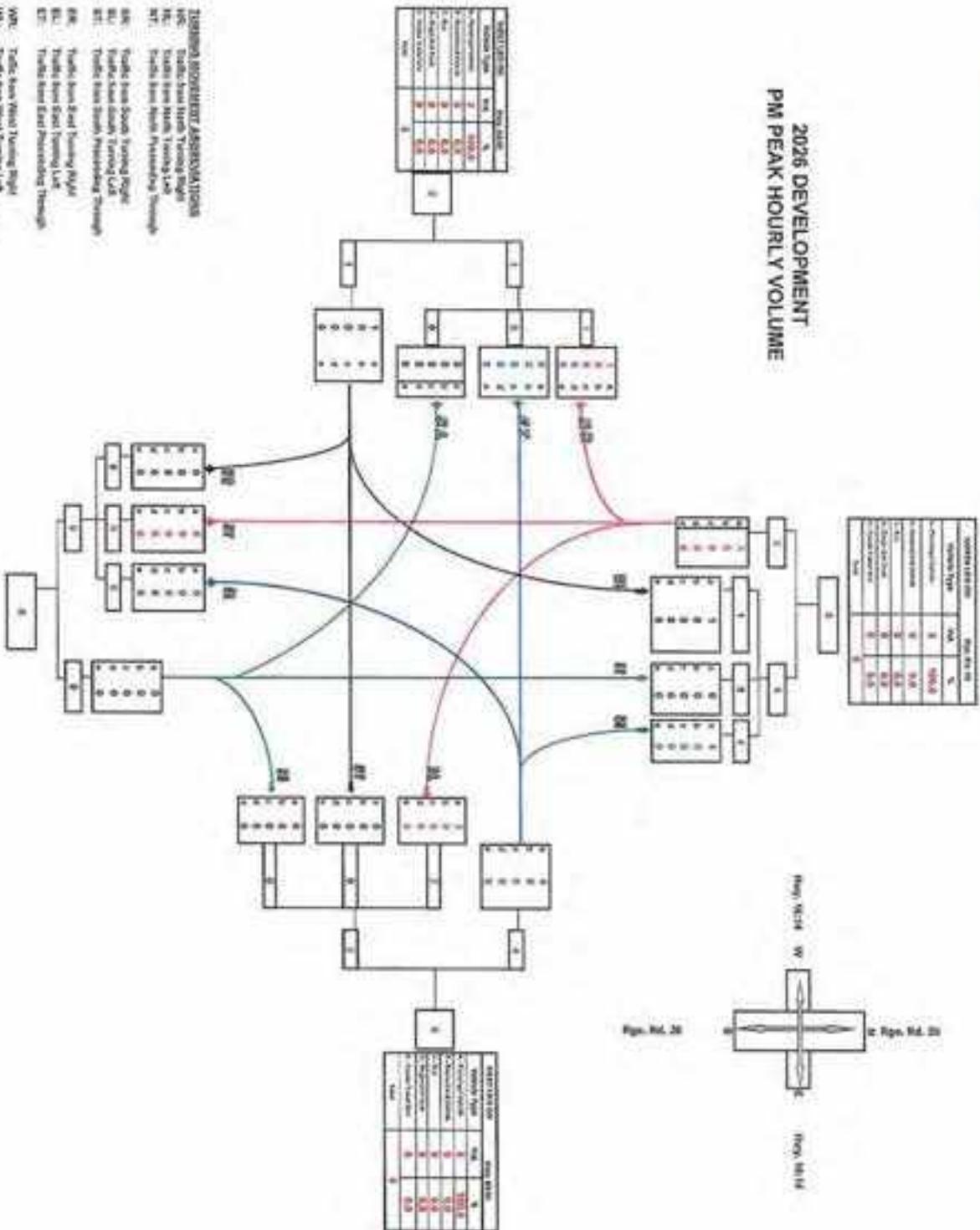








2026 DEVELOPMENT
PM PEAK HOURLY VOLUME



TURNING MOVEMENT ASSUMPTIONS

- 100% Traffic from North Turning Right
- 100% Traffic from South Turning Left
- 100% Traffic from South Proceeding Through
- 100% Traffic from South Turning Right
- 100% Traffic from East Turning Left
- 100% Traffic from East Proceeding Through
- 100% Traffic from West Turning Right
- 100% Traffic from West Turning Left
- 100% Traffic from West Proceeding Through

TURNING MOVEMENT ASSUMPTIONS

- 100% Traffic from North Turning Right
- 100% Traffic from South Turning Left
- 100% Traffic from South Proceeding Through
- 100% Traffic from South Turning Right
- 100% Traffic from East Turning Left
- 100% Traffic from East Proceeding Through
- 100% Traffic from West Turning Right
- 100% Traffic from West Turning Left
- 100% Traffic from West Proceeding Through

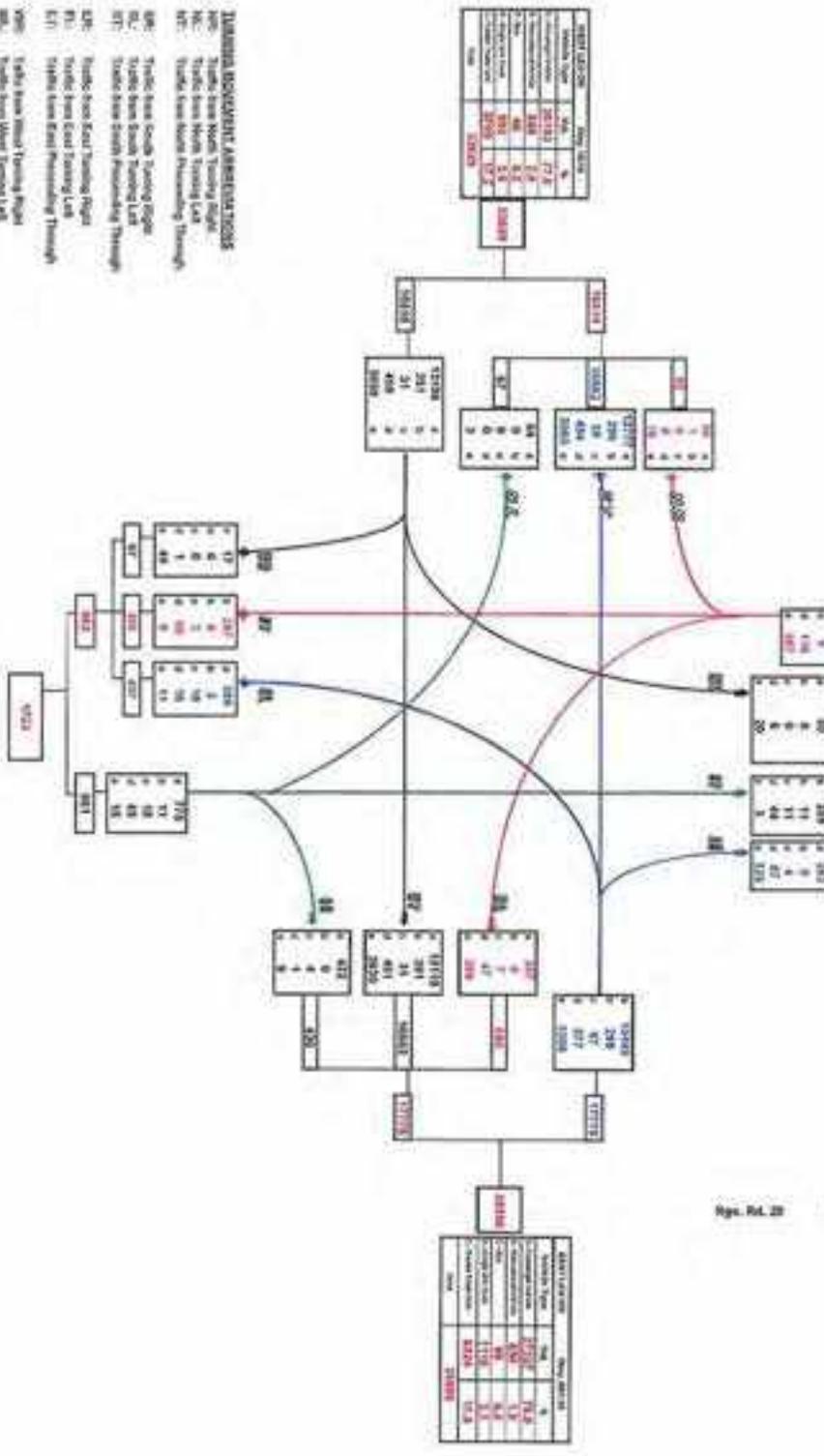
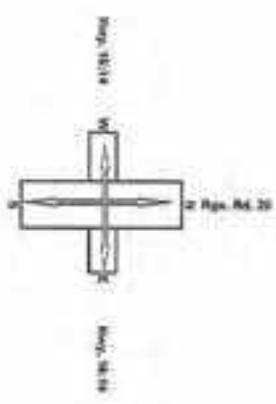
ASSUMPTIONS

- Average Annual Daily Traffic
- Average daily traffic represented as vehicles per day for period of May 1 to September 30 (152 days)

MOVEMENT	PM	AM
THROUGH	100	100
LEFT	50	50
RIGHT	50	50
TOTAL	200	200

2026 COMBINED ANNUAL DAILY TRAFFIC

OPERATION			
Volume Type	Vol	Vol %	%
Through	1272	12.72	12.72
Left Turn	12	0.12	0.12
Right Turn	237	2.37	2.37
Total	1521	15.21	15.21



OPERATION			
Volume Type	Vol	Vol %	%
Through	2013	77.8	77.8
Left Turn	48	2.4	2.4
Right Turn	595	29.8	29.8
Total	2656	100	100

OPERATION			
Volume Type	Vol	Vol %	%
Through	272	17.8	17.8
Left Turn	11	0.7	0.7
Right Turn	1377	81.5	81.5
Total	1520	100	100

- TURNING MOVEMENT ABBREVIATIONS**
- HT: Traffic from North Turning Right
 - HL: Traffic from North Turning Left
 - HT: Traffic from North Forwarding Through
 - HT: Traffic from South Turning Right
 - HL: Traffic from South Turning Left
 - HT: Traffic from South Forwarding Through
 - HT: Traffic from East Turning Right
 - HL: Traffic from East Turning Left
 - HT: Traffic from East Forwarding Through
 - HT: Traffic from West Turning Right
 - HL: Traffic from West Turning Left
 - HT: Traffic from West Forwarding Through

TURNING MOVEMENT ABBREVIATIONS

ADDT: Average Annual Daily Traffic

ADDT: Average daily traffic measured as vehicles per day for period of January 1 to December 31 (2026 ADT)

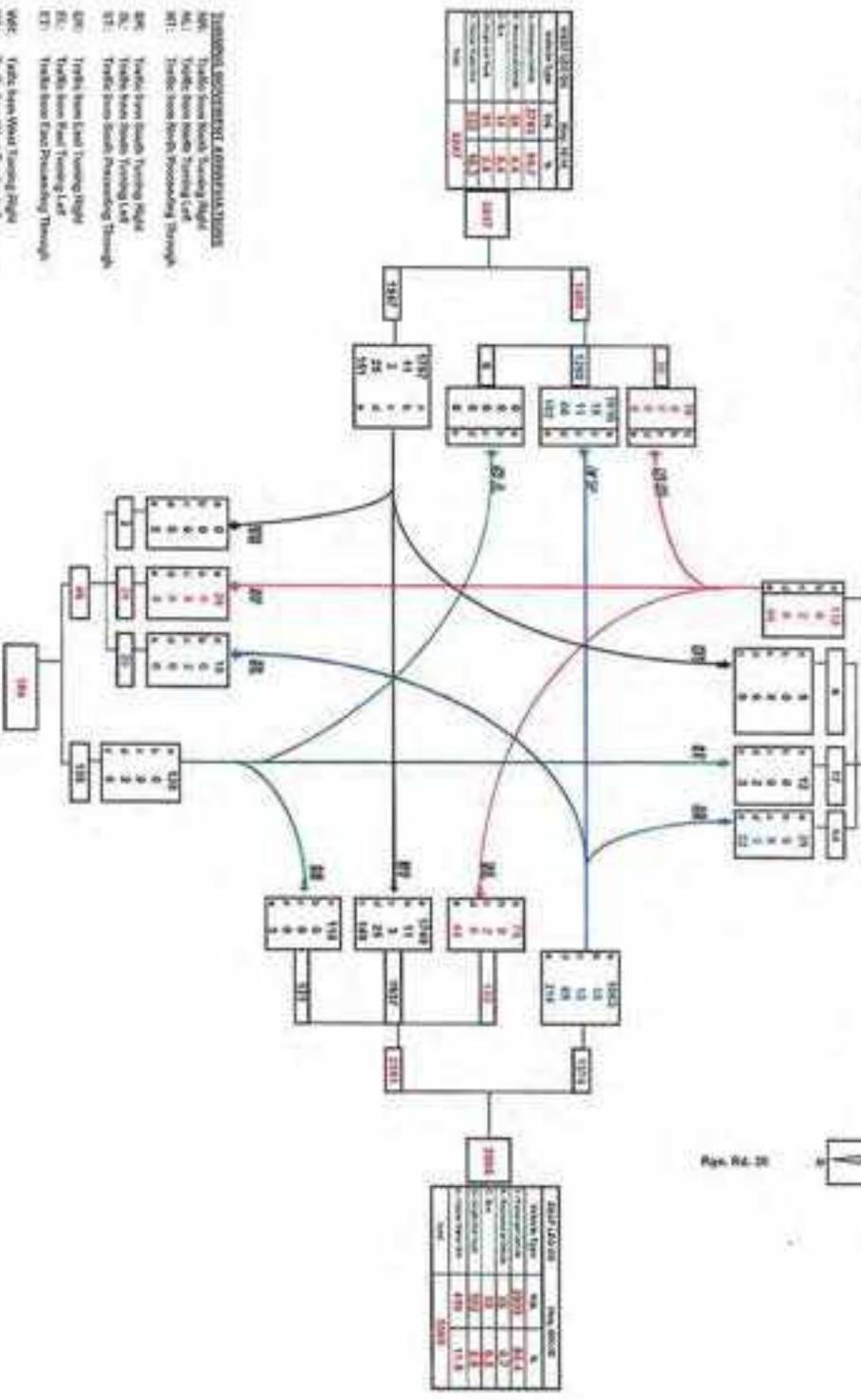
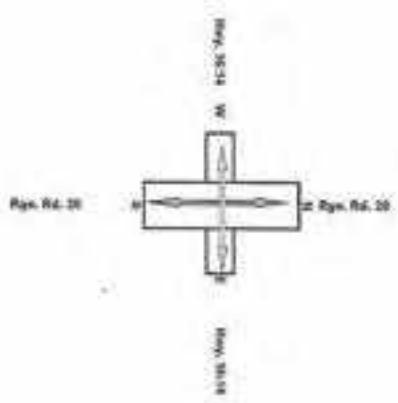
ADDT: Average Through Only Traffic

Average daily traffic reported as vehicles per day for period of May 1 to September 30 (180 days)

OPERATION			
Volume Type	Vol	Vol %	%
Through	1272	12.72	12.72
Left Turn	12	0.12	0.12
Right Turn	237	2.37	2.37
Total	1521	15.21	15.21

**2026 COMBINED
AM PEAK HOURLY VOLUME**

2026 AM PEAK		2026 PM PEAK	
Volume Type	Vol	Vol	%
Through	102	102	100.0
Left	0	0	0.0
Right	0	0	0.0
Left+Right	0	0	0.0
Through+Left+Right	102	102	100.0
Total	102	102	



2026 AM PEAK		2026 PM PEAK	
Volume Type	Vol	Vol	%
Through	102	102	100.0
Left	0	0	0.0
Right	0	0	0.0
Left+Right	0	0	0.0
Through+Left+Right	102	102	100.0
Total	102	102	

2026 PM PEAK		2026 AM PEAK	
Volume Type	Vol	Vol	%
Through	102	102	100.0
Left	0	0	0.0
Right	0	0	0.0
Left+Right	0	0	0.0
Through+Left+Right	102	102	100.0
Total	102	102	

TURNING MOVEMENT ABBREVIATIONS

T: Traffic from South Turning Right
 AL: Traffic from South Turning Left
 RT: Traffic from South Proceeding Through
 TR: Traffic from South Turning Right
 LR: Traffic from South Turning Left
 ST: Traffic from South Proceeding Through
 TR: Traffic from East Turning Right
 LR: Traffic from East Turning Left
 ET: Traffic from East Proceeding Through
 TR: Traffic from West Turning Right
 LR: Traffic from West Turning Left
 WT: Traffic from West Proceeding Through

TURNING MOVEMENT ABBREVIATIONS

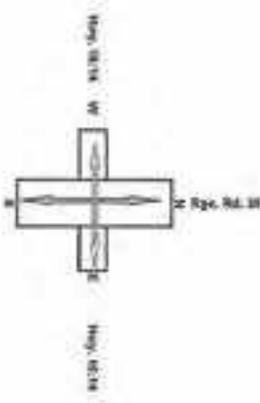
AVOL: Average Annual Daily Volume
 AVOL: Average daily vehicle equivalent to vehicles per day for period of January 1 to December 31 (2026 based)

ADOT: Average Annual Daily Traffic
 ADOT: Average daily traffic equivalent to vehicles per day for period of May 1 to September 30 (2026 based)

2026 AM PEAK		2026 PM PEAK	
Volume Type	Vol	Vol	%
Through	102	102	100.0
Left	0	0	0.0
Right	0	0	0.0
Left+Right	0	0	0.0
Through+Left+Right	102	102	100.0
Total	102	102	

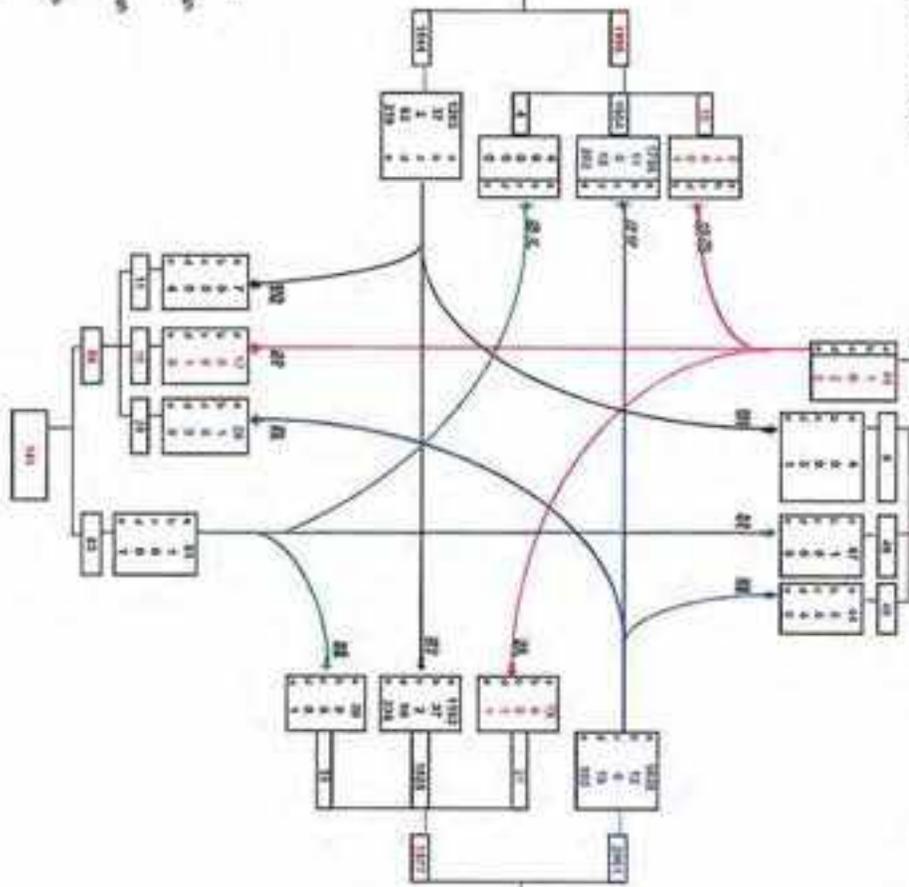
2026 COMBINED PM PEAK HOURLY VOLUME

WEST LEG			
MOVEMENT	Vol	PHF	%
THRU	129	0.62	
LT	2	0.00	
RT	2	0.00	
Left Turn	2	0.00	
Right Turn	2	0.00	
TOTAL	133		



WEST LEG			
MOVEMENT	Vol	PHF	%
THRU	227	0.52	
LT	0	0.00	
RT	7	0.00	
Left Turn	0	0.00	
Right Turn	7	0.00	
TOTAL	234		

EAST LEG			
MOVEMENT	Vol	PHF	%
THRU	204	0.52	
LT	0	0.00	
RT	1	0.00	
Left Turn	0	0.00	
Right Turn	1	0.00	
TOTAL	205		

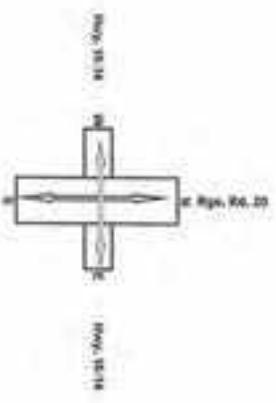


- TURNING MOVEMENT ABBREVIATIONS:**
- TH- Thru from South Turning Right
 - TR- Thru from South Turning Left
 - RT- Thru from South Proceeding Through
 - RL- Thru from South Turning Right
 - TL- Thru from South Turning Left
 - RT- Thru from South Proceeding Through
 - TR- Thru from East Turning Right
 - TL- Thru from East Turning Left
 - RT- Thru from East Proceeding Through
 - RL- Thru from West Turning Right
 - TL- Thru from West Turning Left
 - RT- Thru from West Proceeding Through
- TURNING MOVEMENT ABBREVIATIONS:**
- ADPH- Average Daily Peak Hour
 - ADPHV- Average Daily Peak Hour Volume
 - ADPHVPH- Average Daily Peak Hour Volume per Hour
 - ADPHVPHPH- Average Daily Peak Hour Volume per Hour per Phase
 - ADPHVPHPHPH- Average Daily Peak Hour Volume per Hour per Phase per Lane

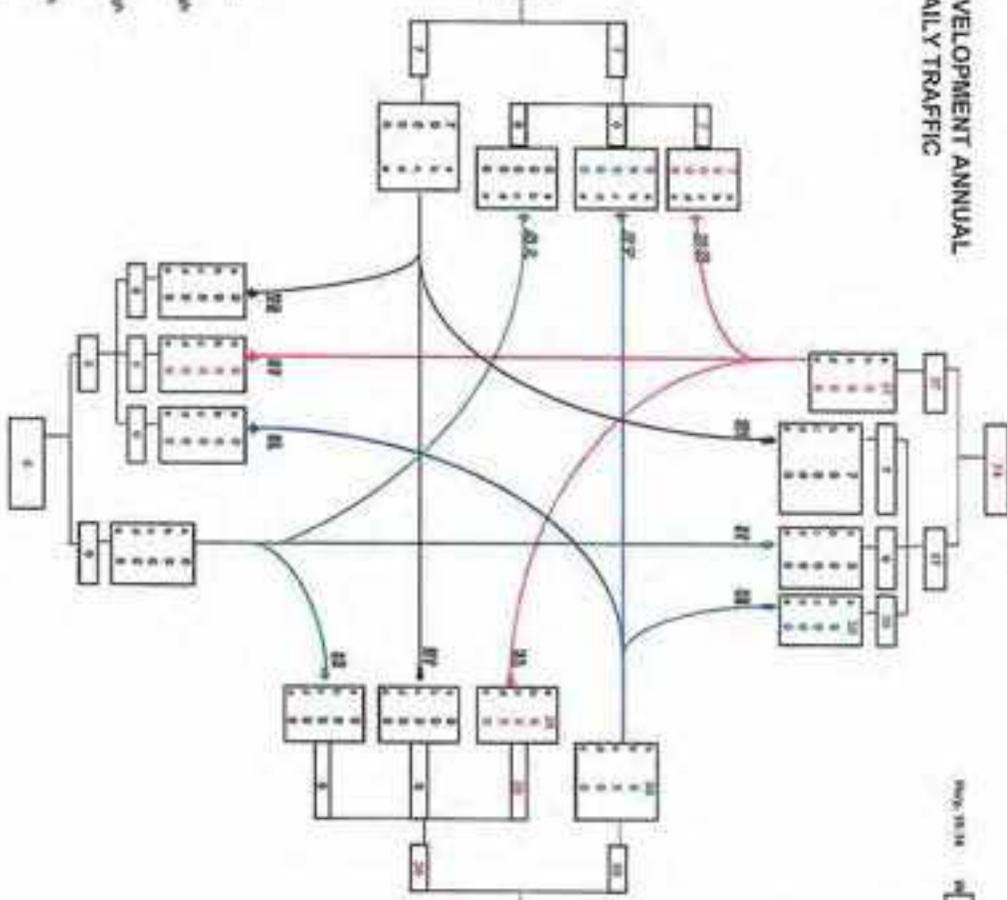
WEST LEG			
MOVEMENT	Vol	PHF	%
THRU	129	0.62	
LT	2	0.00	
RT	2	0.00	
Left Turn	2	0.00	
Right Turn	2	0.00	
TOTAL	133		

**2036 PROJECTED DEVELOPMENT ANNUAL
AVERAGE DAILY TRAFFIC**

SIGNALIZED PHASE - Avg. No. V/s			
Vehicle Type	V/s	%	Total
Through	73	100.0%	73
Left	0	0.0%	0
Right	0	0.0%	0
Left+Right	0	0.0%	0
Total	73		73



UNCONTROLLED PHASE - Avg. No. V/s			
Vehicle Type	V/s	%	Total
Through	14	100.0%	14
Left	0	0.0%	0
Right	0	0.0%	0
Left+Right	0	0.0%	0
Total	14		14



SIGNALIZED PHASE - Avg. No. V/s			
Vehicle Type	V/s	%	Total
Through	20	100.0%	20
Left	0	0.0%	0
Right	0	0.0%	0
Left+Right	0	0.0%	0
Total	20		20

- TURNING MOVEMENT ABBREVIATIONS**
- TR: Traffic from South Turning Right
 - TL: Traffic from South Turning Left
 - TR: Traffic from North Turning Right
 - TL: Traffic from North Turning Left
 - TR: Traffic from East Turning Right
 - TL: Traffic from East Turning Left
 - TR: Traffic from West Turning Right
 - TL: Traffic from West Turning Left

ADDITIONAL INFORMATION

Average Annual Daily Traffic
 Average Daily Traffic, reported as vehicles per day for period of January 1 to December 31 (2035 Study)

Average Annual Daily Traffic
 Average Daily Traffic, reported as vehicles per day for period of May 1 to September 30 (195 Study)

SIGNALIZED PHASE - Avg. No. V/s			
Vehicle Type	V/s	%	Total
Through	14	100.0%	14
Left	0	0.0%	0
Right	0	0.0%	0
Left+Right	0	0.0%	0
Total	14		14

2036 DEVELOPMENT
AM PEAK HOURLY VOLUME

WEST LEG			
MOVEMENT	Vol	PHV	%
THROUGH	9	100.0	100.0
THROUGH-LEFT	0	0.0	0.0
THROUGH-RIGHT	0	0.0	0.0
LEFT-TURN	0	0.0	0.0
RIGHT-TURN	0	0.0	0.0
TOTAL	9	100.0	100.0

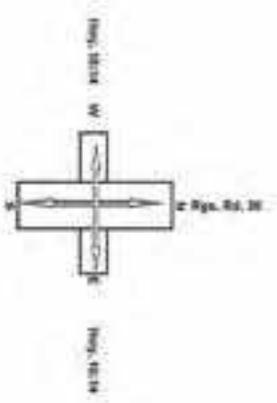
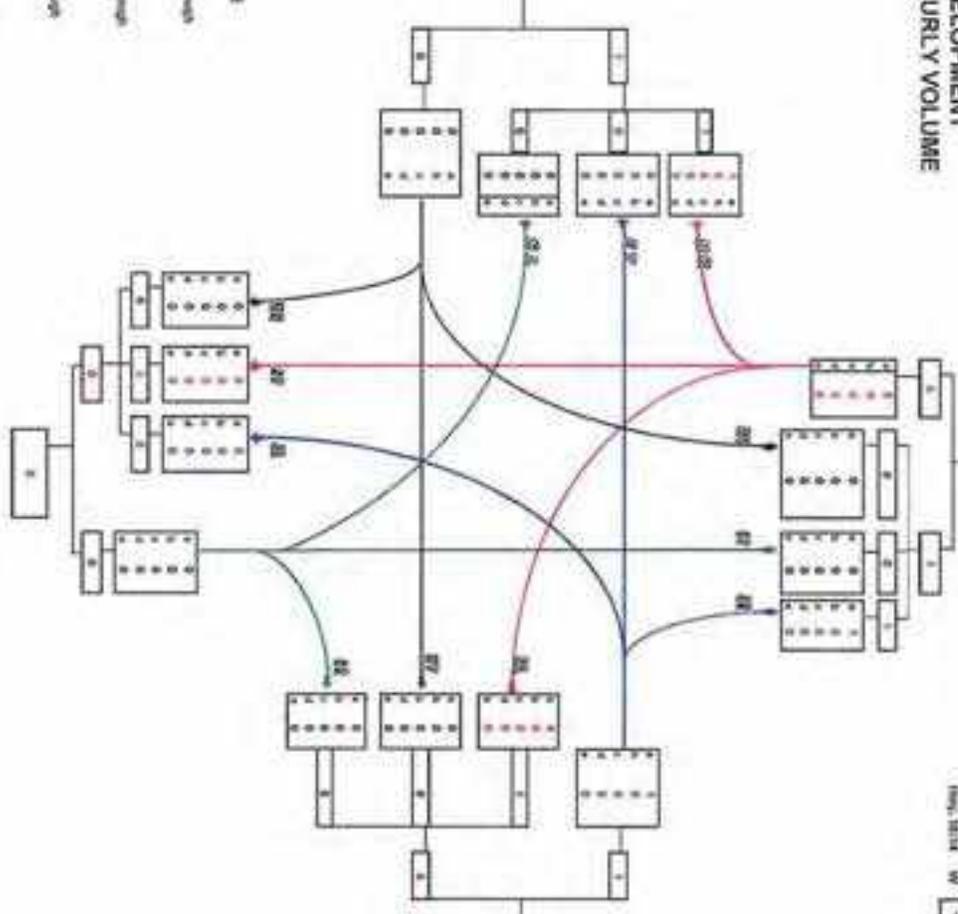


Fig. 04.20

EAST LEG			
MOVEMENT	Vol	PHV	%
THROUGH	7	100.0	100.0
THROUGH-LEFT	0	0.0	0.0
THROUGH-RIGHT	0	0.0	0.0
LEFT-TURN	0	0.0	0.0
RIGHT-TURN	0	0.0	0.0
TOTAL	7	100.0	100.0

WEST LEG			
MOVEMENT	Vol	PHV	%
THROUGH	7	100.0	100.0
THROUGH-LEFT	0	0.0	0.0
THROUGH-RIGHT	0	0.0	0.0
LEFT-TURN	0	0.0	0.0
RIGHT-TURN	0	0.0	0.0
TOTAL	7	100.0	100.0



- TURNING MOVEMENT ABBREVIATIONS**
- TR: Through
 - TL: Through-Left
 - TR: Through-Right
 - LT: Left-Turn
 - RT: Right-Turn
 - TH: Through
 - THL: Through-Left
 - THR: Through-Right
 - LT: Left-Turn
 - RT: Right-Turn
 - TH: Through
 - THL: Through-Left
 - THR: Through-Right
 - LT: Left-Turn
 - RT: Right-Turn

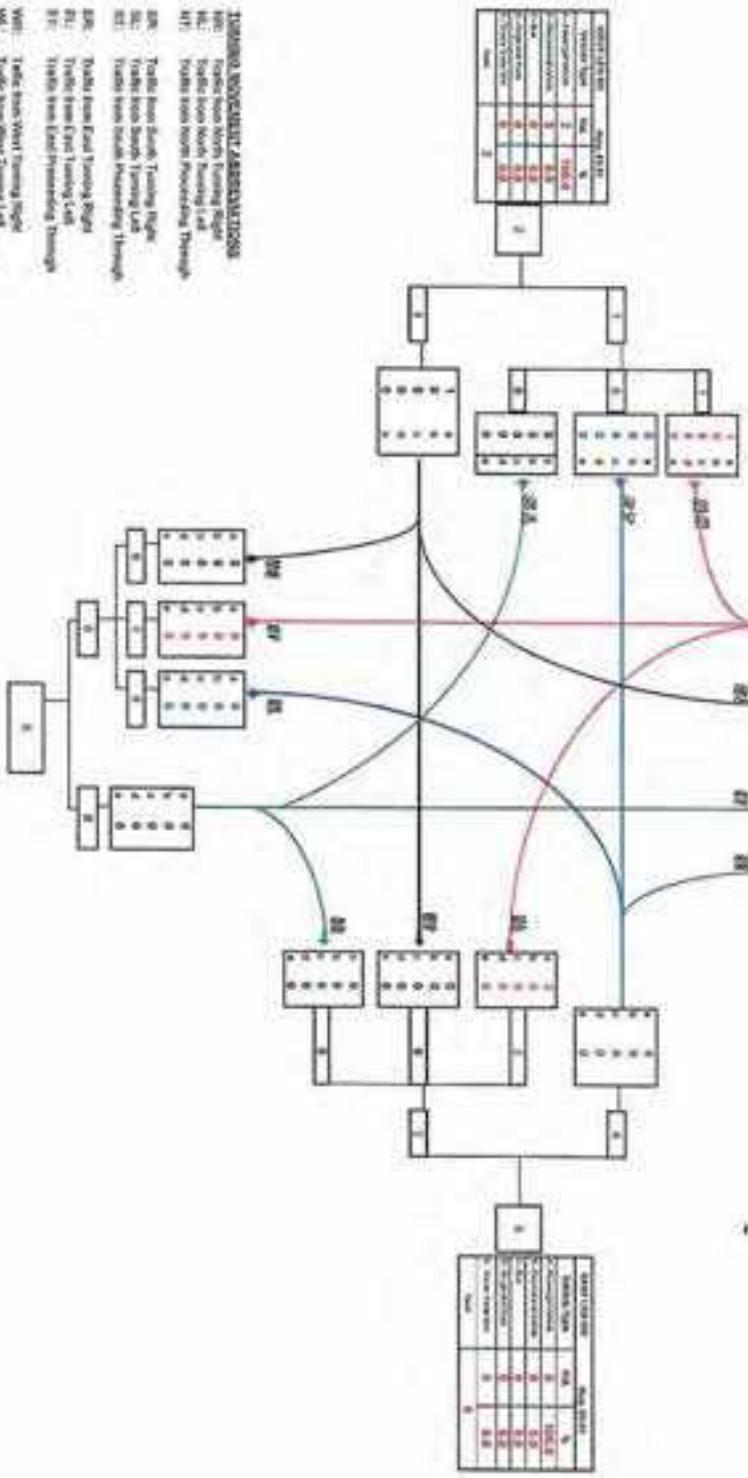
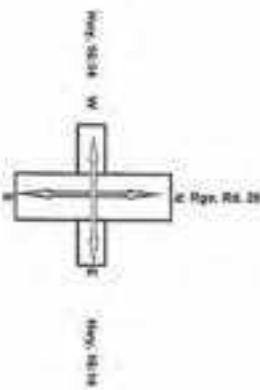
TURNING MOVEMENT ABBREVIATIONS

- TR: Through
- TL: Through-Left
- TR: Through-Right
- LT: Left-Turn
- RT: Right-Turn
- TH: Through
- THL: Through-Left
- THR: Through-Right
- LT: Left-Turn
- RT: Right-Turn
- TH: Through
- THL: Through-Left
- THR: Through-Right
- LT: Left-Turn
- RT: Right-Turn

WEST LEG			
MOVEMENT	Vol	PHV	%
THROUGH	9	100.0	100.0
THROUGH-LEFT	0	0.0	0.0
THROUGH-RIGHT	0	0.0	0.0
LEFT-TURN	0	0.0	0.0
RIGHT-TURN	0	0.0	0.0
TOTAL	9	100.0	100.0

**2036 DEVELOPMENT
PM PEAK HOURLY VOLUME**

DIRECTION		PM PEAK	
TRAFFIC TYPE	VAL.	%	VAL.
Approach	8	100%	8
Departure	8	100%	8
Through	8	100%	8
Left	8	100%	8
Right	8	100%	8
U-TURN	8	100%	8
TOTAL	8		8



DIRECTION		PM PEAK	
TRAFFIC TYPE	VAL.	%	VAL.
Approach	1	100%	1
Departure	1	100%	1
Through	1	100%	1
Left	1	100%	1
Right	1	100%	1
TOTAL	1		1

DIRECTION		PM PEAK	
TRAFFIC TYPE	VAL.	%	VAL.
Approach	8	100%	8
Departure	8	100%	8
Through	8	100%	8
Left	8	100%	8
Right	8	100%	8
TOTAL	8		8

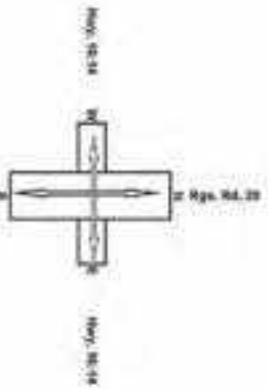
- TURNING MOVEMENT ABBREVIATIONS**
- TL: Traffic from North Turning Right
 - TR: Traffic from North Turning Left
 - MT: Traffic from North Proceeding Through
 - RL: Traffic from South Turning Right
 - RR: Traffic from South Turning Left
 - MR: Traffic from South Proceeding Through
 - SL: Traffic from East Turning Right
 - SR: Traffic from East Turning Left
 - ML: Traffic from East Proceeding Through
 - WL: Traffic from West Turning Right
 - WR: Traffic from West Turning Left
 - ML: Traffic from West Proceeding Through
- TURNING MOVEMENT ABBREVIATIONS**
- ADCT: Average Annual Daily Traffic
 - ADCT: Average daily traffic, estimated in vehicles per day for period of January 1 to October 31 (261 days)
 - ADCT: Average Annual Daily Traffic
 - ADCT: Average daily traffic, estimated in vehicles per day for period of May 1 to September 30 (151 days)

DIRECTION		PM PEAK	
TRAFFIC TYPE	VAL.	%	VAL.
Approach	8	100%	8
Departure	8	100%	8
Through	8	100%	8
Left	8	100%	8
Right	8	100%	8
TOTAL	8		8

Turning Movement Summary Diagram

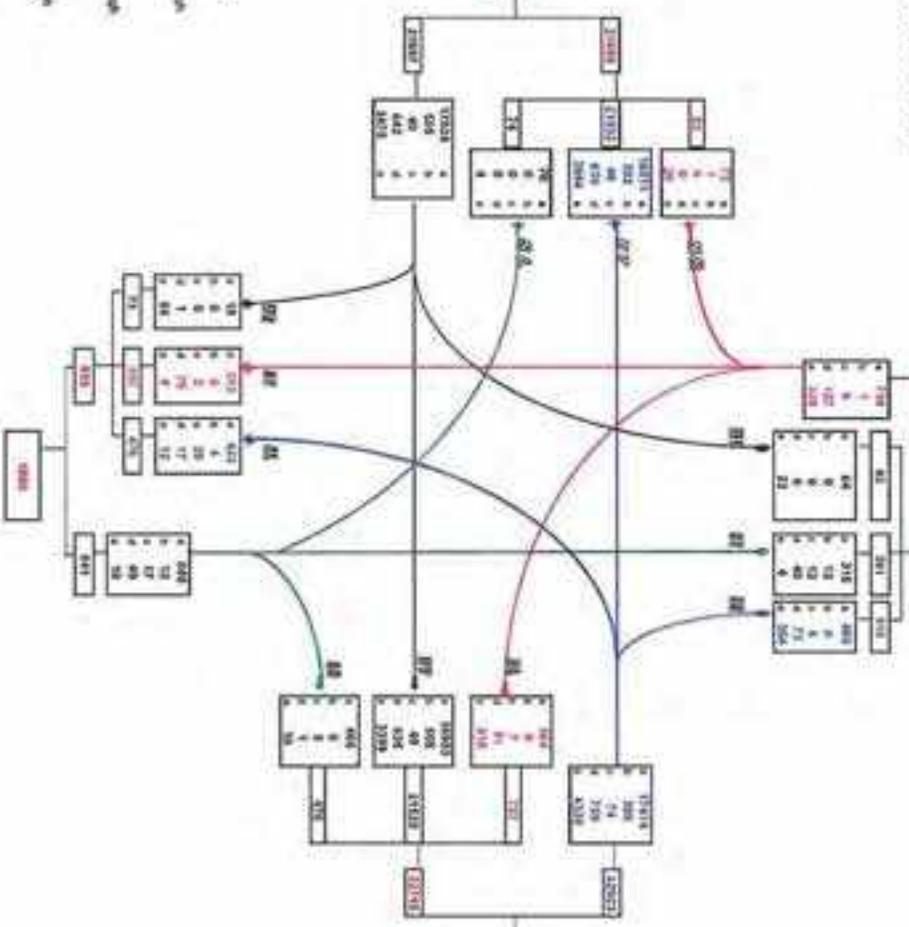
2026 COMBINED ANNUAL DAILY TRAFFIC

OPERATIONAL			
Vehicle Type	Vol.	% of T	N
Light Duty	1047	1.5	-
Medium Duty	77	0.1	-
Heavy Duty	133	0.2	-
TOTAL	1257	1.8	-



WEST SIDE			
Vehicle Type	Vol.	% of T	N
Light Duty	11488	27.2	-
Medium Duty	88	0.2	-
Heavy Duty	75	0.2	-
TOTAL	12451	17.6	-

EAST SIDE			
Vehicle Type	Vol.	% of T	N
Light Duty	10111	23.8	-
Medium Duty	63	0.1	-
Heavy Duty	128	0.3	-
TOTAL	10302	14.1	-



- TURNING MOVEMENT ABBREVIATIONS:**
- LR: Traffic from North Turning Right
 - LL: Traffic from North Turning Left
 - LR-: Traffic from North Proceeding Through
 - RL: Traffic from South Turning Right
 - LL: Traffic from South Turning Left
 - RL-: Traffic from South Proceeding Through
 - LR-: Traffic from East Turning Right
 - LL-: Traffic from East Turning Left
 - RL-: Traffic from East Proceeding Through
 - WR: Traffic from West Turning Right
 - WL: Traffic from West Turning Left
 - WR-: Traffic from West Proceeding Through

TURNING MOVEMENT ABBREVIATIONS:
ADDT: Average Annual Daily Traffic
ADDT: Average daily traffic converted as vehicles per day for posted at Agency 1 on December 31, 2026 ADPT
ADDT: Average Season Daily Traffic
ADDT: Average daily traffic measured at vehicles per day for posted at Day 1 to September 26, 2026 ADPT

WEST SIDE			
Vehicle Type	Vol.	% of T	N
Light Duty	11488	27.2	-
Medium Duty	88	0.2	-
Heavy Duty	75	0.2	-
TOTAL	12451	17.6	-

2036 COMBINED AM PEAK HOURLY VOLUME

MOVEMENT	Vol	Vol. %
Northbound	117	63.8
Southbound	6	4.0
Eastbound	7	4.0
Westbound	11	6.2
Left Turn	10	5.7
Right Turn	27	15.3
Total	183	

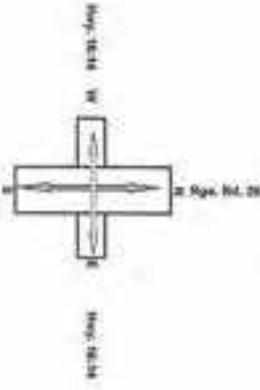
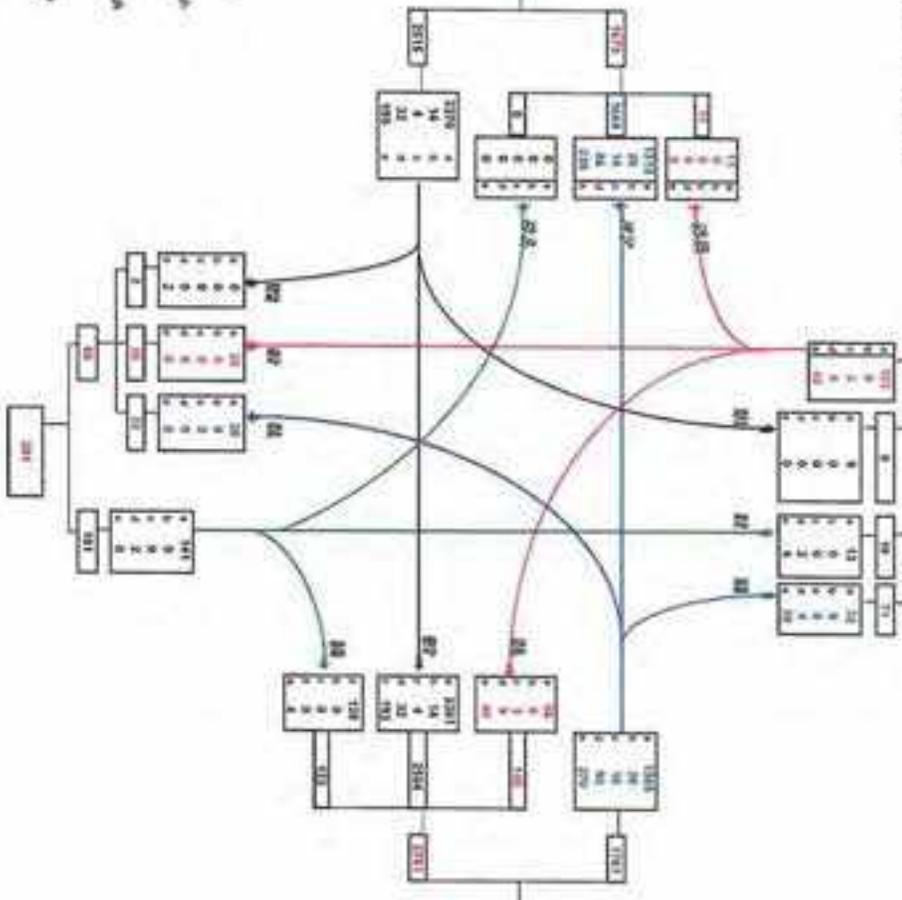


Fig. 94.20

MOVEMENT	Vol	Vol. %
Northbound	104	58.7
Southbound	5	3.0
Eastbound	6	3.3
Westbound	10	5.5
Left Turn	10	5.5
Right Turn	28	15.3
Total	173	



MOVEMENT	Vol	Vol. %
Northbound	130	73.3
Southbound	4	2.3
Eastbound	5	2.8
Westbound	10	5.6
Left Turn	10	5.6
Right Turn	24	13.6
Total	178	

- TURNING MOVEMENT ABBREVIATIONS**
- NR - Traffic from North Turning Right
 - NR - Traffic from North Turning Left
 - NR - Traffic from North Proceeding Through
 - SR - Traffic from South Turning Right
 - SR - Traffic from South Turning Left
 - SR - Traffic from South Proceeding Through
 - ER - Traffic from East Turning Right
 - ER - Traffic from East Turning Left
 - ER - Traffic from East Proceeding Through
 - WR - Traffic from West Turning Right
 - WR - Traffic from West Turning Left
 - WR - Traffic from West Proceeding Through

TURNING MOVEMENT ABBREVIATIONS

AAOT - Average Annual Daily Traffic
 AAOT - Average Daily Traffic Expressed as a Multiple per Day for period of
 January 1 to December 31 (2000 days)

ADOT - Average Annual Daily Traffic
 ADOT - Average Daily Traffic Expressed as a Multiple per Day for period of
 May 1 to September 30 (153 days)

MOVEMENT	Vol	Vol. %
Northbound	117	63.8
Southbound	6	4.0
Eastbound	7	4.0
Westbound	11	6.2
Left Turn	10	5.7
Right Turn	27	15.3
Total	183	

2036 COMBINED PM PEAK HOURLY VOLUME

SIGNALIZATION		PM PEAK HOUR	
Signal Type	%	Volume	ADT
Through	100	150	600
Left	0	0	0
Right	0	0	0
Total		150	600

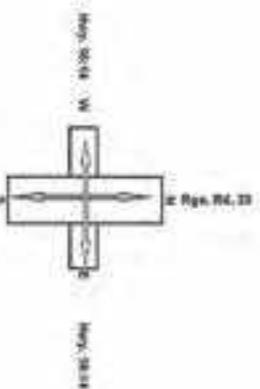
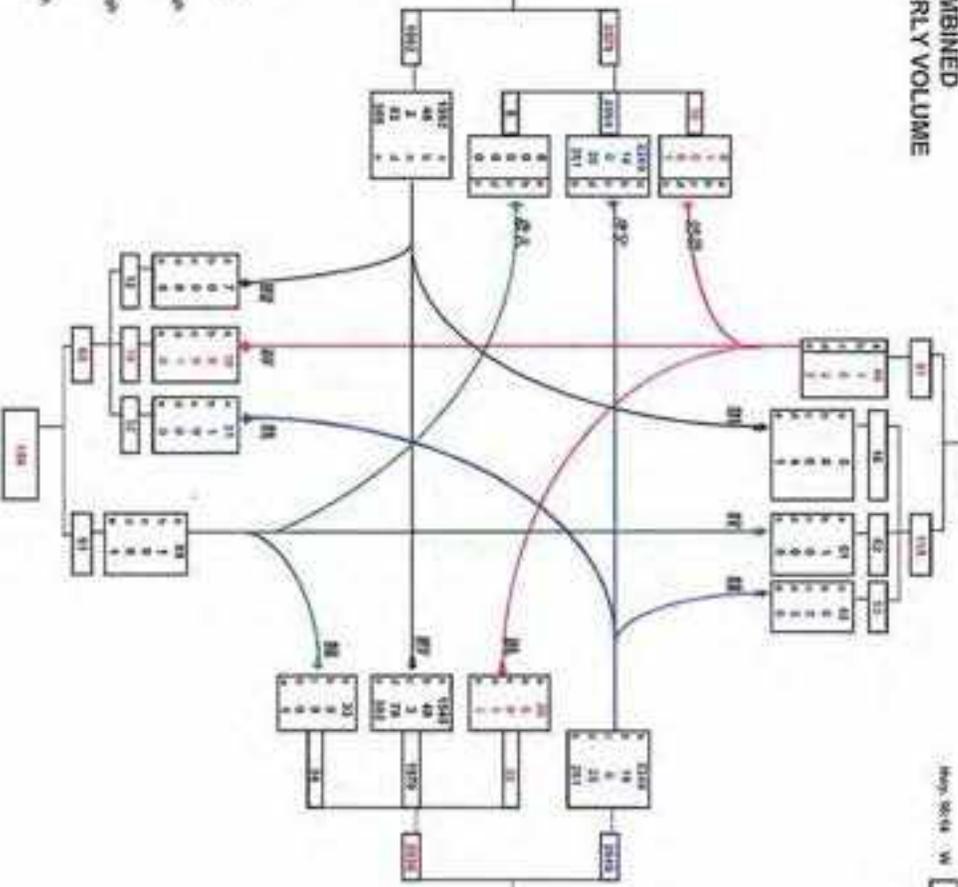


Fig. No. 20

SIGNALIZATION		PM PEAK HOUR	
Signal Type	%	Volume	ADT
Through	100	150	600
Left	0	0	0
Right	0	0	0
Total		150	600



SIGNALIZATION		PM PEAK HOUR	
Signal Type	%	Volume	ADT
Through	100	150	600
Left	0	0	0
Right	0	0	0
Total		150	600

TURNING MOVEMENT APPROXIMATIONS

- SR: Trucks from South Turning Right
- SL: Trucks from South Turning Left
- SR: Trucks from South Proceeding Through
- SR: Trucks from South Turning Right
- SL: Trucks from South Turning Left
- SR: Trucks from South Proceeding Through
- SR: Trucks from East Turning Right
- SL: Trucks from East Turning Left
- SR: Trucks from East Proceeding Through
- SR: Trucks from West Turning Right
- SL: Trucks from West Turning Left
- SR: Trucks from West Proceeding Through

TURNING MOVEMENT APPROXIMATIONS

- SR: Average Annual Daily Trucks
- SL: Average Right volume expected as vehicles per day for period of January 1 to December 31 (90% ADT)
- ADT: Average Annual Daily Trucks
- ADT: Average daily truck expected as vehicles per day for period of May 1 to September 30 (50% ADT)

SIGNALIZATION		PM PEAK HOUR	
Signal Type	%	Volume	ADT
Through	100	150	600
Left	0	0	0
Right	0	0	0
Total		150	600

2026 COMBINED AM PEAK HOURLY VOLUME

Direction	Volume	%
Northbound	41	20.4
Southbound	1	0.5
Eastbound	4	2.0
Westbound	1	0.5
Total	47	23.4

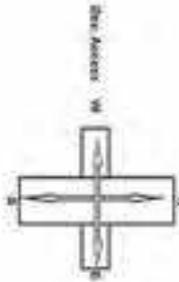
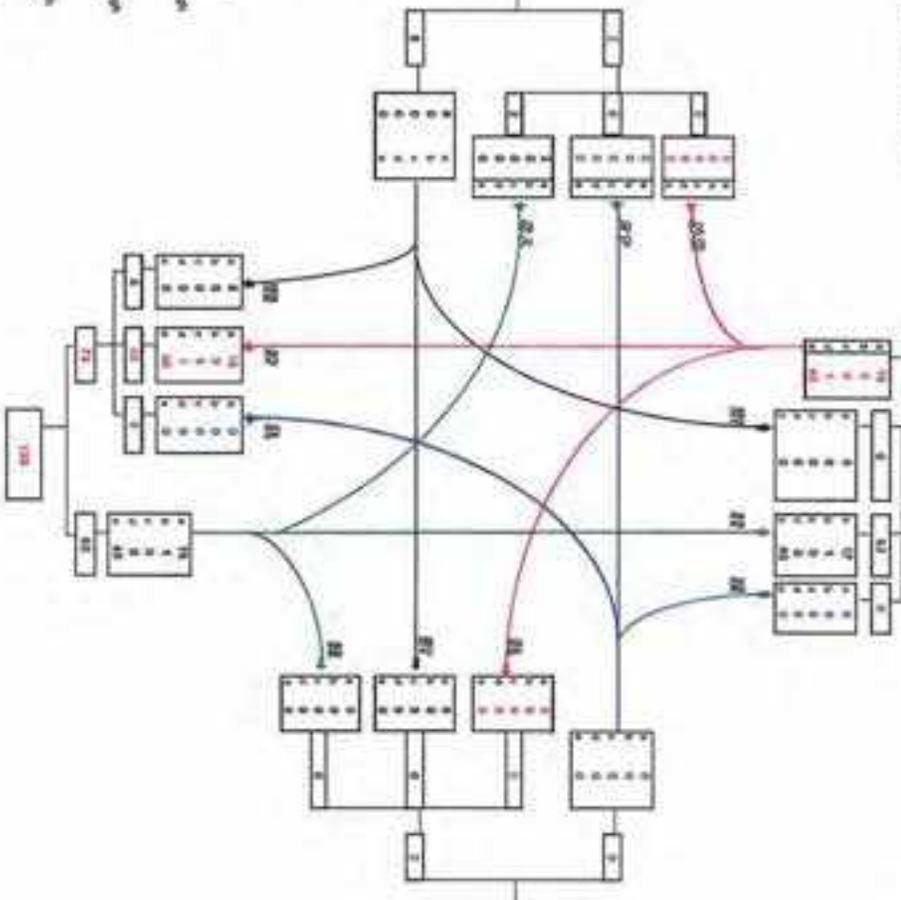


Fig. A4.20

Vehicle Type	Vol	%
Passenger Vehicle	42	89.4
Commercial Vehicle	4	8.6
Truck	1	2.1
Total	47	100.0

Vehicle Type	Vol	%
Passenger Vehicle	42	89.4
Commercial Vehicle	4	8.6
Truck	1	2.1
Total	47	100.0



- TURNING MOVEMENT ABBREVIATIONS**
- NT - North from North Turning Right
 - NL - North from North Turning Left
 - NTL - North from North Proceeding Through
 - SN - South from South Turning Right
 - SL - South from South Turning Left
 - STL - South from South Proceeding Through
 - EN - East from East Turning Right
 - EL - East from East Turning Left
 - ETL - East from East Proceeding Through
 - WN - West from West Turning Right
 - WL - West from West Turning Left
 - WTL - West from West Proceeding Through

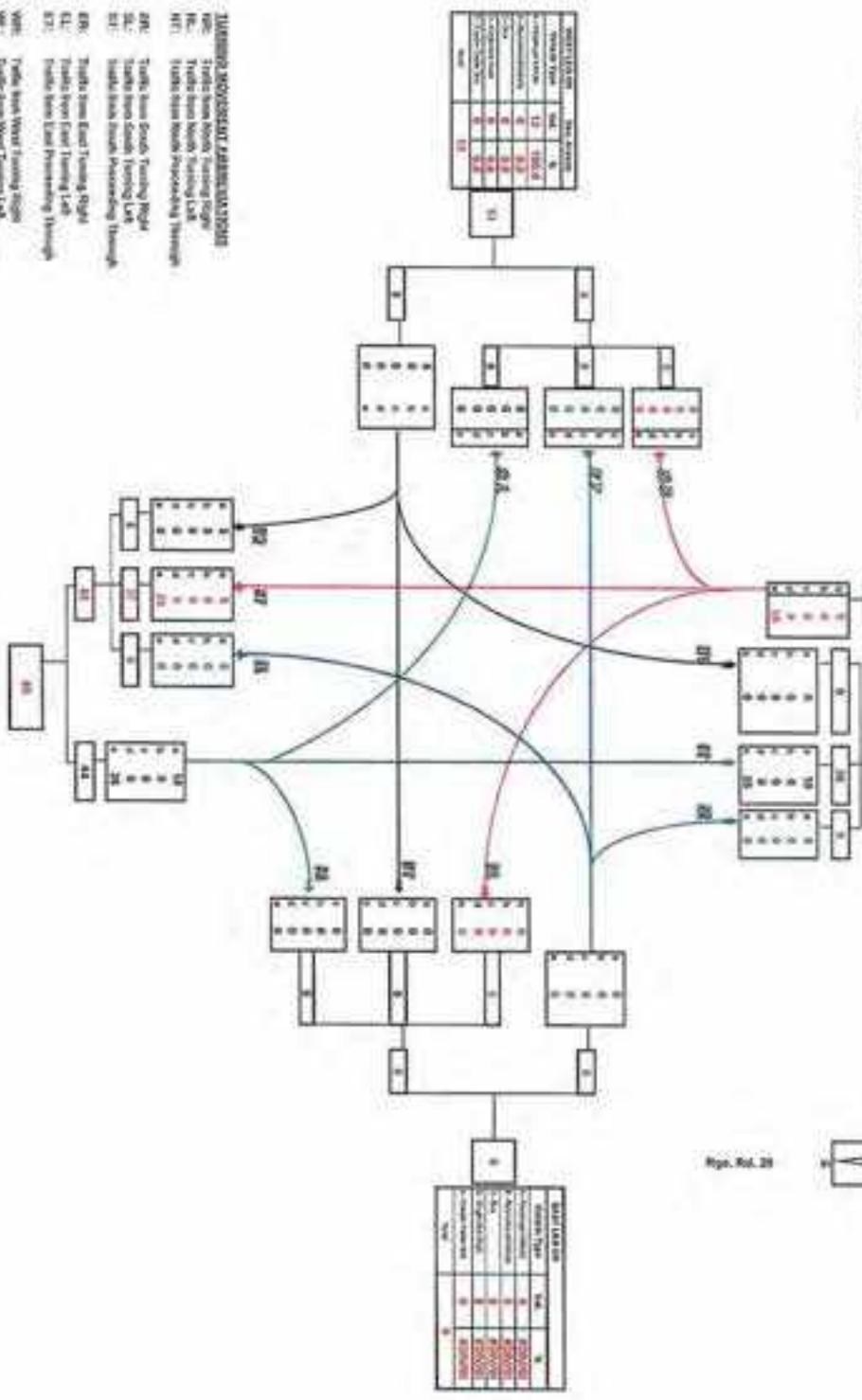
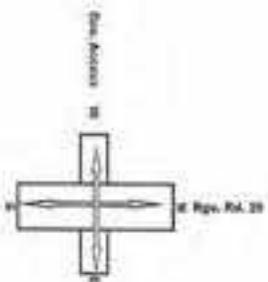
TRAFFIC MOVEMENT ABBREVIATIONS

- ADL - Average Annual Daily Traffic
- ADL₁₅ - Average daily traffic experienced on weekdays per day for period of January 1 to December 31 (260 days)
- ADL₃₀ - Average Annual Daily Traffic
- ADL₃₀ - Average daily traffic experienced on weekdays per day for period of May 1 to September 30 (153 days)

Vehicle Type	Vol	%
Passenger Vehicle	42	89.4
Commercial Vehicle	4	8.6
Truck	1	2.1
Total	47	100.0

**2026 COMBINED
PM PEAK HOURLY VOLUME**

DIRECTION		TRUCKS	PERCENT
Northbound	Left	13	14.8
Northbound	Through	7	8.0
Northbound	Right	3	3.5
Southbound	Left	8	9.2
Southbound	Through	26	30.2
Southbound	Right	11	12.6
Total		88	



WEST LANE		TRUCKS	PERCENT
Through	Left	13	14.8
Through	Through	7	8.0
Through	Right	3	3.5
Left	Left	8	9.2
Left	Through	26	30.2
Left	Right	11	12.6
Total		88	

EAST LANE		TRUCKS	PERCENT
Through	Left	13	14.8
Through	Through	7	8.0
Through	Right	3	3.5
Left	Left	8	9.2
Left	Through	26	30.2
Left	Right	11	12.6
Total		88	

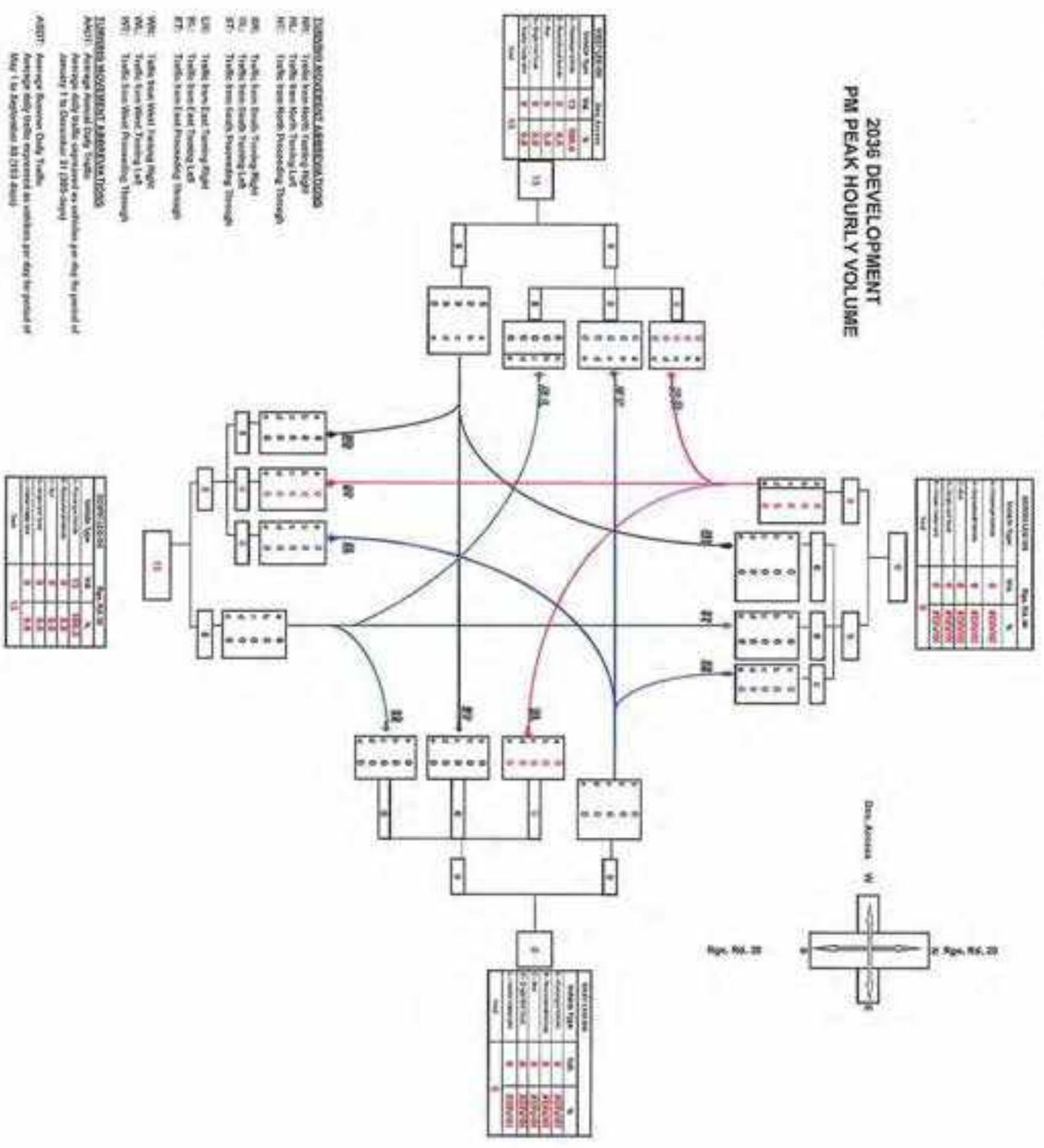
- TURNING MOVEMENT ABBREVIATIONS**
- WLT: Trucks from West Turning Left
 - WTH: Trucks from West Turning Through
 - WRT: Trucks from West Turning Right
 - SLT: Trucks from South Turning Left
 - SLTH: Trucks from South Turning Through
 - SLRT: Trucks from South Turning Right
 - ELT: Trucks from East Turning Left
 - ELTH: Trucks from East Turning Through
 - ELRT: Trucks from East Turning Right
 - NLT: Trucks from North Turning Left
 - NLTH: Trucks from North Turning Through
 - NLRT: Trucks from North Turning Right

GENERAL INFORMATION

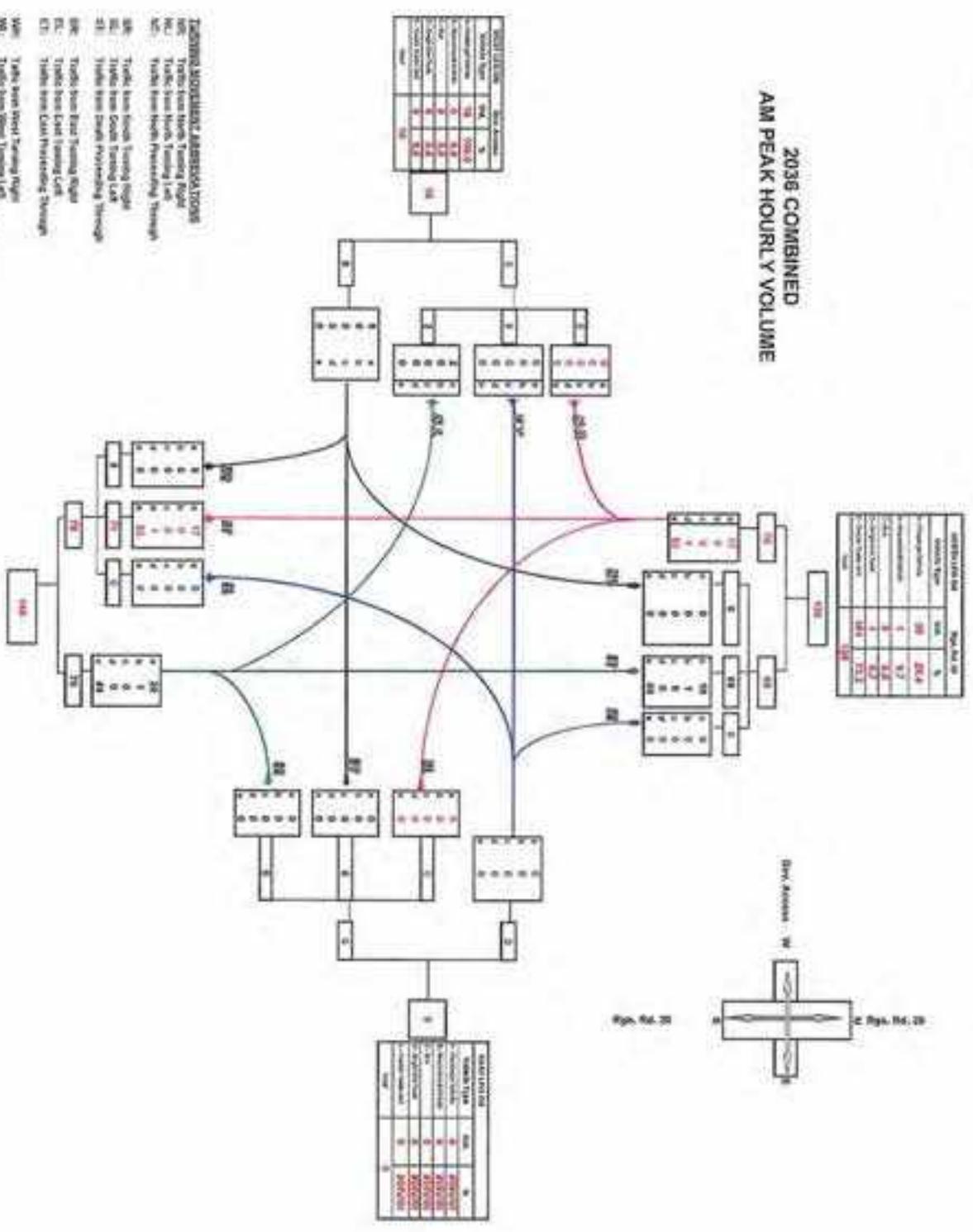
ADDT: Average Annual Daily Traffic
 ADVT: Average Daily Traffic
 ADVT: Average Daily Traffic
 ADVT: Average Daily Traffic

DIRECTION		TRUCKS	PERCENT
Northbound	Left	13	14.8
Northbound	Through	7	8.0
Northbound	Right	3	3.5
Southbound	Left	8	9.2
Southbound	Through	26	30.2
Southbound	Right	11	12.6
Total		88	

**2036 DEVELOPMENT
PM PEAK HOURLY VOLUME**



**2036 COMBINED
AM PEAK HOURLY VOLUME**



APPROACH 04

MOVEMENT	Vol	%
Through	50	83.3
Left	1	1.7
Right	1	1.7
TOTAL	52	100.0

APPROACH 02

MOVEMENT	Vol	%
Through	1	100.0
Left	0	0.0
Right	0	0.0
TOTAL	1	100.0

APPROACH 01

MOVEMENT	Vol	%
Through	2	100.0
Left	0	0.0
Right	0	0.0
TOTAL	2	100.0

TURNING MOVEMENT ASSUMPTIONS

NR: Traffic from North Turning Right
 NL: Traffic from North Turning Left
 NR: Traffic from North Turning Through
 SR: Traffic from South Turning Right
 SL: Traffic from South Turning Left
 SR: Traffic from South Turning Through
 ER: Traffic from East Turning Right
 EL: Traffic from East Turning Left
 ER: Traffic from East Turning Through
 WR: Traffic from West Turning Right
 WL: Traffic from West Turning Left
 WR: Traffic from West Turning Through

TURNING MOVEMENT ASSUMPTIONS

AVR1: Average Annual Daily Traffic
 AVR2: Average daily traffic, expressed as a volume per day for period of January 1 to December 31 (089 Ave)
 ADPT: Average Demand Daily Traffic
 ADPT: Average daily traffic, expressed as a volume per day for period of May 1 to September 30 (151 Ave)

APPROACH 03

MOVEMENT	Vol	%
Through	1	100.0
Left	0	0.0
Right	0	0.0
TOTAL	1	100.0

**2036 COMBINED
PM PEAK HOURLY VOLUME**

SIGNALISED		NON-SIGNALISED	
Vehicle Type	Vol	Vol	%
Through	21	20.3	
Left	3	6.0	
Right	3	6.0	
Left+Right	6	12.0	
Through+Left+Right	30	38.3	100
Total	30	38.3	

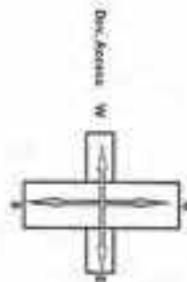
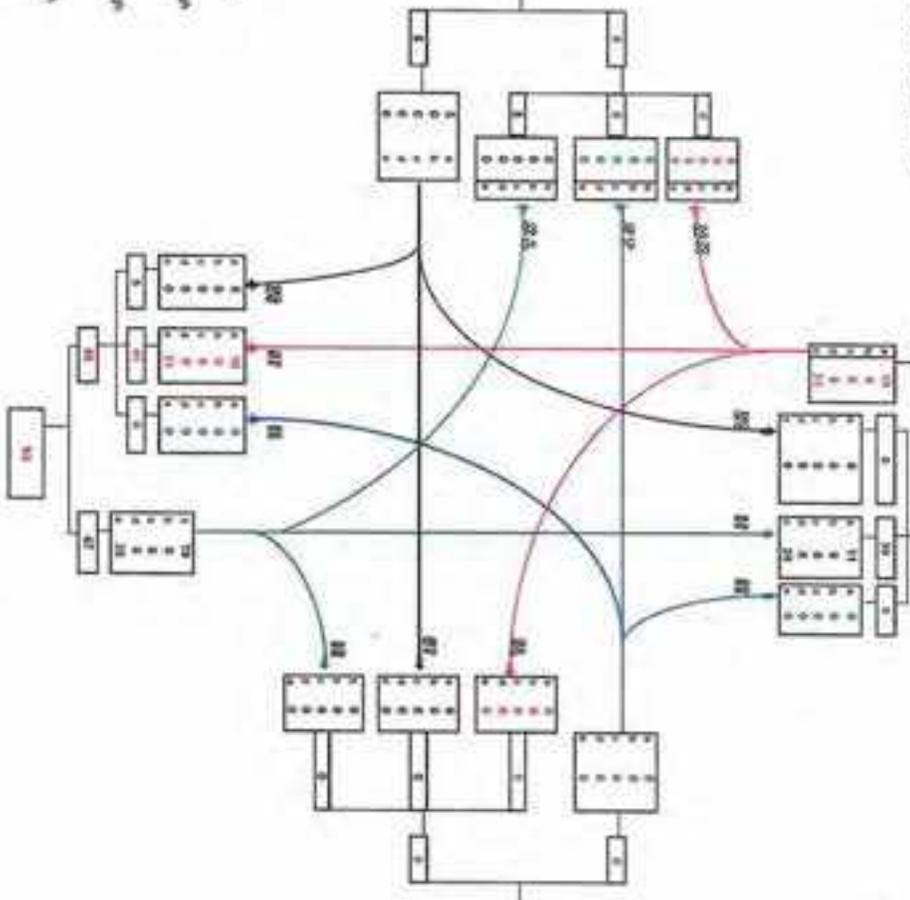


Fig. No. 25

Fig. No. 26

SIGNALISED		NON-SIGNALISED	
Vehicle Type	Vol	Vol	%
Through	13	100.0	
Left	2	15.4	
Right	2	15.4	
Left+Right	4	30.8	
Through+Left+Right	17	130.8	100
Total	17	130.8	



SIGNALISED		NON-SIGNALISED	
Vehicle Type	Vol	Vol	%
Through	13	100.0	
Left	2	15.4	
Right	2	15.4	
Left+Right	4	30.8	
Through+Left+Right	17	130.8	100
Total	17	130.8	

- TURNING MOVEMENT ABREVIATIONS**
- MT: Traffic from North Turning Right
 - ML: Traffic from North Turning Left
 - MTL: Traffic from North Proceeding Through
 - MR: Traffic from South Turning Right
 - MRL: Traffic from South Turning Left
 - MRL: Traffic from South Proceeding Through
 - MLR: Traffic from East Turning Right
 - MLL: Traffic from East Turning Left
 - MLL: Traffic from East Proceeding Through
 - MLR: Traffic from West Turning Right
 - MLL: Traffic from West Turning Left
 - MLL: Traffic from West Proceeding Through

TURNING MOVEMENT ABREVIATIONS

AVGRT: Average Annual Daily Traffic

AVGRL: Average daily traffic represented as vehicles per day for period of January 1 to December 31 (365 days)

SIGNALISED		NON-SIGNALISED	
Vehicle Type	Vol	Vol	%
Through	13	100.0	
Left	2	15.4	
Right	2	15.4	
Left+Right	4	30.8	
Through+Left+Right	17	130.8	100
Total	17	130.8	

AVGRT: Average Annual Daily Traffic

AVGRL: Average daily traffic represented as vehicles per day for period of May 1 to September 30 (153 days)

APPENDIX C – Hwy. 16 & Rge. Rd. 20
TURNING MOVEMENT DIAGRAMS
SYNCHRO ANALYSIS

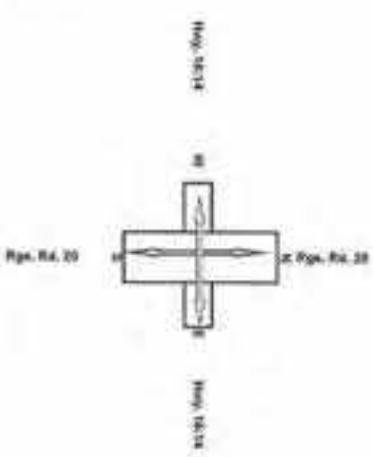
LAUNCH VENTURES RESIDENTIAL
SE 1-54-2-W5M
Loc Ste. Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

TRAFFIC IMPACT ASSESSMENT

Intersection of Highway 5814 & Rgs. No. 28

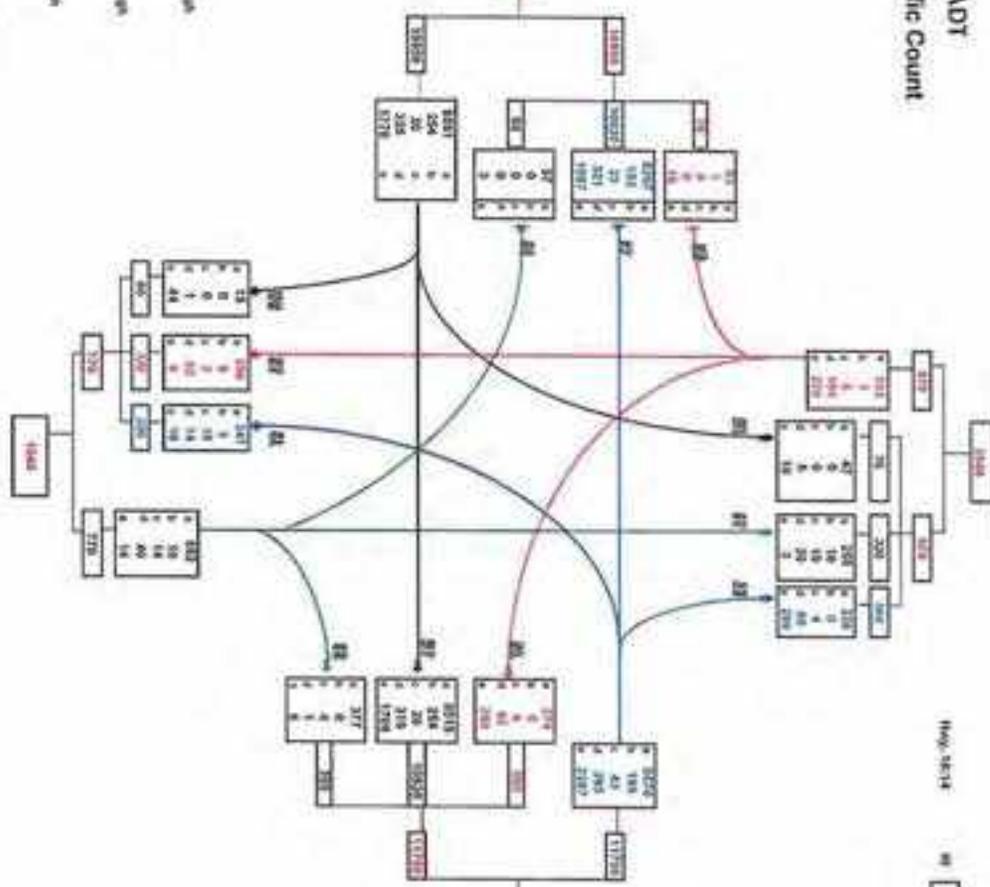
2014 AADT
from AT Traffic Count

SUMMARY		2014 AADT	
MOVEMENT TYPE	TRAFFIC	TRUCKS	%
Through	1154	27.41	
Left	11	0.97	
Right	20	1.74	
Other	406	35.17	
TOTAL	1591	65.29	



SUMMARY		2014 AADT	
MOVEMENT TYPE	TRAFFIC	TRUCKS	%
Through	1080	17.87	
Left	47	4.08	
Right	11	0.95	
Other	161	13.88	
TOTAL	1299	56.78	

SUMMARY		2014 AADT	
MOVEMENT TYPE	TRAFFIC	TRUCKS	%
Through	1018	16.98	
Left	77	6.61	
Right	70	5.91	
Other	181	15.47	
TOTAL	1346	54.97	

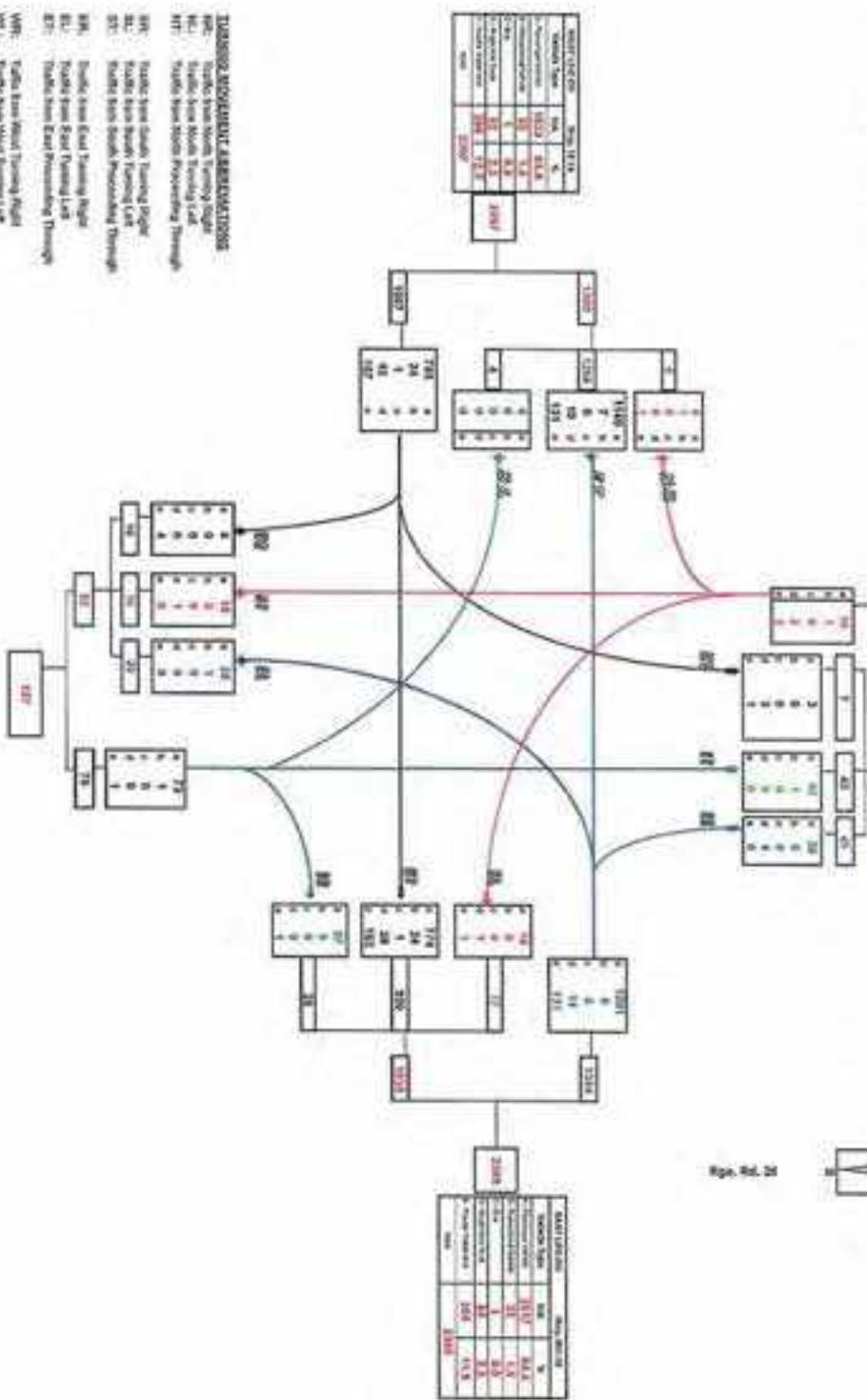
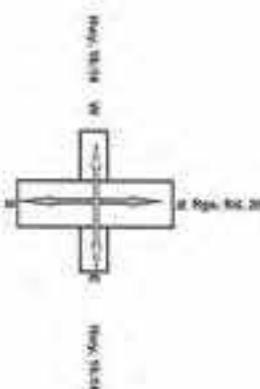


- NE: Traffic from North Turning Right
- NL: Traffic from North Turning Left
- NT: Traffic from North Proceeding Through
- SE: Traffic from South Turning Right
- SL: Traffic from South Turning Left
- ST: Traffic from South Proceeding Through
- EE: Traffic from East Turning Right
- EL: Traffic from East Turning Left
- ET: Traffic from East Proceeding Through
- WE: Traffic from West Turning Right
- WL: Traffic from West Turning Left
- WT: Traffic from West Proceeding Through

SUMMARY		2014 AADT	
MOVEMENT TYPE	TRAFFIC	TRUCKS	%
Through	1154	27.41	
Left	11	0.97	
Right	20	1.74	
Other	406	35.17	
TOTAL	1591	65.29	

2014 PM PEAK HOURLY VOLUME

Direction	Vol. (Veh/hr)	%
Northbound	1,127	69.3
Southbound	77	4.8
Eastbound	7	0.4
Westbound	1	0.1
Total	1,212	



Direction	Vol. (Veh/hr)	%
Northbound	1,127	69.3
Southbound	77	4.8
Eastbound	7	0.4
Westbound	1	0.1
Total	1,212	

Direction	Vol. (Veh/hr)	%
Northbound	1,127	69.3
Southbound	77	4.8
Eastbound	7	0.4
Westbound	1	0.1
Total	1,212	

- TURNING MOVEMENT ABBREVIATIONS:**
- RT - Turns from South, Turning Right
 - LT - Turns from South, Turning Left
 - HT - Turns from South, Proceeding Through
 - RL - Turns from South, Turning Right
 - LL - Turns from South, Turning Left
 - HL - Turns from South, Proceeding Through
 - RR - Turns from East, Turning Right
 - LR - Turns from East, Turning Left
 - HR - Turns from East, Proceeding Through
 - RR - Turns from West, Turning Right
 - LR - Turns from West, Turning Left
 - WR - Turns from West, Proceeding Through

TURNING MOVEMENT ABBREVIATIONS:

Average Annual Daily Traffic:
 Average daily traffic experienced on vehicles per day for period of January 1 to December 31 (2014 days)

NOTES: Average Annual Daily Traffic
 Average daily traffic experienced on vehicles per day for period of May 1 to September 30 (155 days)

Direction	Vol. (Veh/hr)	%
Northbound	1,127	69.3
Southbound	77	4.8
Eastbound	7	0.4
Westbound	1	0.1
Total	1,212	

DATE: 10/20/14
 DRAWN: [Signature]

2015 ESTIMATED ANNUAL DAILY TRAFFIC

2015 ADT		2015 ADT	
Direction	ADT	Direction	ADT
Northbound	1,120	Southbound	1,120
Eastbound	1,120	Westbound	1,120
Total	4,480	Total	4,480

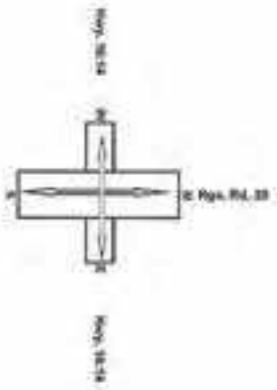
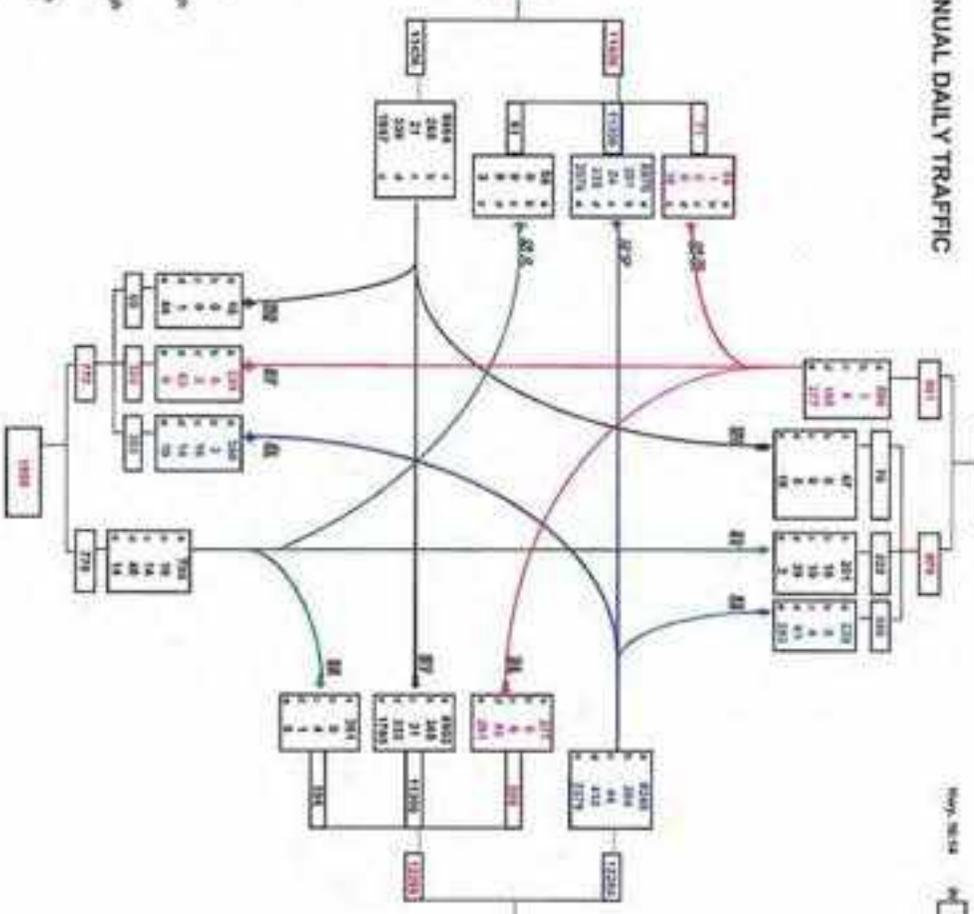


Fig. 14.22

2015 ADT		2015 ADT	
Direction	ADT	Direction	ADT
Northbound	1,120	Southbound	1,120
Eastbound	1,120	Westbound	1,120
Total	4,480	Total	4,480



2015 ADT		2015 ADT	
Direction	ADT	Direction	ADT
Northbound	1,120	Southbound	1,120
Eastbound	1,120	Westbound	1,120
Total	4,480	Total	4,480

- TURNING MOVEMENT ABBREVIATIONS**
- NR: Traffic from North Turning Right
 - NL: Traffic from North Turning Left
 - NR: Traffic from North Proceeding Through
 - NR: Traffic from South Turning Right
 - NL: Traffic from South Turning Left
 - NR: Traffic from South Proceeding Through
 - NR: Traffic from East Turning Right
 - NE: Traffic from East Turning Left
 - NR: Traffic from East Proceeding Through
 - NR: Traffic from West Turning Right
 - NL: Traffic from West Turning Left
 - NR: Traffic from West Proceeding Through

TURNING MOVEMENT ABBREVIATIONS

- NR: Average Annual Daily Traffic
- NR: Average Daily Traffic converted to vehicles per day for period of January 1 to December 31, 2015 (avg)
- NR: Average Season Daily Traffic
- NR: Average daily traffic converted to vehicles per day for period of May 1 to September 30, 2015 (avg)

2015 ADT		2015 ADT	
Direction	ADT	Direction	ADT
Northbound	1,120	Southbound	1,120
Eastbound	1,120	Westbound	1,120
Total	4,480	Total	4,480

**2015 ESTIMATED
AM PEAK HOURLY VOLUME**

SUMMARY DATA		Vol. 1	%
Volume Type	1000	52.3	
Average Speed	5	5.8	
Capacity	10	5.3	
Level of Service	1.1	21.7	
Level	200		

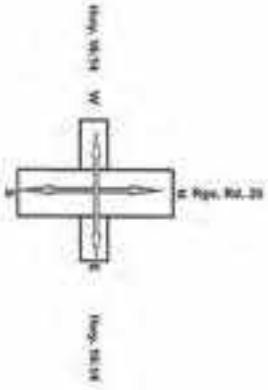
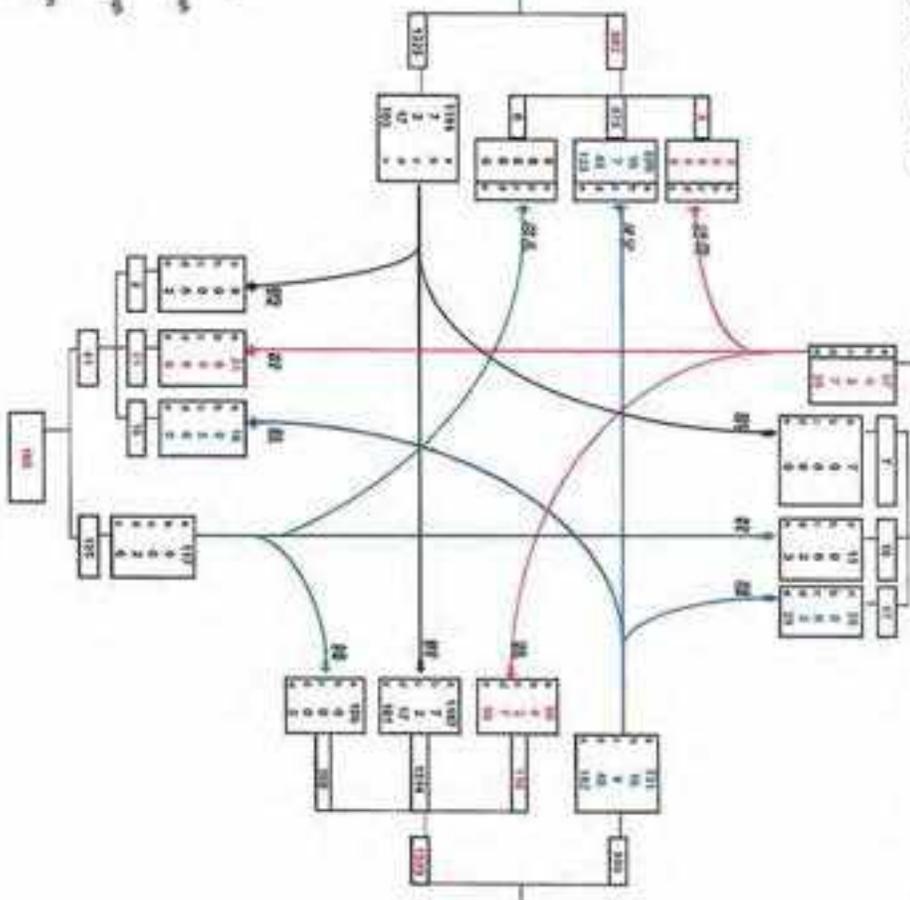


Fig. 04-20

SUMMARY DATA		Vol. 1	%
Volume Type	1000	52.3	
Average Speed	5	5.8	
Capacity	10	5.3	
Level of Service	1.1	21.7	
Level	200		



SUMMARY DATA		Vol. 1	%
Volume Type	1000	52.3	
Average Speed	5	5.8	
Capacity	10	5.3	
Level of Service	1.1	21.7	
Level	200		

- TURNING MOVEMENT ABBREVIATIONS**
- WRT: Traffic from West Turning Right
 - WLT: Traffic from West Turning Left
 - WPT: Traffic from West Proceeding Through
 - SWT: Traffic from South Turning Right
 - SWT: Traffic from South Turning Left
 - SWT: Traffic from South Proceeding Through
 - EWL: Traffic from East Turning Left
 - EWL: Traffic from East Proceeding Through
 - EWL: Traffic from East Turning Right
 - EWL: Traffic from West Turning Left
 - WRT: Traffic from West Proceeding Through

ASSUMPTIONS

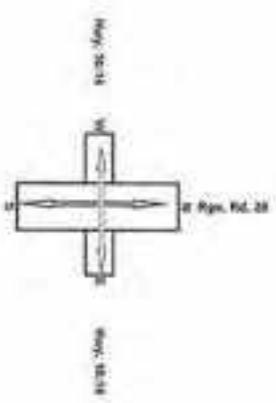
Average Annual Daily Traffic: 1000
 Average daily traffic experienced on weekdays per day for period of January 1 to December 31 (2015 day)

NOTE: Average Annual Daily Traffic
 Average daily traffic experienced on weekdays per day for period of May 1 to September 30 (153 days)

SUMMARY DATA		Vol. 1	%
Volume Type	1000	52.3	
Average Speed	5	5.8	
Capacity	10	5.3	
Level of Service	1.1	21.7	
Level	200		

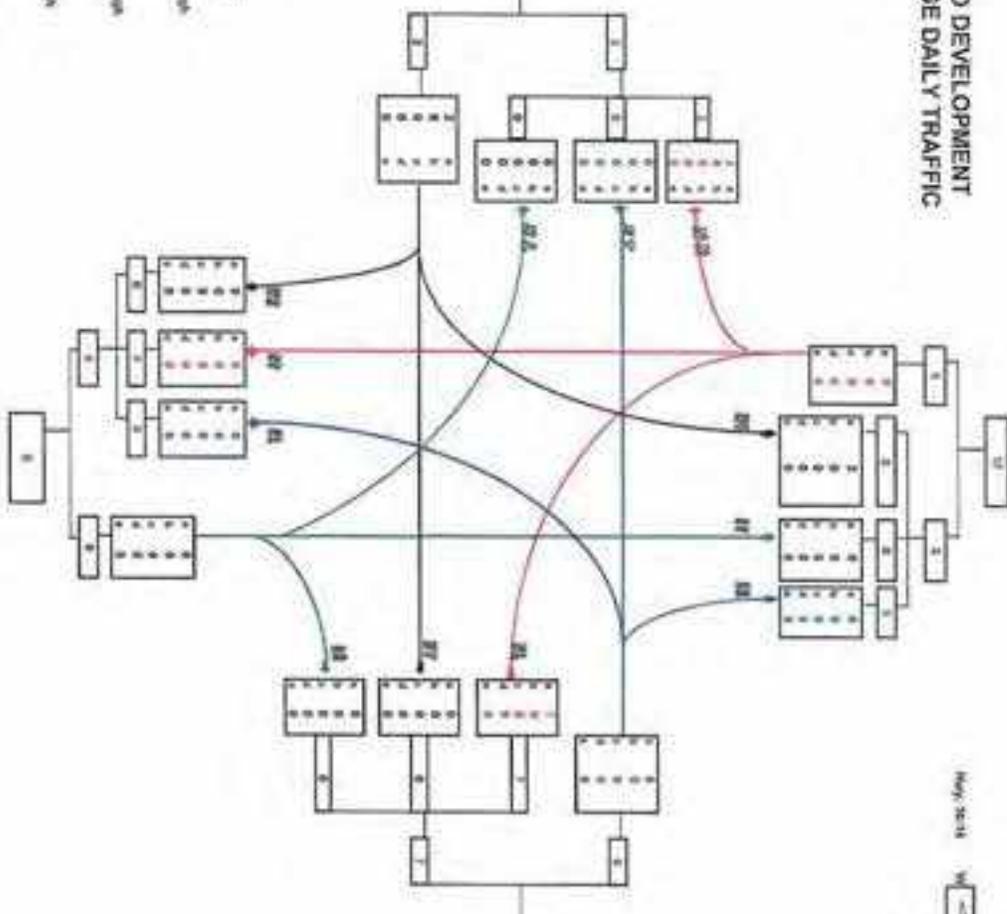
**2016 PROJECTED DEVELOPMENT
ANNUAL AVERAGE DAILY TRAFFIC**

WESTERN AVE			
Direction	Year	ADT	%
Northbound	2016	108,470	
Southbound	2016	108,470	
Total	2016	216,940	
Development	2016	1,200	
Total	2016	218,140	



WESTERN AVE			
Year	ADT	%	ADT
2016	216,940		216,940
2017	218,140		218,140
2018	219,340		219,340
2019	220,540		220,540
2020	221,740		221,740
2021	222,940		222,940
2022	224,140		224,140
2023	225,340		225,340
2024	226,540		226,540
2025	227,740		227,740
2026	228,940		228,940
2027	230,140		230,140
2028	231,340		231,340
2029	232,540		232,540
2030	233,740		233,740
Total	2,316,940		2,316,940

EASTERN AVE			
Year	ADT	%	ADT
2016	108,470		108,470
2017	108,470		108,470
2018	108,470		108,470
2019	108,470		108,470
2020	108,470		108,470
2021	108,470		108,470
2022	108,470		108,470
2023	108,470		108,470
2024	108,470		108,470
2025	108,470		108,470
2026	108,470		108,470
2027	108,470		108,470
2028	108,470		108,470
2029	108,470		108,470
2030	108,470		108,470
Total	3,262,110		3,262,110



- TURNING MOVEMENT ABBREVIATIONS**
- WLT: Traffic from West Turning Right
 - WLT: Traffic from West Turning Left
 - WLT: Traffic from West Proceeding Through
 - WLT: Traffic from West Turning Right
 - WLT: Traffic from West Turning Left
 - WLT: Traffic from West Proceeding Through
 - WLT: Traffic from West Turning Right
 - WLT: Traffic from West Turning Left
 - WLT: Traffic from West Proceeding Through
 - WLT: Traffic from West Turning Right
 - WLT: Traffic from West Turning Left
 - WLT: Traffic from West Proceeding Through
 - WLT: Traffic from West Turning Right
 - WLT: Traffic from West Turning Left
 - WLT: Traffic from West Proceeding Through

TURNING MOVEMENT ABBREVIATIONS

ADT: Average Annual Daily Traffic

ADT: Average daily traffic estimated as vehicles per day for period of January 1 to December 31 (2016 ADT)

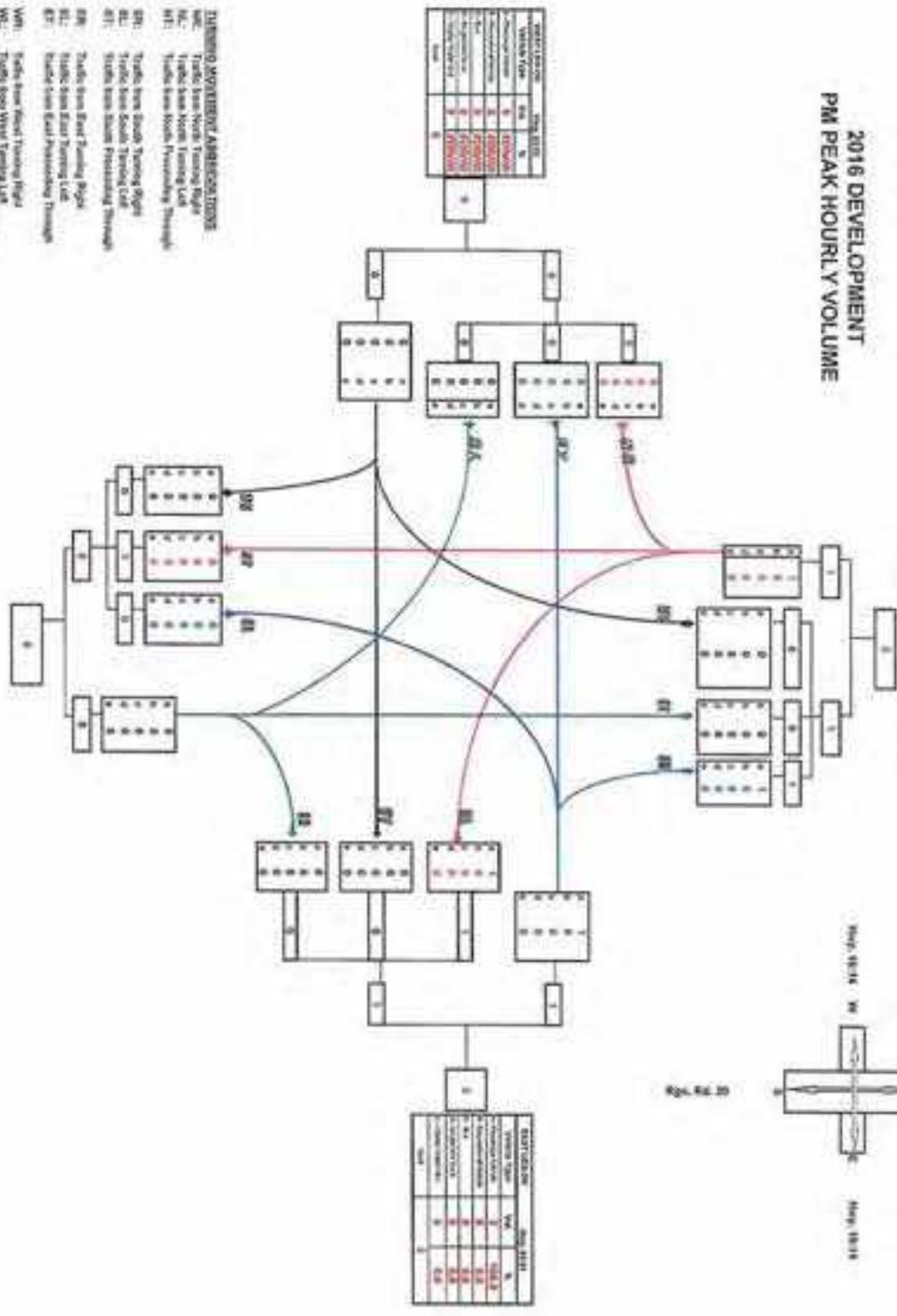
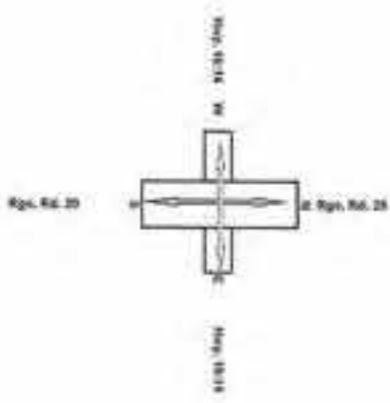
ADT: Average Annual Daily Traffic

ADT: Average daily traffic estimated as vehicles per day for period of May 1 to September 30 (2016 ADT)

WESTERN AVE			
Year	ADT	%	ADT
2016	216,940		216,940
2017	218,140		218,140
2018	219,340		219,340
2019	220,540		220,540
2020	221,740		221,740
2021	222,940		222,940
2022	224,140		224,140
2023	225,340		225,340
2024	226,540		226,540
2025	227,740		227,740
2026	228,940		228,940
2027	230,140		230,140
2028	231,340		231,340
2029	232,540		232,540
2030	233,740		233,740
Total	2,316,940		2,316,940

2016 DEVELOPMENT
PM PEAK HOURLY VOLUME

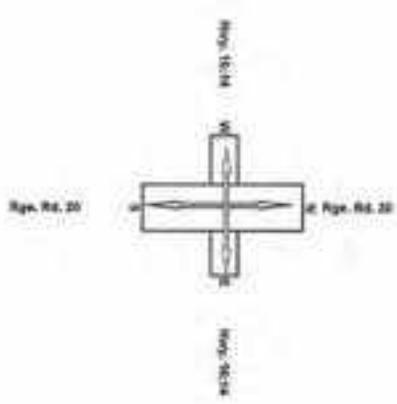
OPERATION			
Vehicle Type	Vol	Vol. %	Vol. %
Through	7	100%	0%
Left	0	0%	0%
Right	0	0%	0%
Total	7	100%	0%



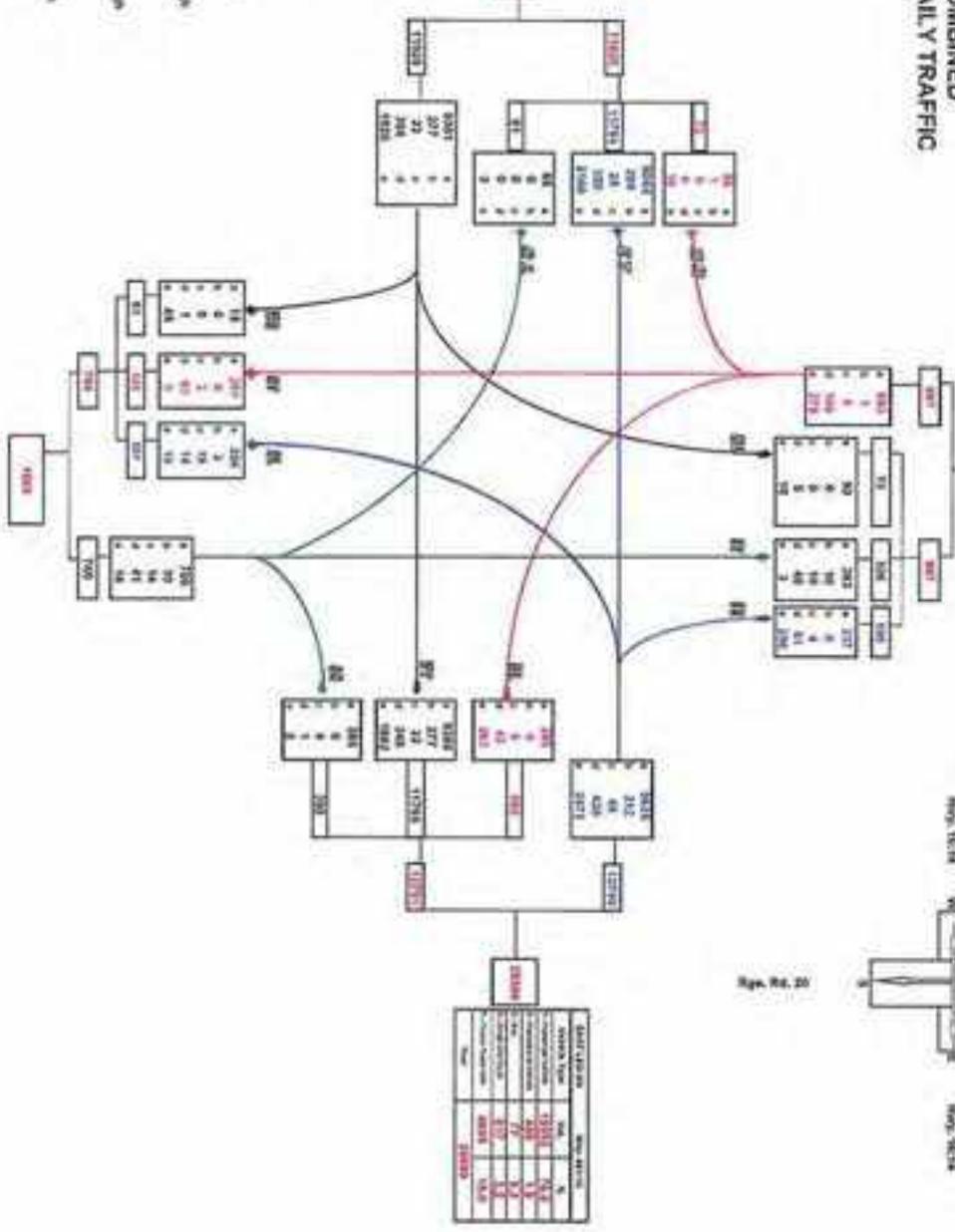
ASOT: Average Section Daily Traffic
Average daily traffic reported on sections per day for period of May 1 to September 30 (165 days)

2016 COMBINED ANNUAL DAILY TRAFFIC

DAILY TRAFFIC			
Direction	Vol.	ADT	%
Northbound	1180	1180	-
Southbound	1170	1170	-
Westbound	217	217	-
Eastbound	208	208	-
Total	2773	2773	100%



DAILY TRAFFIC			
Direction	Vol.	ADT	%
Northbound	1180	1180	-
Southbound	1170	1170	-
Westbound	217	217	-
Eastbound	208	208	-
Total	2773	2773	100%



DAILY TRAFFIC			
Direction	Vol.	ADT	%
Northbound	1180	1180	-
Southbound	1170	1170	-
Westbound	217	217	-
Eastbound	208	208	-
Total	2773	2773	100%

- TURNING MOVEMENT ABBREVIATIONS**
- NR: Traffic from North Turning Right
 - NL: Traffic from North Turning Left
 - NR: Traffic from North Proceeding Through
 - SR: Traffic from South Turning Right
 - SL: Traffic from South Turning Left
 - SR: Traffic from South Proceeding Through
 - WR: Traffic from West Turning Right
 - WL: Traffic from West Turning Left
 - WR: Traffic from West Proceeding Through
 - ER: Traffic from East Turning Right
 - EL: Traffic from East Turning Left
 - ER: Traffic from East Proceeding Through

DAILY TRAFFIC			
Direction	Vol.	ADT	%
Northbound	1180	1180	-
Southbound	1170	1170	-
Westbound	217	217	-
Eastbound	208	208	-
Total	2773	2773	100%

NOTES: Average Annual Daily Traffic
 Average Annual Daily Traffic
 Average Daily Traffic
 Average Daily Traffic
 Average Daily Traffic
 Average Daily Traffic

DAILY TRAFFIC			
Direction	Vol.	ADT	%
Northbound	1180	1180	-
Southbound	1170	1170	-
Westbound	217	217	-
Eastbound	208	208	-
Total	2773	2773	100%

2016 COMBINED AM PEAK HOURLY VOLUME

MOVEMENT	Vol. (Veh/hr)	%
Through	122	50.0
Left	6	5.0
Right	6	5.0
Left+Right	12	10.0
Through+Left+Right	140	100.0
Total	140	100.0

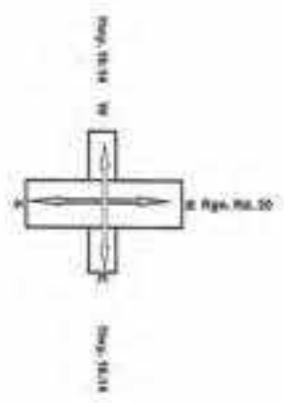
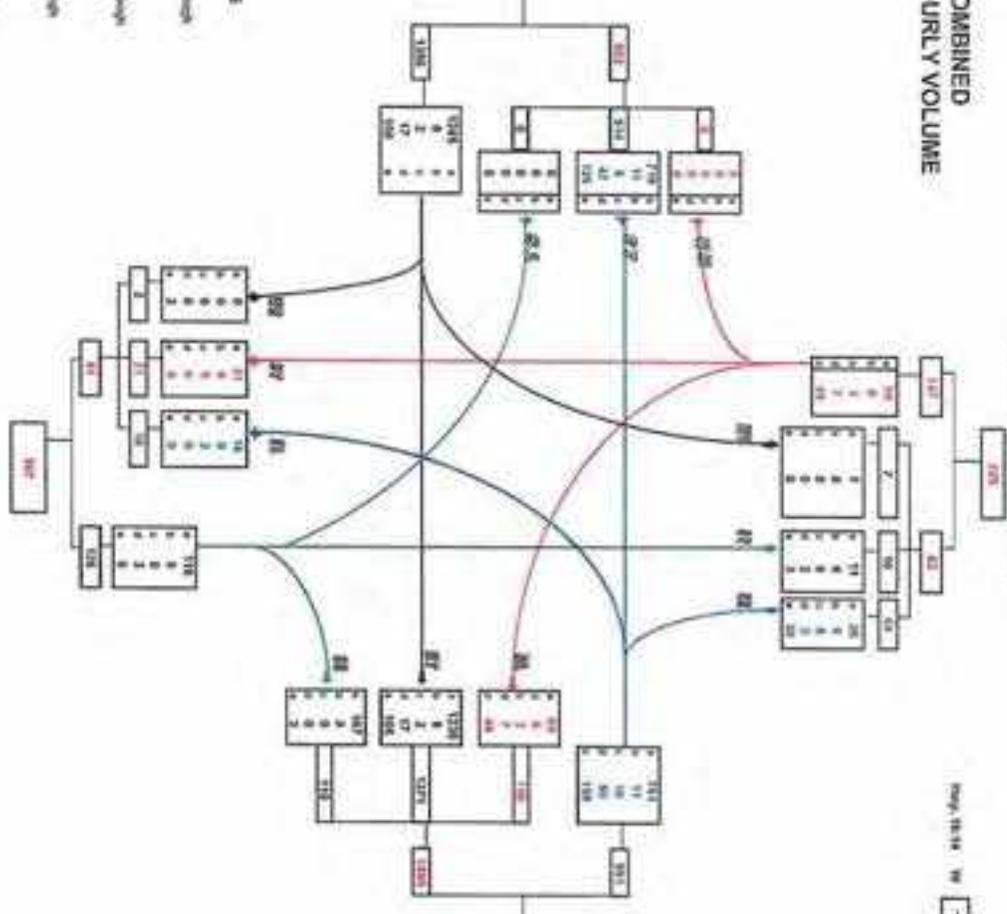


Fig. Rd. 20

MOVEMENT	Vol. (Veh/hr)	%
Through	207	84.7
Left	16	6.4
Right	16	6.4
Left+Right	32	12.8
Through+Left+Right	239	100.0
Total	239	100.0



MOVEMENT	Vol. (Veh/hr)	%
Through	177	80.5
Left	12	5.5
Right	12	5.5
Left+Right	24	11.0
Through+Left+Right	213	100.0
Total	213	100.0

- TURNING MOVEMENT ABBREVIATIONS**
- TR: Traffic from North Turning Right
 - TL: Traffic from North Turning Left
 - TH: Traffic from North Proceeding Through
 - TR: Traffic from South Turning Right
 - TL: Traffic from South Turning Left
 - TH: Traffic from South Proceeding Through
 - LR: Traffic from West Turning Right
 - LL: Traffic from West Turning Left
 - LT: Traffic from West Proceeding Through
 - RR: Traffic from East Turning Right
 - RL: Traffic from East Turning Left
 - RE: Traffic from East Proceeding Through

TURNING MOVEMENT ABBREVIATIONS

AVOL: Average Annual Daily Volume

ADVOL: Average Daily Volume - reported on vehicles per day for period of January 1 to December 31 (2016 days)

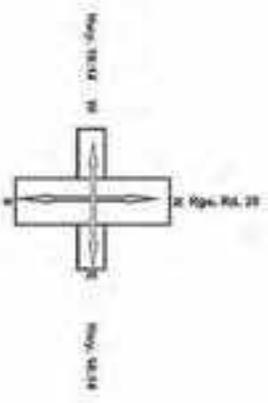
ADVOL: Average Annual Daily Volume

ADVOL: Average Daily Volume - reported on vehicles per day for period of May 1 to September 30 (153 days)

MOVEMENT	Vol. (Veh/hr)	%
Through	10	10.0
Left	1	1.0
Right	1	1.0
Left+Right	2	2.0
Through+Left+Right	13	100.0
Total	13	100.0

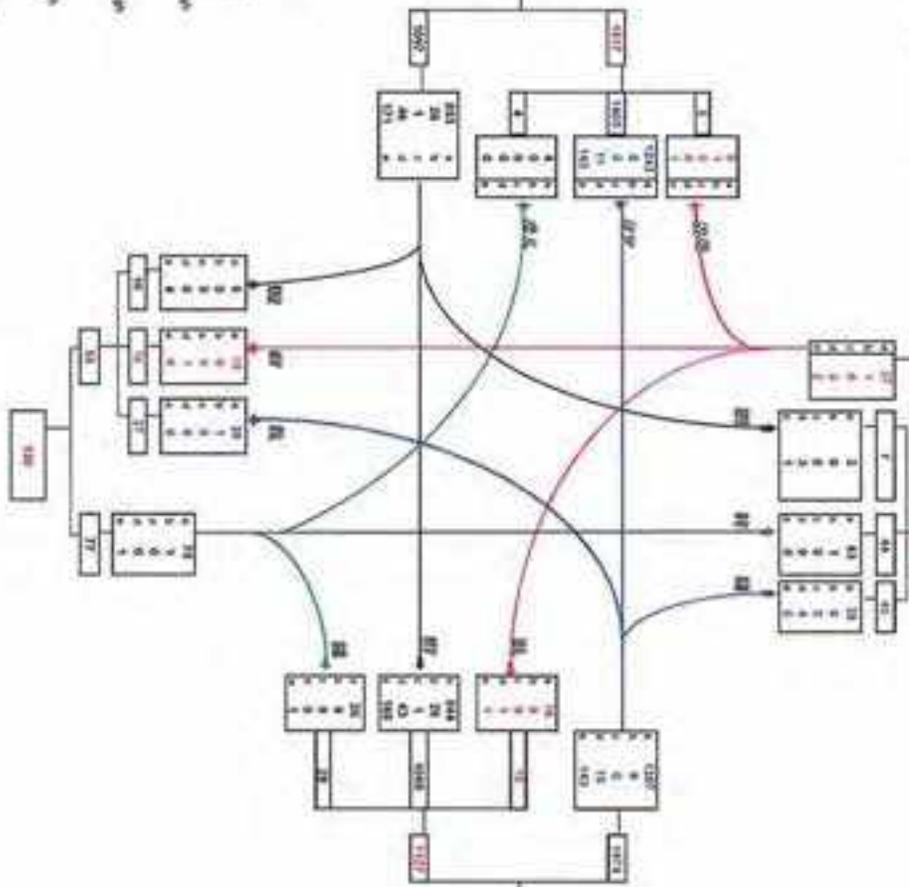
2016 COMBINED PM PEAK HOURLY VOLUME

WESTBOUND			
MOVEMENT	Vol	%	SA/T
THRU	137	85.3	
RIGHT	2	1.3	
LEFT	4	2.5	
TOTAL	143		



EASTBOUND			
MOVEMENT	Vol	%	SA/T
THRU	790	82.8	
RIGHT	7	0.7	
LEFT	7	0.7	
TOTAL	804		

SOUTHBOUND			
MOVEMENT	Vol	%	SA/T
THRU	210	81.5	
RIGHT	7	2.5	
LEFT	17	6.2	
TOTAL	234		

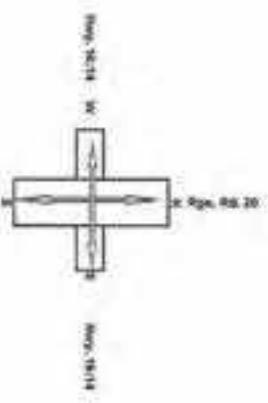


- TURNING MOVEMENT ABBREVIATIONS**
- THRU: Thru from South Turning Right
 - TR: Thru from South Turning Right
 - TR: Thru from South Turning Left
 - TR: Thru from South Proceeding Through
 - TR: Thru from South Turning Right
 - TR: Thru from South Turning Left
 - TR: Thru from South Proceeding Through
 - TR: Thru from East Turning Right
 - TR: Thru from East Turning Left
 - TR: Thru from East Proceeding Through
 - TR: Thru from West Turning Right
 - TR: Thru from West Turning Left
 - TR: Thru from West Proceeding Through

SOUTHBOUND			
MOVEMENT	Vol	%	SA/T
THRU	137	85.3	
RIGHT	2	1.3	
LEFT	4	2.5	
TOTAL	143		

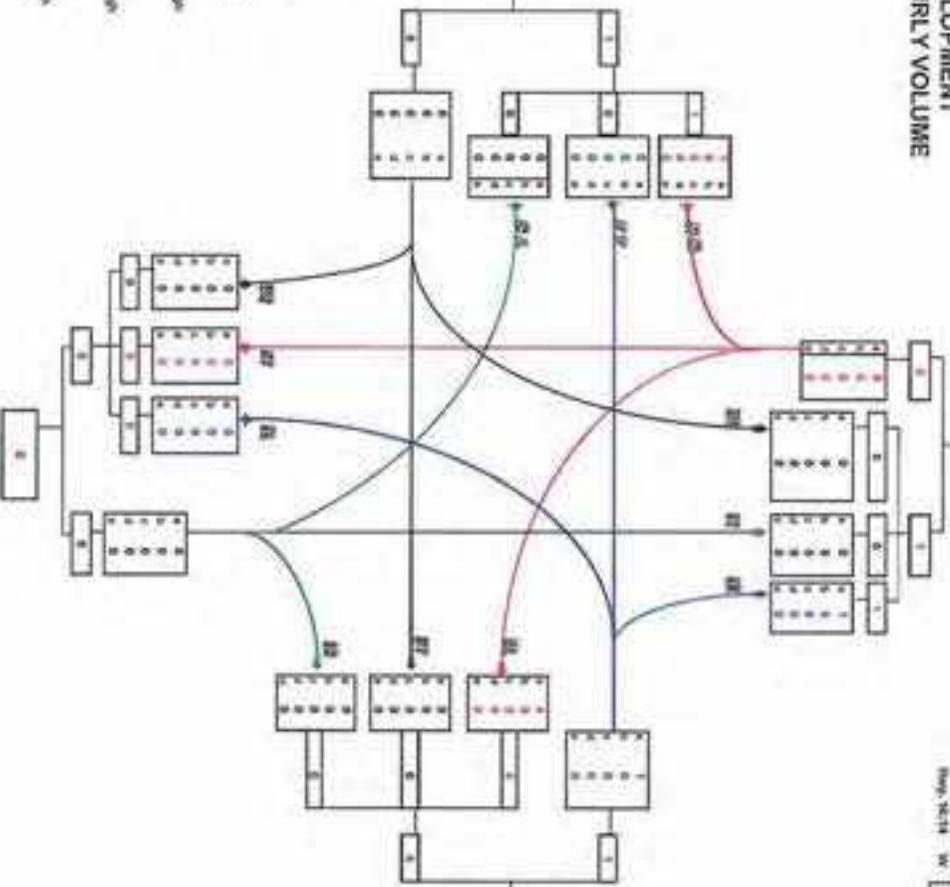
**2021 DEVELOPMENT
AM PEAK HOURLY VOLUME**

DIRECTION		Peak Hour (h)	
Volume	%	Vol	%
Northbound	9	150	9.3
Southbound	9	150	9.3
Eastbound	9	150	9.3
Westbound	9	150	9.3
Total	36	600	36.2



DIRECTION		Peak Hour (h)	
Volume	%	Vol	%
Northbound	1	100	100
Southbound	1	100	100
Eastbound	1	100	100
Westbound	1	100	100
Total	4	400	400

DIRECTION		Peak Hour (h)	
Volume	%	Vol	%
Northbound	1	100	100
Southbound	1	100	100
Eastbound	1	100	100
Westbound	1	100	100
Total	4	400	400



- TURNING MOVEMENT ASSUMPTIONS**
- TR: Traffic from South Turning Right
 - TRC: Traffic from South Turning Left
 - TRL: Traffic from South Turning Left
 - TRT: Traffic from South Through
 - TRC: Traffic from South Turning Right
 - TRC: Traffic from South Turning Left
 - TRC: Traffic from South Through
 - TRC: Traffic from East Turning Right
 - TRC: Traffic from East Turning Left
 - TRC: Traffic from East Through
 - TRC: Traffic from West Turning Right
 - TRC: Traffic from West Turning Left
 - TRC: Traffic from West Through

TURNING MOVEMENT ASSUMPTIONS

TR: Traffic from South Turning Right
 TRC: Traffic from South Turning Left
 TRL: Traffic from South Turning Left
 TRT: Traffic from South Through

TRC: Traffic from South Turning Right
 TRC: Traffic from South Turning Left
 TRC: Traffic from South Through

TRC: Traffic from East Turning Right
 TRC: Traffic from East Turning Left
 TRC: Traffic from East Through

TRC: Traffic from West Turning Right
 TRC: Traffic from West Turning Left
 TRC: Traffic from West Through

ADDITIONAL ASSUMPTIONS

ADOT: Average Annual Daily Traffic
 Average daily traffic reported as vehicles per day for period of January 1 to December 31 (2015 only)

ADOT: Average Annual Daily Traffic
 Average daily traffic reported as vehicles per day for period of May 1 to September 30 (2015 only)

DIRECTION		Peak Hour (h)	
Volume	%	Vol	%
Northbound	9	150	9.3
Southbound	9	150	9.3
Eastbound	9	150	9.3
Westbound	9	150	9.3
Total	36	600	36.2

**2021 DEVELOPMENT
PM PEAK HOURLY VOLUME**

SUMMARY TABLE				
Volume Type	Vol.	PHV	PHV/PH	%
Through	9	9	100%	100%
Left	0	0	0%	0%
Right	0	0	0%	0%
TOTAL	9	9	100%	100%

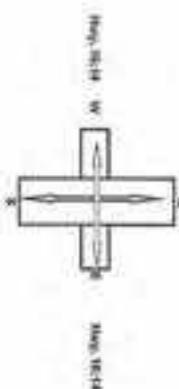
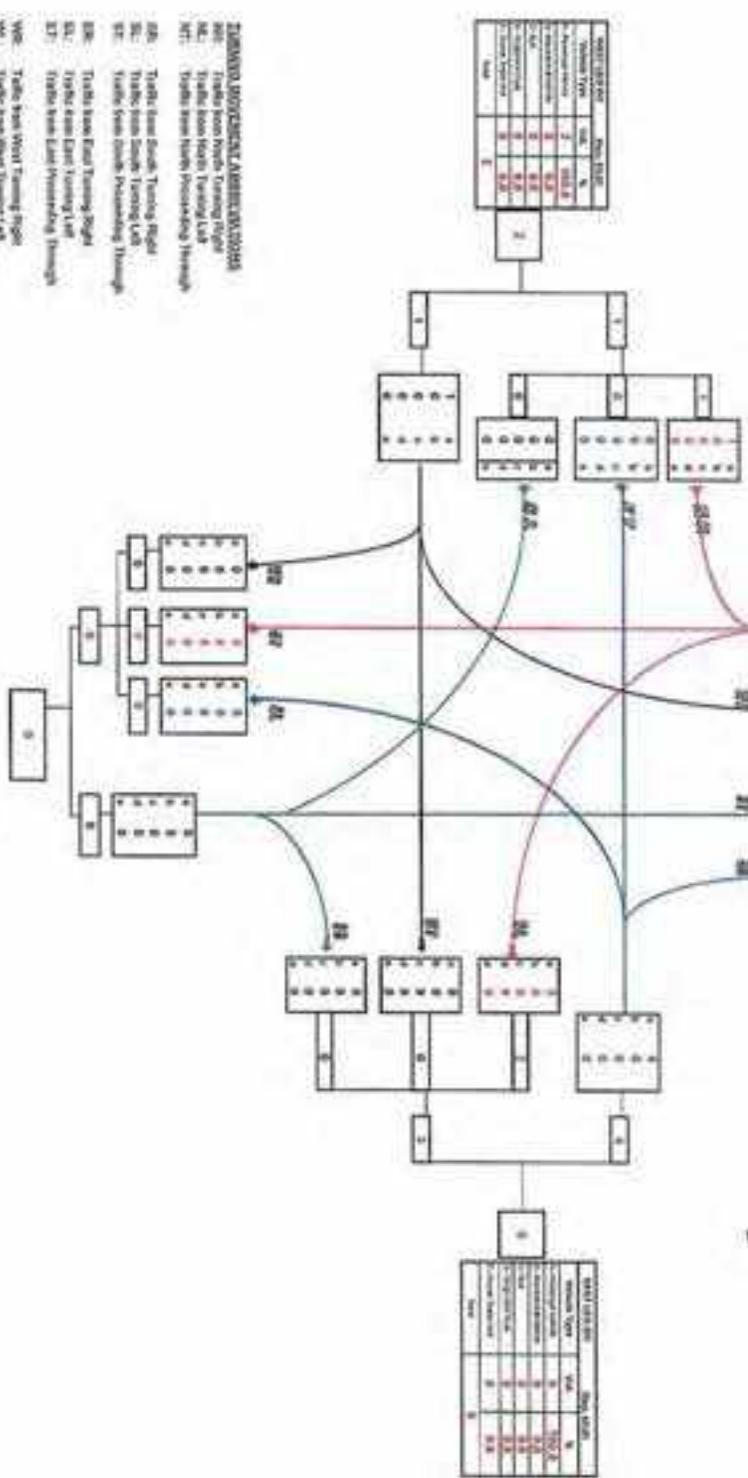


Fig. P4.20

Fig. P4.20



SUMMARY TABLE				
Volume Type	Vol.	PHV	PHV/PH	%
Through	7	7	100%	100%
Left	0	0	0%	0%
Right	0	0	0%	0%
TOTAL	7	7	100%	100%

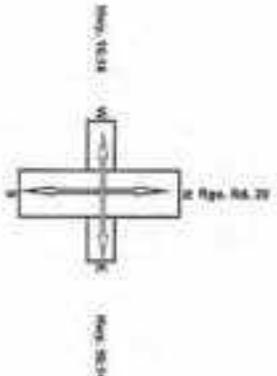
SUMMARY TABLE				
Volume Type	Vol.	PHV	PHV/PH	%
Through	0	0	0%	0%
Left	0	0	0%	0%
Right	0	0	0%	0%
TOTAL	0	0	0%	0%

- TURNING MOVEMENT ABREVIATIONS**
- TH: Traffic from North Turning Right
 - TL: Traffic from North Turning Left
 - TR: Traffic from North Turning Through
 - SH: Traffic from South Turning Right
 - SL: Traffic from South Turning Left
 - SR: Traffic from South Turning Through
 - EH: Traffic from East Turning Right
 - EL: Traffic from East Turning Left
 - ER: Traffic from East Turning Through
 - WH: Traffic from West Turning Right
 - WL: Traffic from West Turning Left
 - WR: Traffic from West Turning Through
- TURNING MOVEMENT ABREVIATIONS**
- AVO: Average Annual Daily Traffic
 - AVO_{PM}: Average daily traffic, expressed as vehicles per day, for period of January 1 to December 31 (24 hr)
 - AVO_{PM}: Average summer daily traffic
 - AVO_{PM}: Average daily traffic, expressed as vehicles per day for period of May 1 to September 30 (15 hr)

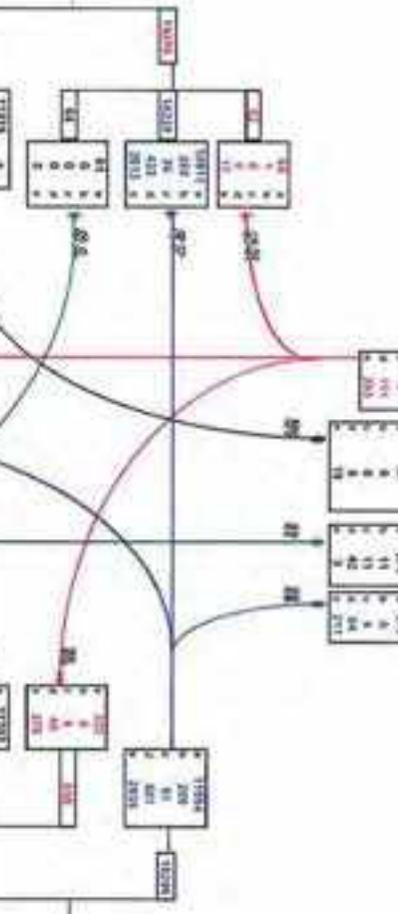
SUMMARY TABLE				
Volume Type	Vol.	PHV	PHV/PH	%
Through	0	0	0%	0%
Left	0	0	0%	0%
Right	0	0	0%	0%
TOTAL	0	0	0%	0%

2021 COMBINED ANNUAL DAILY TRAFFIC

ROADWAY TOTAL		Avg. Daily Traffic	
Direction	Volume	ADT	%
Northbound	1290	12	1%
Southbound	210	2	0%
Total	1500	14	1%



WESTBOUND		Avg. Daily Traffic	
Volume Type	Volume	ADT	%
Through	1290	12	1%
Left	210	2	0%
Right	0	0	0%
Total	1500	14	1%



EASTBOUND		Avg. Daily Traffic	
Volume Type	Volume	ADT	%
Through	1290	12	1%
Left	210	2	0%
Right	0	0	0%
Total	1500	14	1%

- TURNING MOVEMENT NOTATION**
- TR: Traffic from South Turning Right
 - TL: Traffic from South Turning Left
 - LT: Traffic from South Proceeding Through
 - TR: Traffic from South Turning Right
 - TL: Traffic from South Turning Left
 - LT: Traffic from South Proceeding Through
 - TR: Traffic from East Turning Right
 - TL: Traffic from East Turning Left
 - LT: Traffic from East Proceeding Through
 - TR: Traffic from West Turning Right
 - TL: Traffic from West Turning Left
 - LT: Traffic from West Proceeding Through

TURNING MOVEMENT ASSUMPTIONS

ADT: Average Annual Daily Traffic
 ADT: Average daily traffic, estimated by activity per day, or provided at Agency 1 to December 31 (ADT days)

ADDT: Average December Daily Traffic
 Average daily traffic, estimated as vehicles per day for period of May 1 to September 30 (153 days)

ROADWAY TOTAL		Avg. Daily Traffic	
Direction	Volume	ADT	%
Northbound	1290	12	1%
Southbound	210	2	0%
Total	1500	14	1%

**2021 COMBINED
AM PEAK HOURLY VOLUME**

MOVEMENT	Vol. (Veh./hr)	%	SA/T
Northbound	1,033	34.3	
Southbound	9	0.3	
Eastbound	2	0.1	
Westbound	11	0.4	
Left Turn	26	0.8	
Right Turn	243	7.8	
Total	2,983	100	

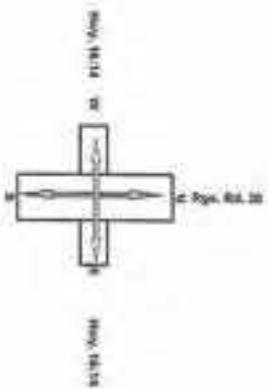
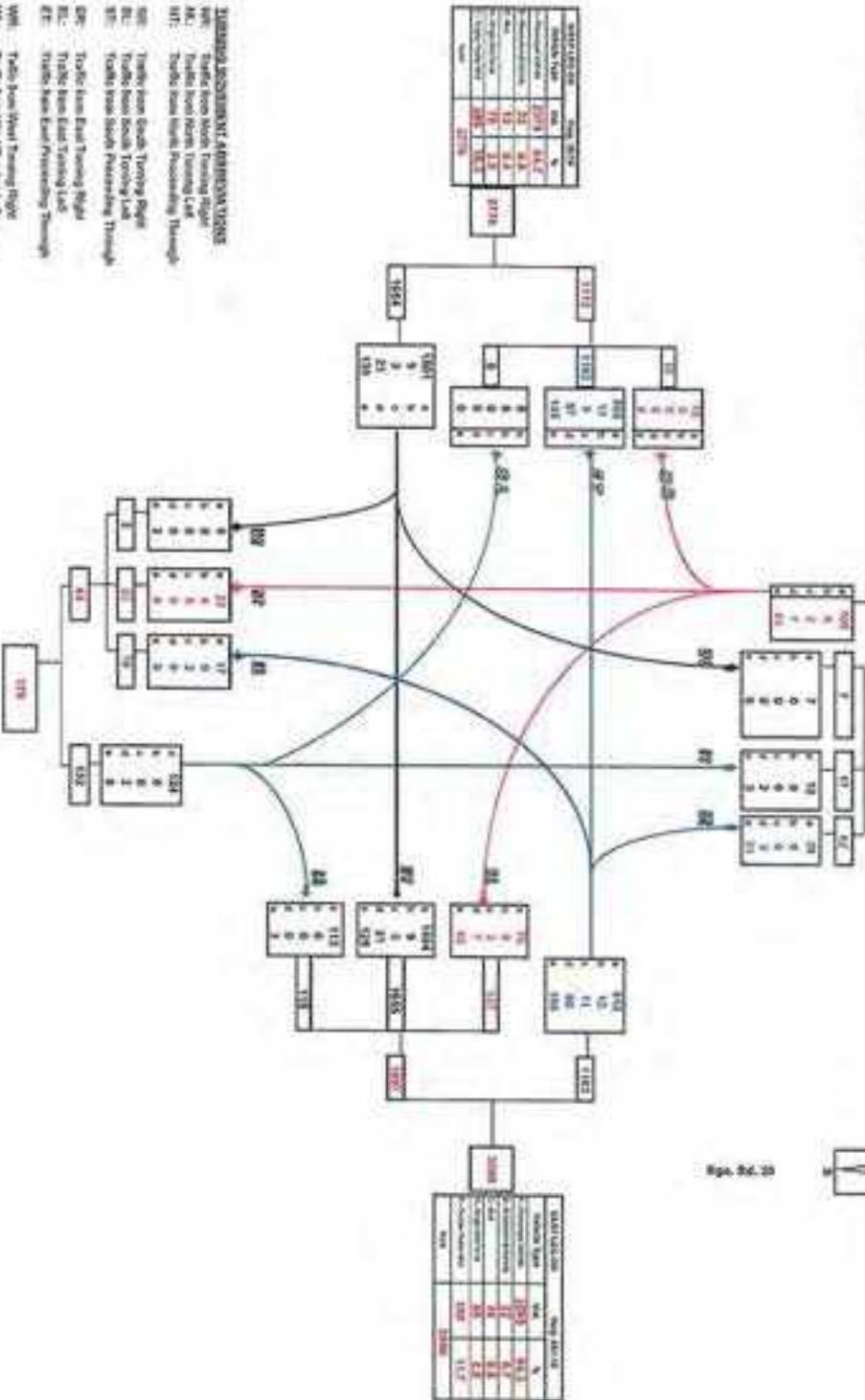


Fig. 24.10



STATISTICAL	Vol. (Veh./hr)	%	SA/T
Northbound	1,033	34.3	
Southbound	9	0.3	
Eastbound	2	0.1	
Westbound	11	0.4	
Left Turn	26	0.8	
Right Turn	243	7.8	
Total	2,983	100	

STATISTICAL	Vol. (Veh./hr)	%	SA/T
Northbound	1,033	34.3	
Southbound	9	0.3	
Eastbound	2	0.1	
Westbound	11	0.4	
Left Turn	26	0.8	
Right Turn	243	7.8	
Total	2,983	100	

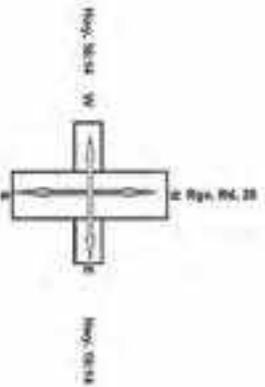
- TURNING MOVEMENT ABBREVIATIONS**
- NT: Traffic from North Turning Right
 - NR: Traffic from North Turning Right
 - NR: Traffic from North Turning Left
 - NR: Traffic from North Proceeding Through
 - NR: Traffic from South Turning Right
 - NR: Traffic from South Turning Left
 - NR: Traffic from South Proceeding Through
 - NR: Traffic from East Turning Right
 - NR: Traffic from East Turning Left
 - NR: Traffic from East Proceeding Through
 - NR: Traffic from West Turning Right
 - NR: Traffic from West Turning Left
 - NR: Traffic from West Proceeding Through

- TURNING MOVEMENT ABBREVIATIONS**
- NR: Average Annual Daily Traffic
 - NR: Average daily traffic captured as vehicles per day for period of January 1 to December 31 (2021 days)
 - NR: Average Annual Daily Traffic
 - NR: Average daily traffic captured as vehicles per day for period of May 1 to September 30 (153 days)

STATISTICAL	Vol. (Veh./hr)	%	SA/T
Northbound	1,033	34.3	
Southbound	9	0.3	
Eastbound	2	0.1	
Westbound	11	0.4	
Left Turn	26	0.8	
Right Turn	243	7.8	
Total	2,983	100	

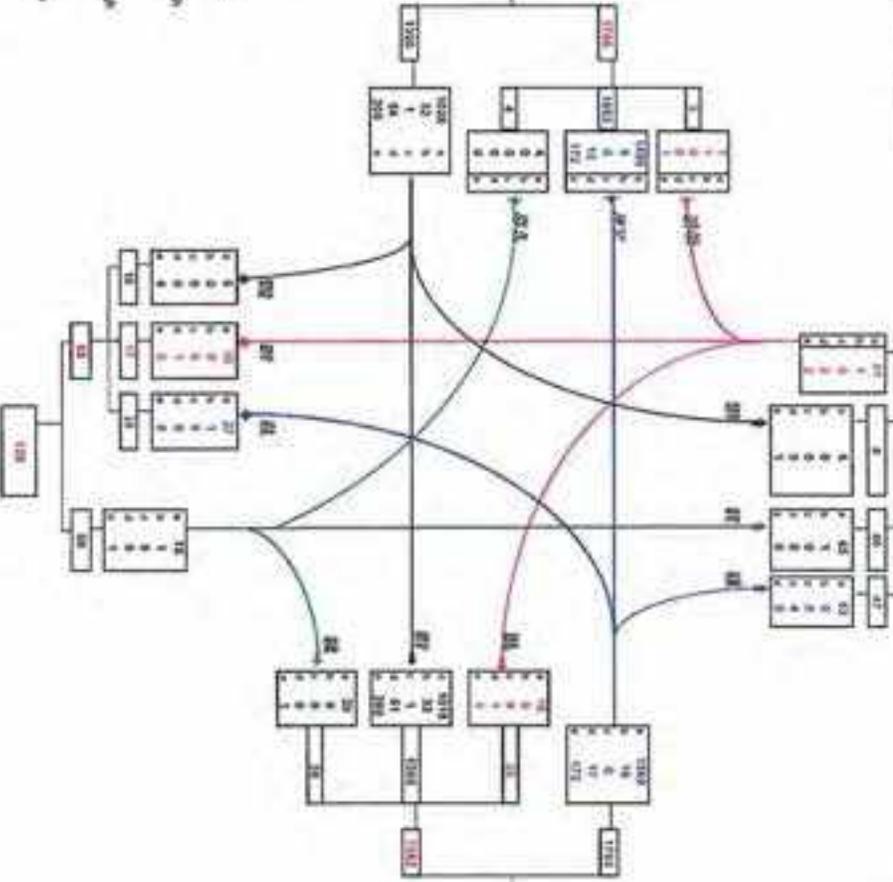
**2021 COMBINED
PM PEAK HOURLY VOLUME**

INTERSECTION		No. of Lanes	
Approach Type	Vol.	%	SA
Through Lane	122	80.3	
Left Turn	2	1.4	
Right Turn	6	4.0	
Other	0	0.0	
Total Volume	150	100	



INTERSECTION		No. of Lanes	
Approach Type	Vol.	%	SA
Through Lane	202	82.8	
Left Turn	1	0.4	
Right Turn	17	6.8	
Other	0	0.0	
Total Volume	240	100	

INTERSECTION		No. of Lanes	
Approach Type	Vol.	%	SA
Through Lane	122	80.3	
Left Turn	2	1.4	
Right Turn	6	4.0	
Other	0	0.0	
Total Volume	150	100	



- TURNING MOVEMENT ABBREVIATIONS**
- TR: Traffic from North Turning Right
 - TL: Traffic from North Turning Left
 - TR: Traffic from South Turning Right
 - TL: Traffic from South Turning Left
 - TR: Traffic from East Turning Right
 - TL: Traffic from East Turning Left
 - TR: Traffic from West Turning Right
 - TL: Traffic from West Turning Left

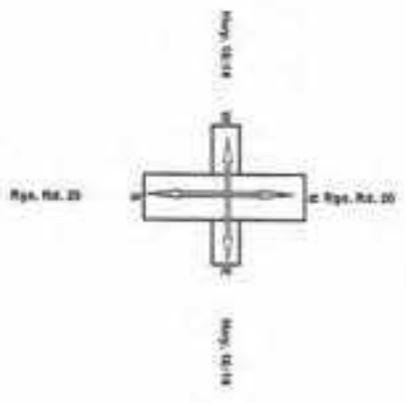
PERCENTAGE ABBREVIATIONS

- SA: Signalized Approach
- AS: Average Annual Daily Traffic
- AD: Average Daily Traffic
- AS: Average Annual Daily Traffic
- AD: Average Daily Traffic

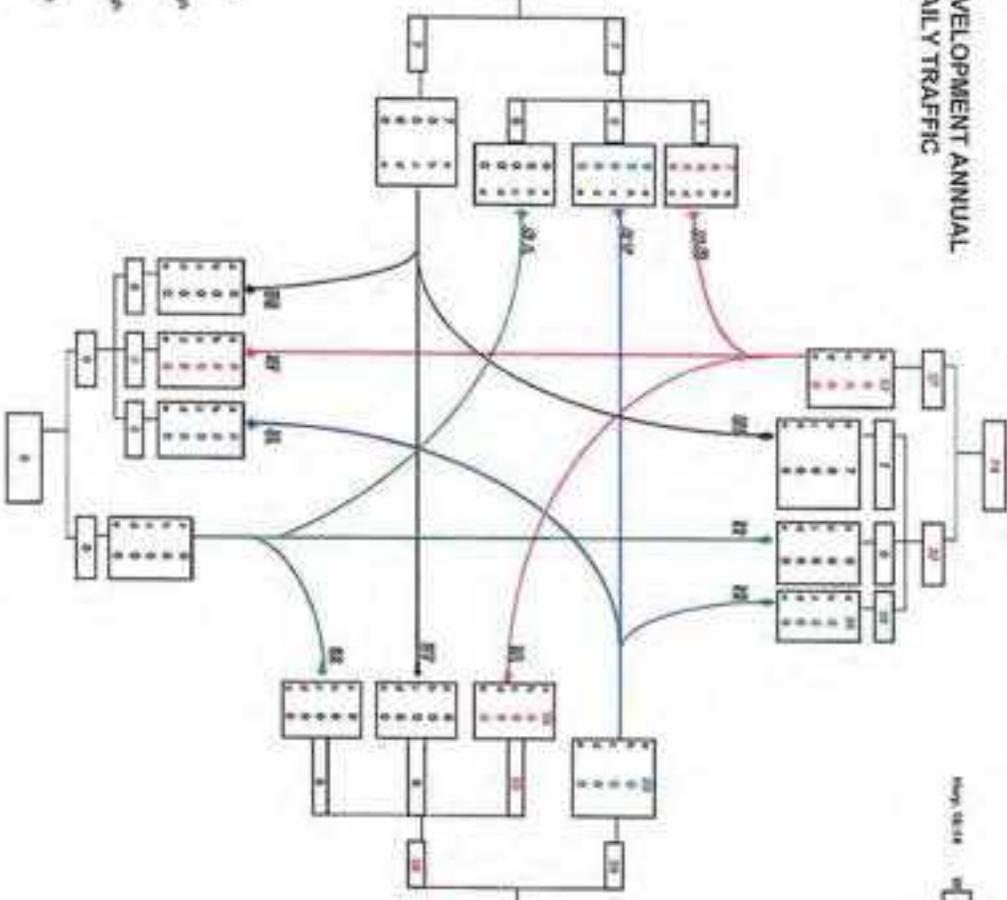
INTERSECTION		No. of Lanes	
Approach Type	Vol.	%	SA
Through Lane	122	80.3	
Left Turn	2	1.4	
Right Turn	6	4.0	
Other	0	0.0	
Total Volume	150	100	

2026 PROJECTED DEVELOPMENT ANNUAL AVERAGE DAILY TRAFFIC

2026 DEVELOPMENT		Avg. ADT	
Vehicle Type	Vol.	%	ADT
Commercial	10	0.2%	36
Industrial	10	0.2%	36
Residential	500	9.6%	1800
Office	100	1.9%	360
Public Use	10	0.2%	36
Other	10	0.2%	36
Total	540	10.3%	1884



2026 DEVELOPMENT		Avg. ADT	
Vehicle Type	Vol.	%	ADT
Commercial	10	0.2%	36
Industrial	10	0.2%	36
Residential	500	9.6%	1800
Office	100	1.9%	360
Public Use	10	0.2%	36
Other	10	0.2%	36
Total	540	10.3%	1884



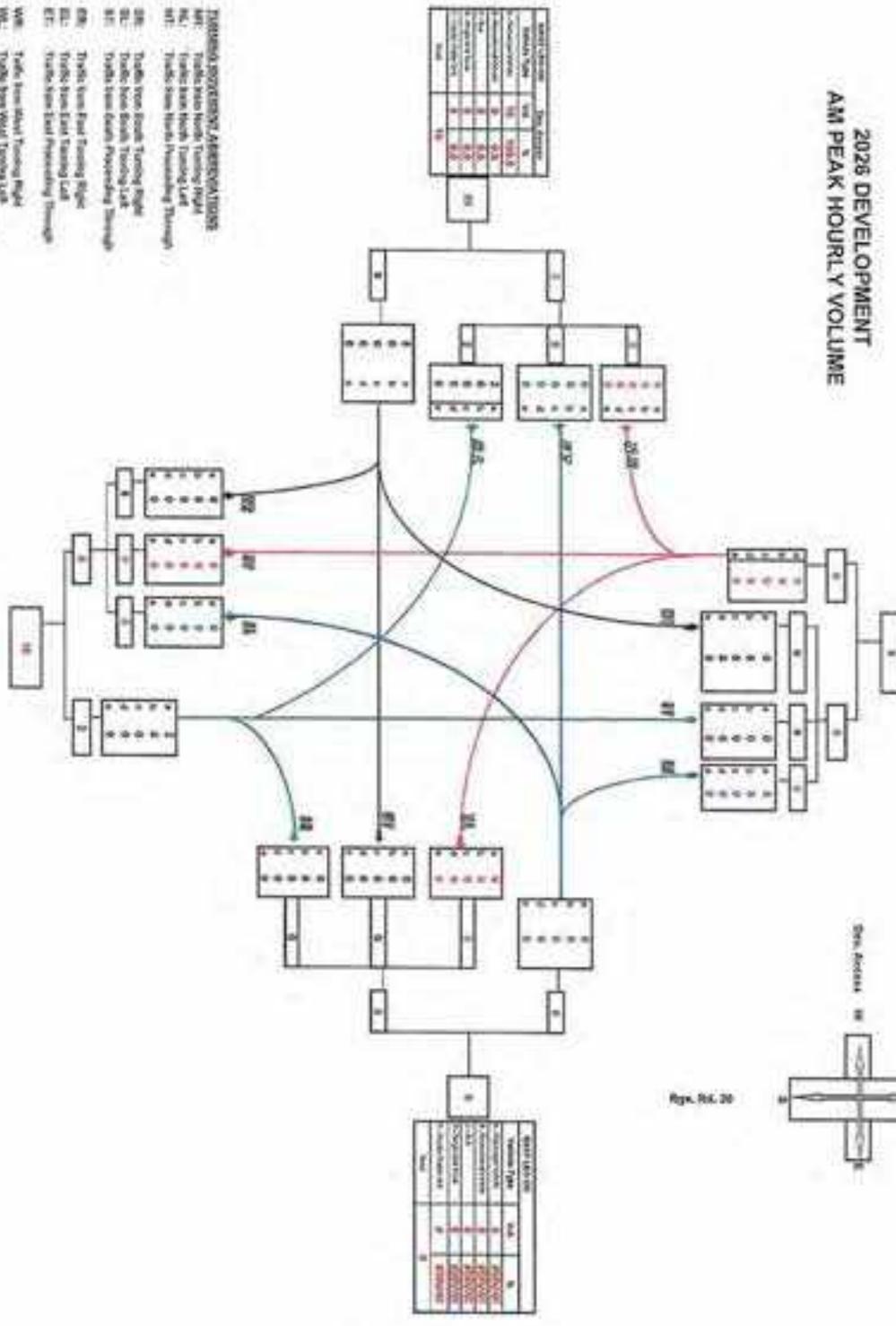
2026 DEVELOPMENT		Avg. ADT	
Vehicle Type	Vol.	%	ADT
Commercial	10	0.2%	36
Industrial	10	0.2%	36
Residential	500	9.6%	1800
Office	100	1.9%	360
Public Use	10	0.2%	36
Other	10	0.2%	36
Total	540	10.3%	1884

- TURNING MOVEMENT ABBREVIATIONS:**
- RT - Traffic from North Turning Right
 - LT - Traffic from North Turning Left
 - BT - Traffic from North Proceeding Through
 - RL - Traffic from South Turning Right
 - LL - Traffic from South Turning Left
 - BS - Traffic from South Proceeding Through
 - RR - Traffic from East Turning Right
 - LR - Traffic from East Turning Left
 - BE - Traffic from East Proceeding Through
 - WR - Traffic from West Turning Right
 - WL - Traffic from West Turning Left
 - WE - Traffic from West Proceeding Through
- LEGEND:**
- ADT - Average Annual Daily Traffic
 - ADT - Average daily traffic expressed as vehicles per day for period of January 1 to December 31, 2026 (ADT)
 - ADT - Average Annual Daily Traffic
 - ADT - Average daily traffic expressed as vehicles per day for period of May 1 to September 30, 2026 (ADT)

2026 DEVELOPMENT		Avg. ADT	
Vehicle Type	Vol.	%	ADT
Commercial	10	0.2%	36
Industrial	10	0.2%	36
Residential	500	9.6%	1800
Office	100	1.9%	360
Public Use	10	0.2%	36
Other	10	0.2%	36
Total	540	10.3%	1884

2026 DEVELOPMENT
AM PEAK HOURLY VOLUME

MOVEMENT	NO. VEH/HR
THRU	100
THRU (LEFT)	100
THRU (RIGHT)	100
THRU (LEFT)	100
THRU (RIGHT)	100
THRU (LEFT)	100
THRU (RIGHT)	100
THRU (LEFT)	100
THRU (RIGHT)	100



MOVEMENT	NO. VEH/HR
THRU	100
THRU (LEFT)	100
THRU (RIGHT)	100
THRU (LEFT)	100
THRU (RIGHT)	100
THRU (LEFT)	100
THRU (RIGHT)	100
THRU (LEFT)	100
THRU (RIGHT)	100

MOVEMENT	NO. VEH/HR
THRU	100
THRU (LEFT)	100
THRU (RIGHT)	100
THRU (LEFT)	100
THRU (RIGHT)	100
THRU (LEFT)	100
THRU (RIGHT)	100
THRU (LEFT)	100
THRU (RIGHT)	100

- TURNING MOVEMENT ABBREVIATIONS**
- MT: Thru Move North Turning Right
 - ML: Thru Move North Turning Left
 - MT: Thru Move North Forwarding Through
 - MR: Thru Move South Turning Right
 - ML: Thru Move South Turning Left
 - MT: Thru Move South Forwarding Through
 - MR: Thru Move East Turning Right
 - ML: Thru Move East Turning Left
 - MT: Thru Move East Forwarding Through
 - MR: Thru Move West Turning Right
 - ML: Thru Move West Turning Left
 - MT: Thru Move West Forwarding Through

ADDITIONAL INFORMATION

AD001: Average Annual Daily Traffic

AD002: Average Daily Traffic

AD003: Average Daily Traffic

AD004: Average Daily Traffic

AD005: Average Daily Traffic

AD006: Average Daily Traffic

AD007: Average Daily Traffic

AD008: Average Daily Traffic

AD009: Average Daily Traffic

AD010: Average Daily Traffic

MOVEMENT	NO. VEH/HR
THRU	100
THRU (LEFT)	100
THRU (RIGHT)	100
THRU (LEFT)	100
THRU (RIGHT)	100
THRU (LEFT)	100
THRU (RIGHT)	100
THRU (LEFT)	100
THRU (RIGHT)	100

**2026 DEVELOPMENT
PM PEAK HOURLY VOLUME**

WEST LANE 101				
MOVEMENT	TRAFFIC TYPE	WAY	WAY NO.	%
Left Turn	Left	West	101	10.0
Through	Left	West	101	10.0
Right Turn	Left	West	101	10.0
Left Turn	Right	West	101	10.0
Through	Right	West	101	10.0
Right Turn	Right	West	101	10.0
TOTAL				60.0

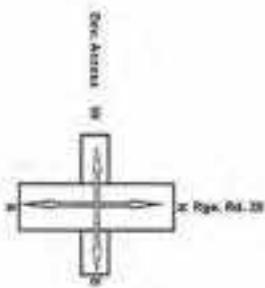
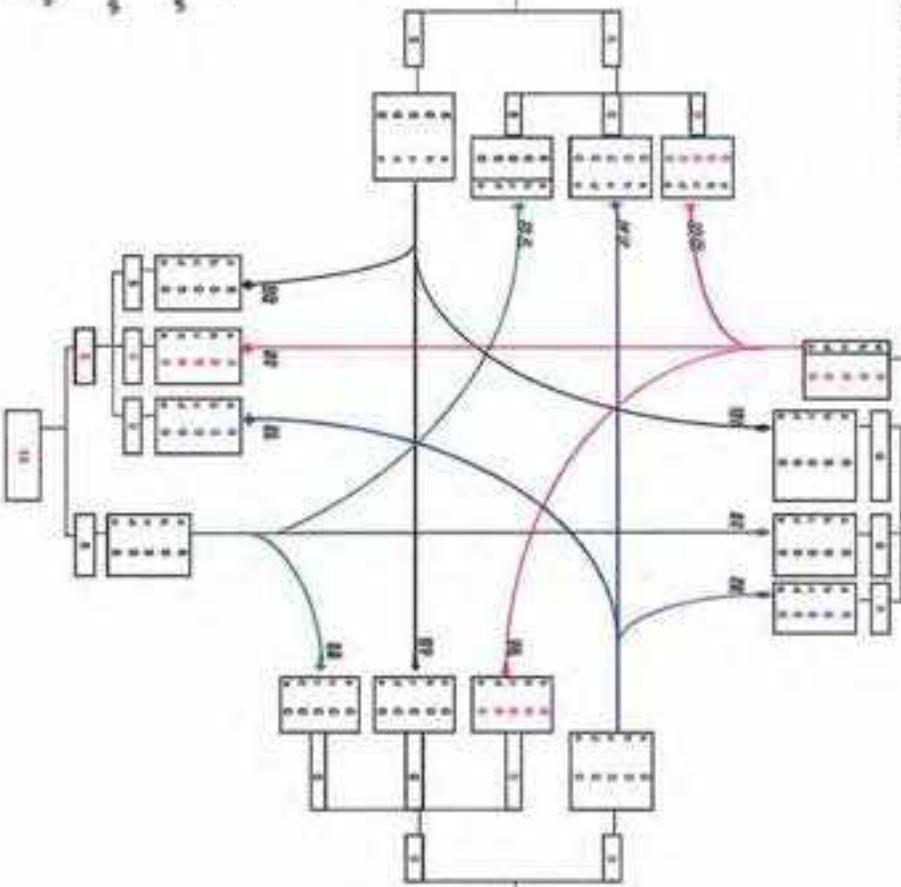


Fig. Rd. 20

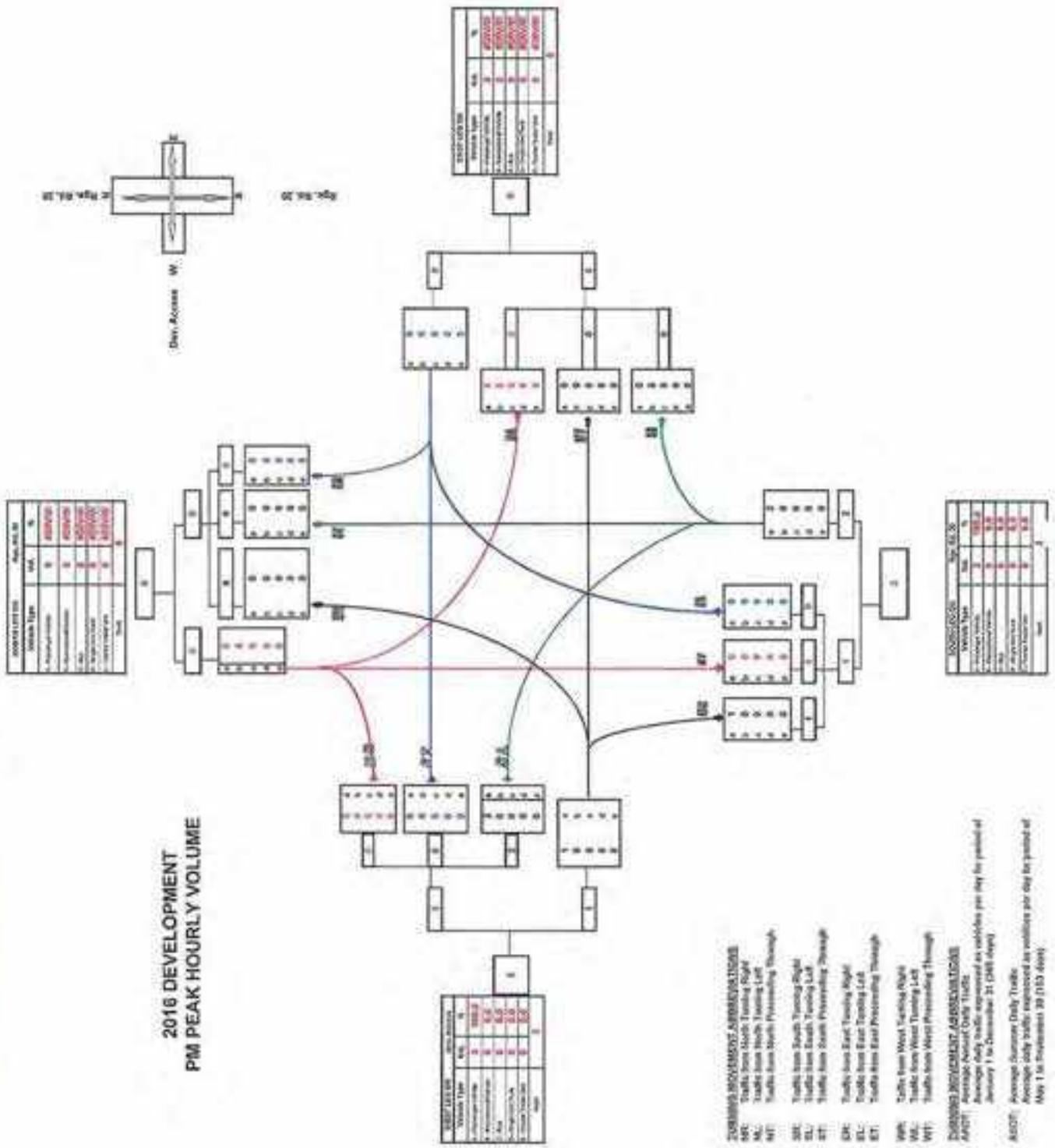
WEST LANE 101				
TRAFFIC TYPE	WAY	WAY NO.	WAY	%
Left Turn	Left	West	101	10.0
Through	Left	West	101	10.0
Right Turn	Left	West	101	10.0
Left Turn	Right	West	101	10.0
Through	Right	West	101	10.0
Right Turn	Right	West	101	10.0
TOTAL				60.0



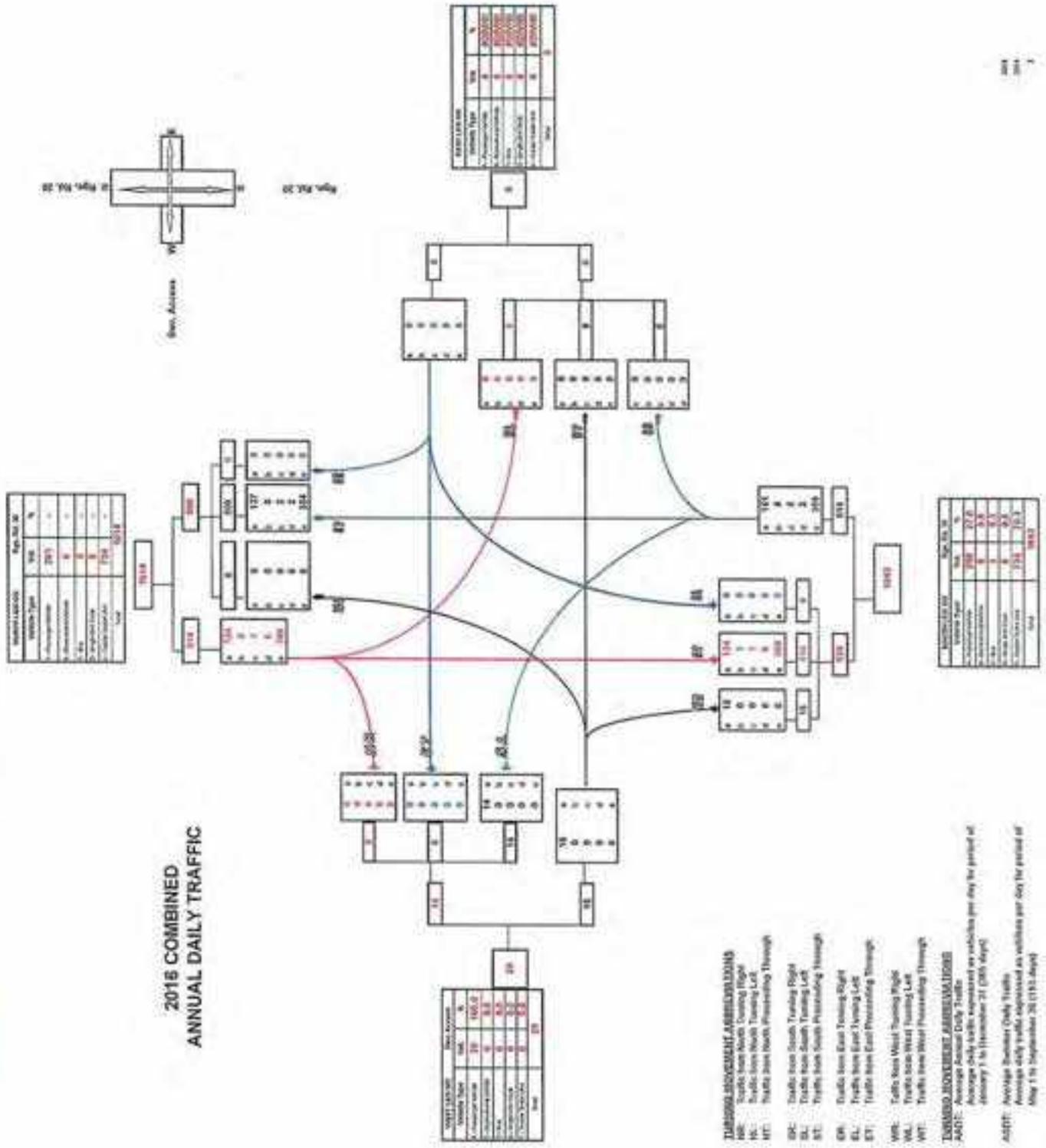
WEST LANE 101				
TRAFFIC TYPE	WAY	WAY NO.	WAY	%
Left Turn	Left	West	101	10.0
Through	Left	West	101	10.0
Right Turn	Left	West	101	10.0
Left Turn	Right	West	101	10.0
Through	Right	West	101	10.0
Right Turn	Right	West	101	10.0
TOTAL				60.0

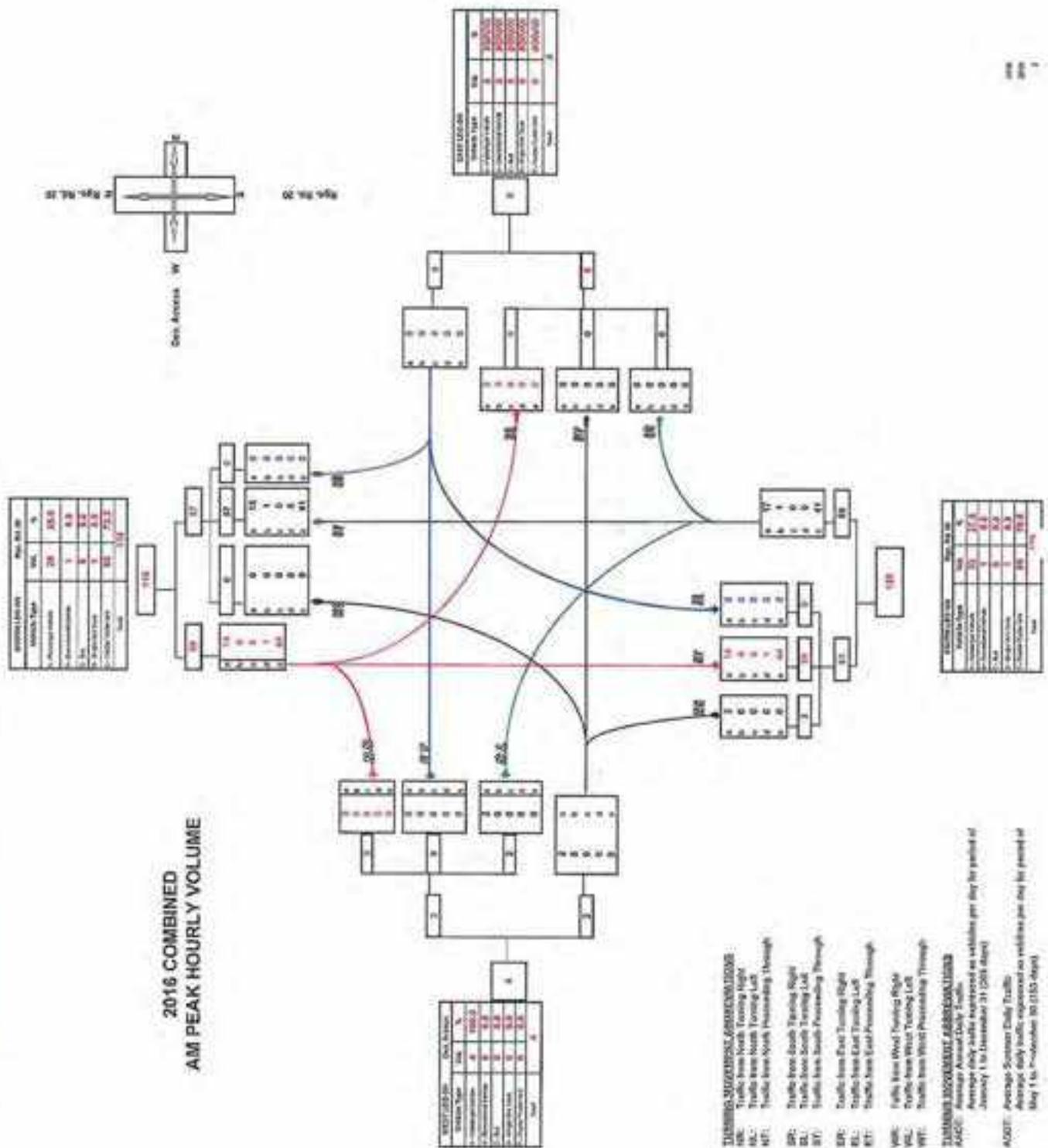
- TURNING MOVEMENT ABBREVIATIONS**
- LT - Turns from South Turning Right
 - TR - Turns from South Turning Left
 - TL - Turns from South Proceeding Through
 - TR - Turns from South Turning Right
 - TL - Turns from South Turning Left
 - TL - Turns from South Proceeding Through
 - LR - Turns from East Turning Right
 - LR - Turns from East Turning Left
 - LR - Turns from East Proceeding Through
 - LR - Turns from West Turning Right
 - LR - Turns from West Turning Left
 - LR - Turns from West Proceeding Through
 - WR - Turns from West Proceeding Through
- TURNING MOVEMENT ABBREVIATIONS**
- ASLT - Average only vehicle converted to vehicles per day per period of analysis 15 to 30 minutes 21 days rolling
 - ASRT - Average only vehicle converted to vehicles per day per period of analysis 15 to 30 minutes 21 days rolling
 - ASPT - Average only vehicle converted to vehicles per day per period of analysis 15 to 30 minutes 21 days rolling

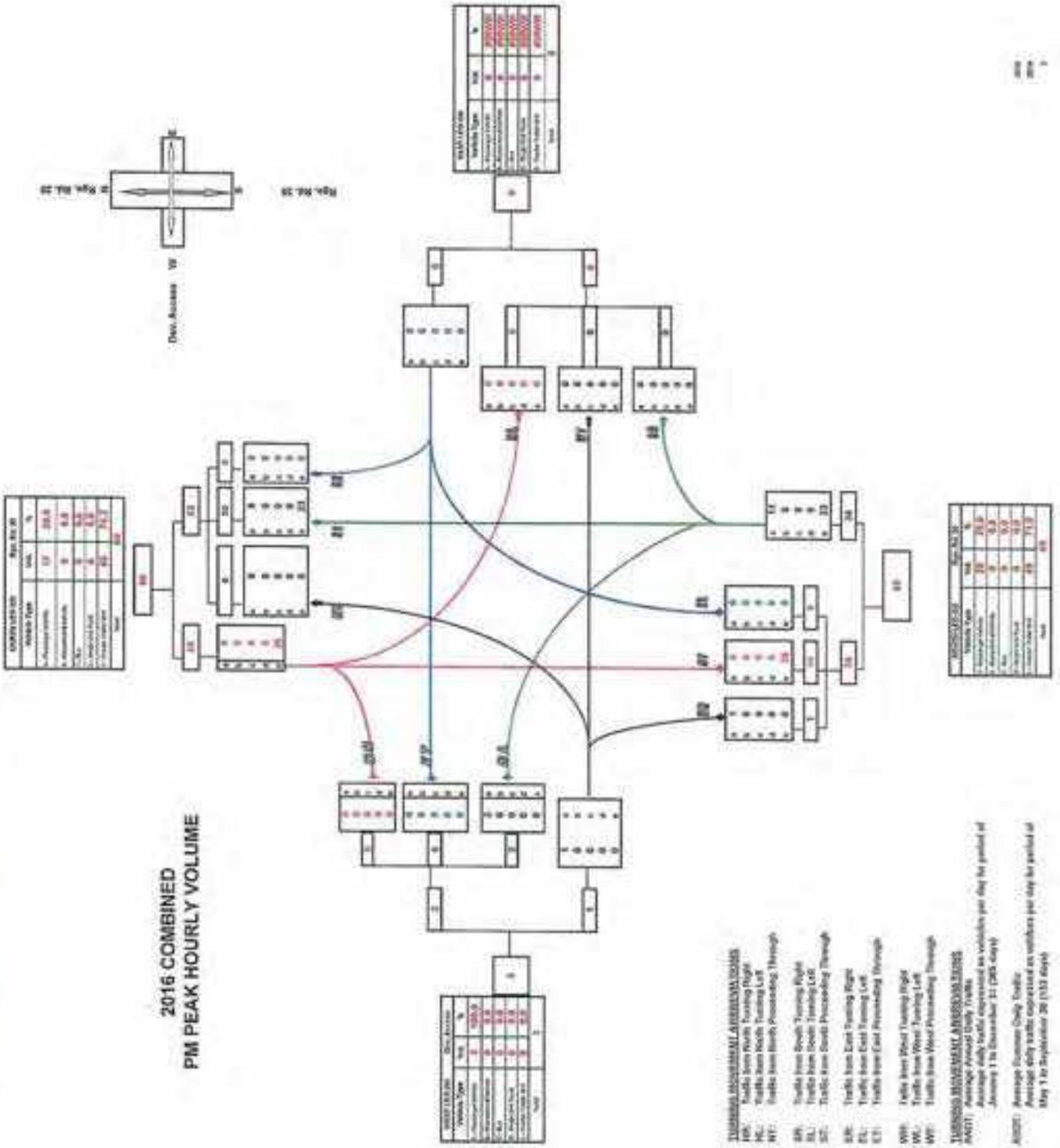
WEST LANE 101				
TRAFFIC TYPE	WAY	WAY NO.	WAY	%
Left Turn	Left	West	101	10.0
Through	Left	West	101	10.0
Right Turn	Left	West	101	10.0
Left Turn	Right	West	101	10.0
Through	Right	West	101	10.0
Right Turn	Right	West	101	10.0
TOTAL				60.0

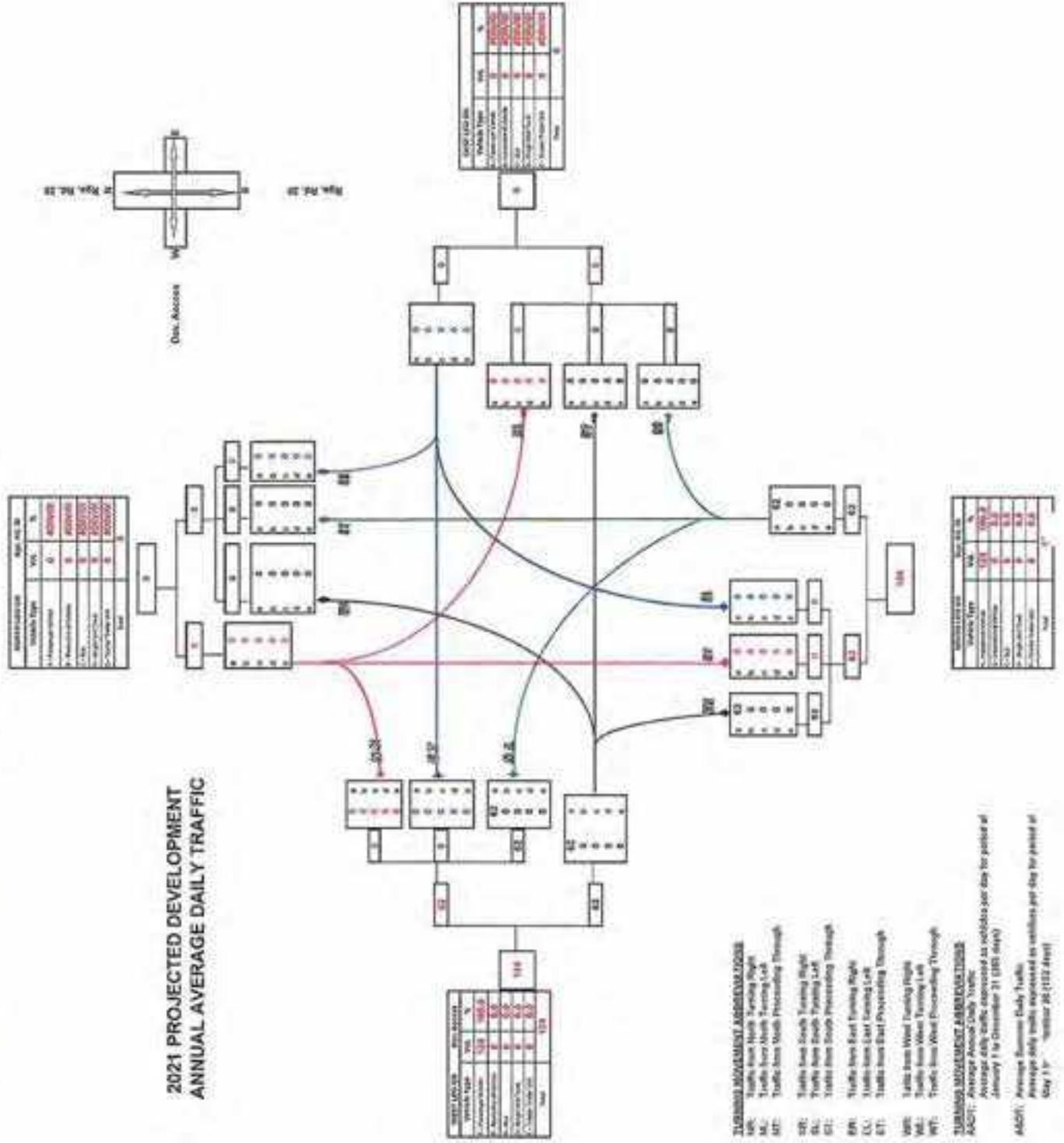


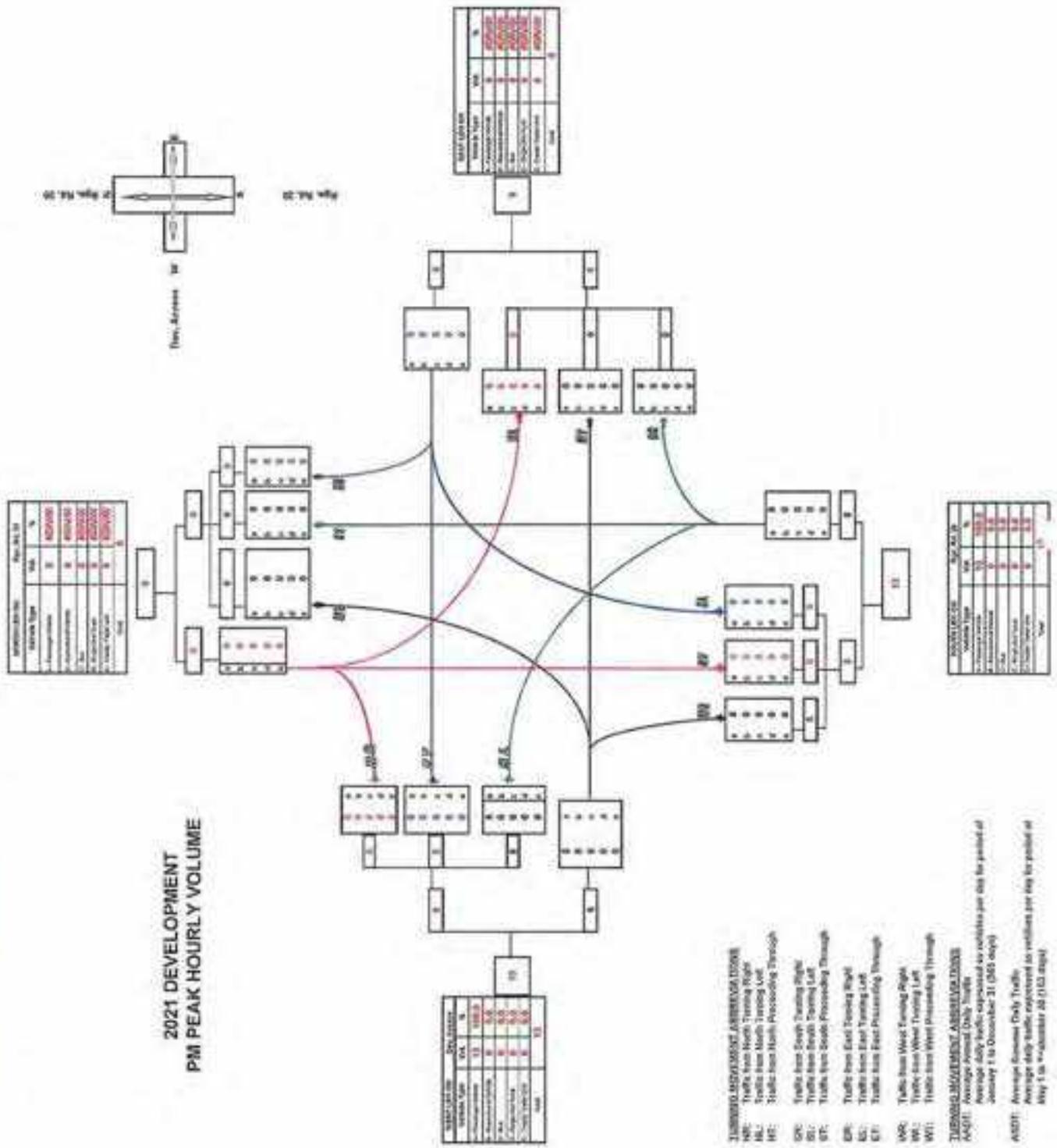
Turning Movement Summary Diagram



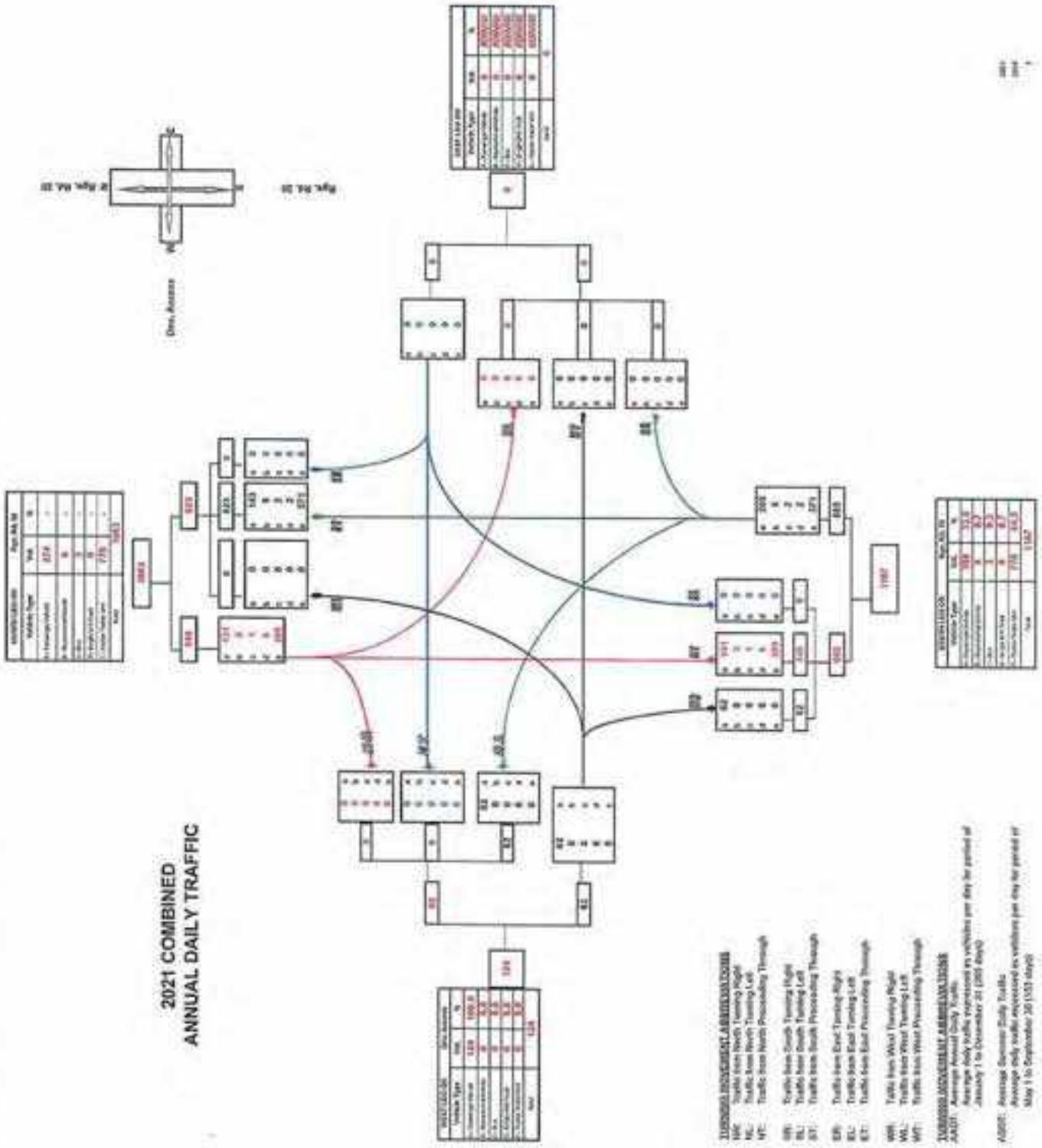


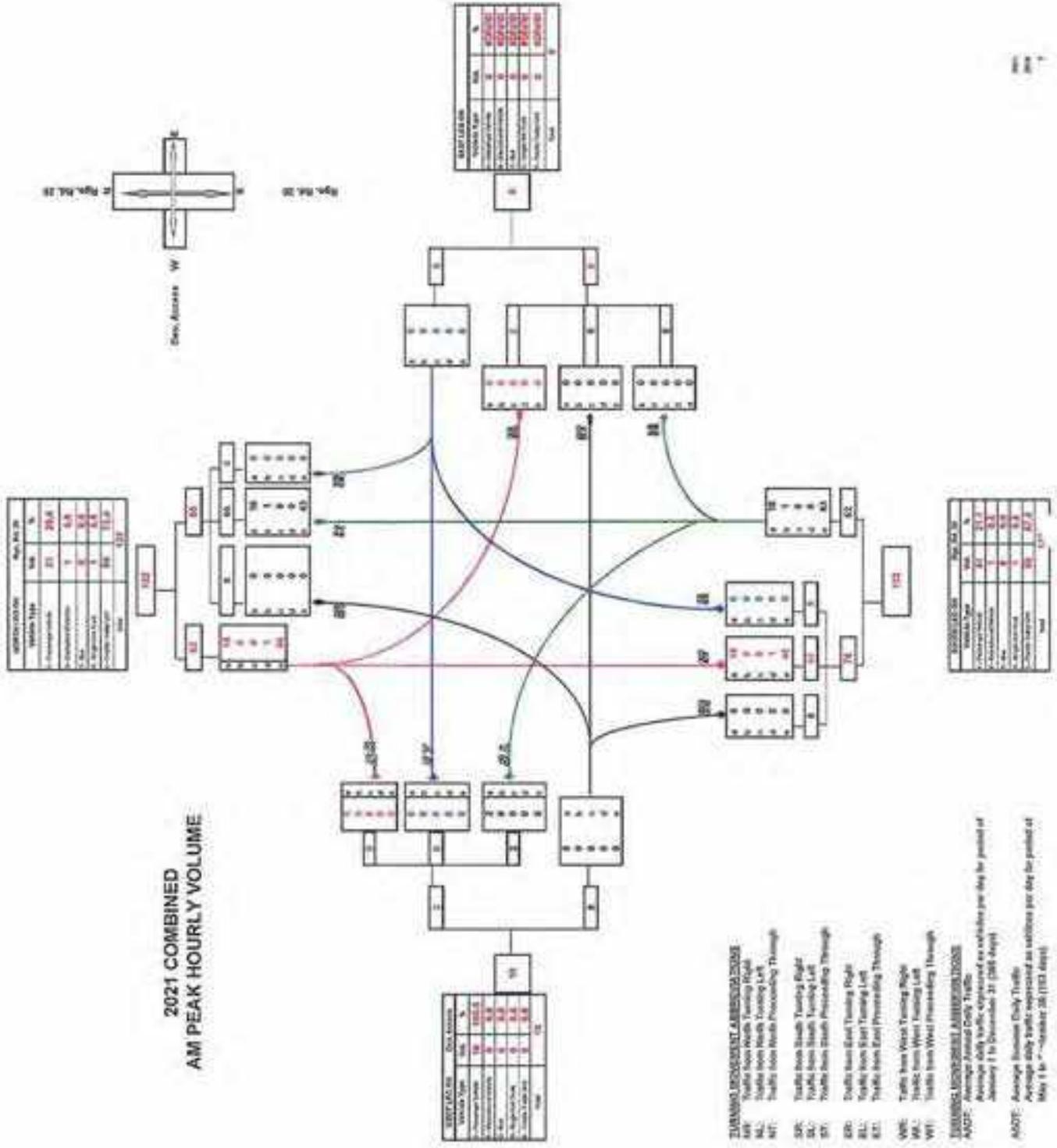






Turning Movement Summary Diagram





APPENDIX D – Hwy. 43 & Twp. Rd. 540
TURNING MOVEMENT DIAGRAMS
SYNCHRO ANALYSIS
ILLUMINATION WARRANT

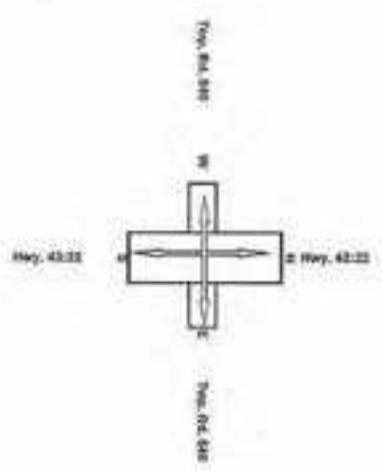
TRAFFIC IMPACT ASSESSMENT

LAUNCH VENTURES RESIDENTIAL
SE 1-54-2-W5M
Loc Site, Anne County
TR 540, Hwy. 43:22, RR 20, Hwy. 16:14

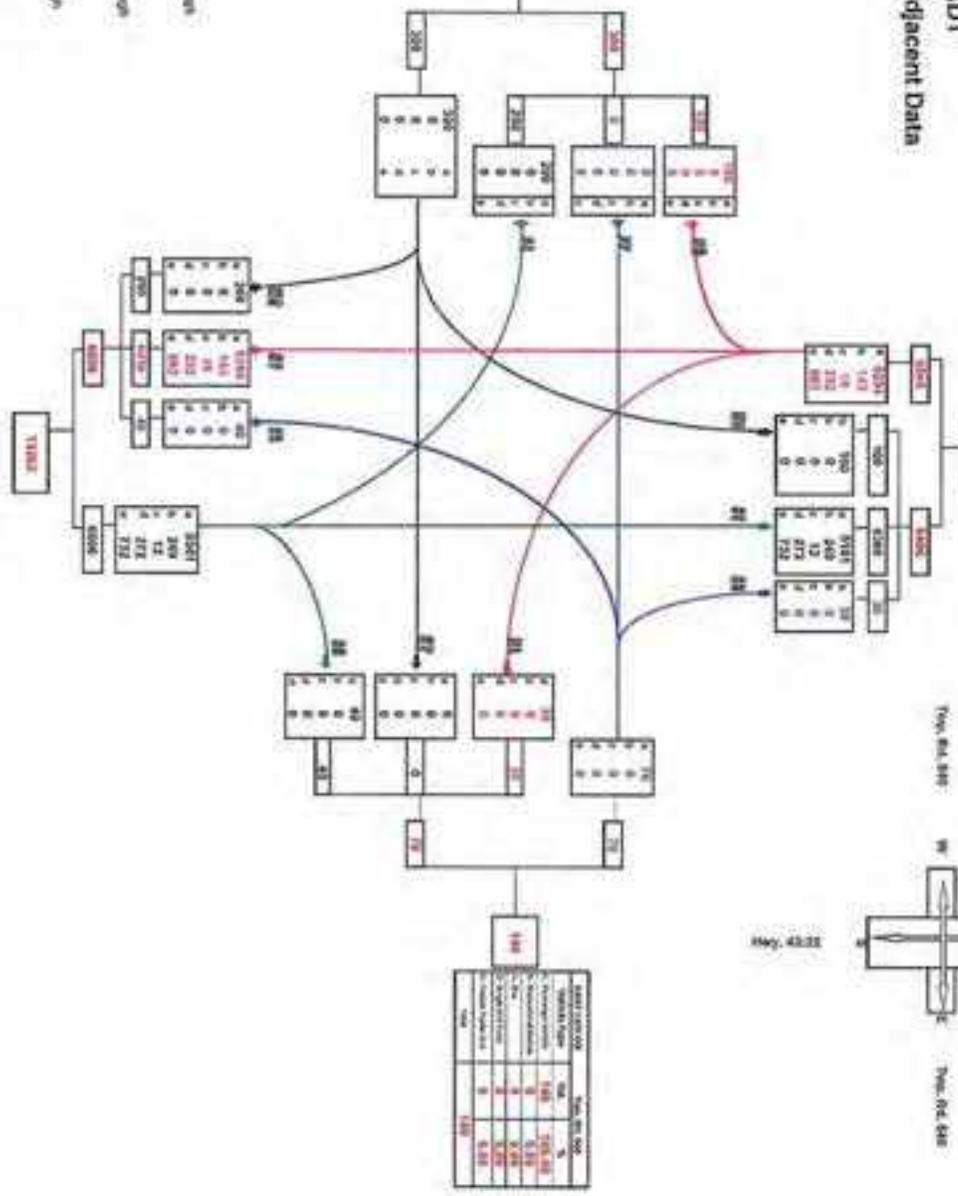
Intersection of: Highway 4222 & Hwy. 64, 648

2014 AADT
Estimated from Adjacent Data

ADJACENT DATA		Avg. Daily	
Location	Year	ADT	%
Highway 4222	2013	5000	82.7%
Highway 64	2013	500	8.3%
Highway 4222	2014	5000	82.7%
Highway 64	2014	500	8.3%
Total		10000	100%



ADJACENT DATA		Avg. Daily	
Location	Year	ADT	%
Highway 4222	2013	5000	82.7%
Highway 64	2013	500	8.3%
Highway 4222	2014	5000	82.7%
Highway 64	2014	500	8.3%
Total		10000	100%



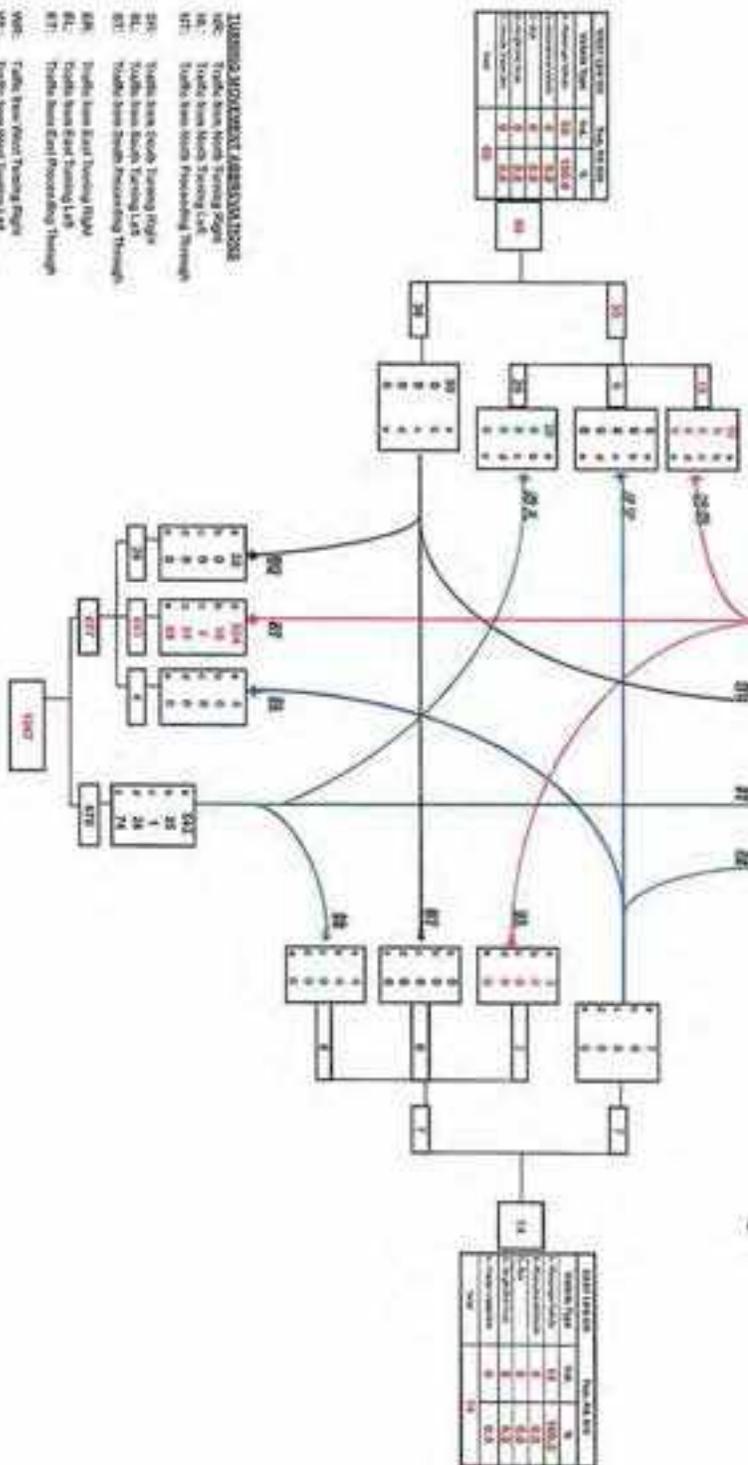
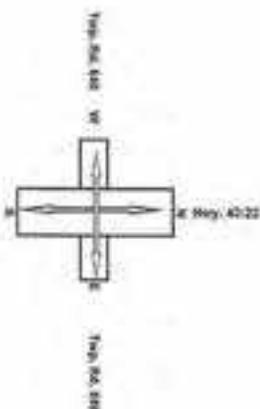
ADJACENT DATA		Avg. Daily	
Location	Year	ADT	%
Highway 4222	2013	5000	82.7%
Highway 64	2013	500	8.3%
Highway 4222	2014	5000	82.7%
Highway 64	2014	500	8.3%
Total		10000	100%

- RT: Traffic from North Turning Right
- LT: Traffic from North Turning Left
- TH: Traffic from North Proceeding Through
- RT: Traffic from South Turning Right
- LT: Traffic from South Turning Left
- TH: Traffic from South Proceeding Through
- RT: Traffic from East Turning Right
- LT: Traffic from East Turning Left
- TH: Traffic from East Proceeding Through
- RT: Traffic from West Turning Right
- LT: Traffic from West Turning Left
- TH: Traffic from West Proceeding Through

ADJACENT DATA		Avg. Daily	
Location	Year	ADT	%
Highway 4222	2013	5000	82.7%
Highway 64	2013	500	8.3%
Highway 4222	2014	5000	82.7%
Highway 64	2014	500	8.3%
Total		10000	100%

2014 AM PEAK HOURLY VOLUME

INTERSECTION		May 4/14	
Direction	Vol	%	%
Northbound	1200	50.0	60.0
Southbound	1000	40.0	50.0
Eastbound	1000	40.0	50.0
Westbound	1000	40.0	50.0
Total	4200	100.0	100.0



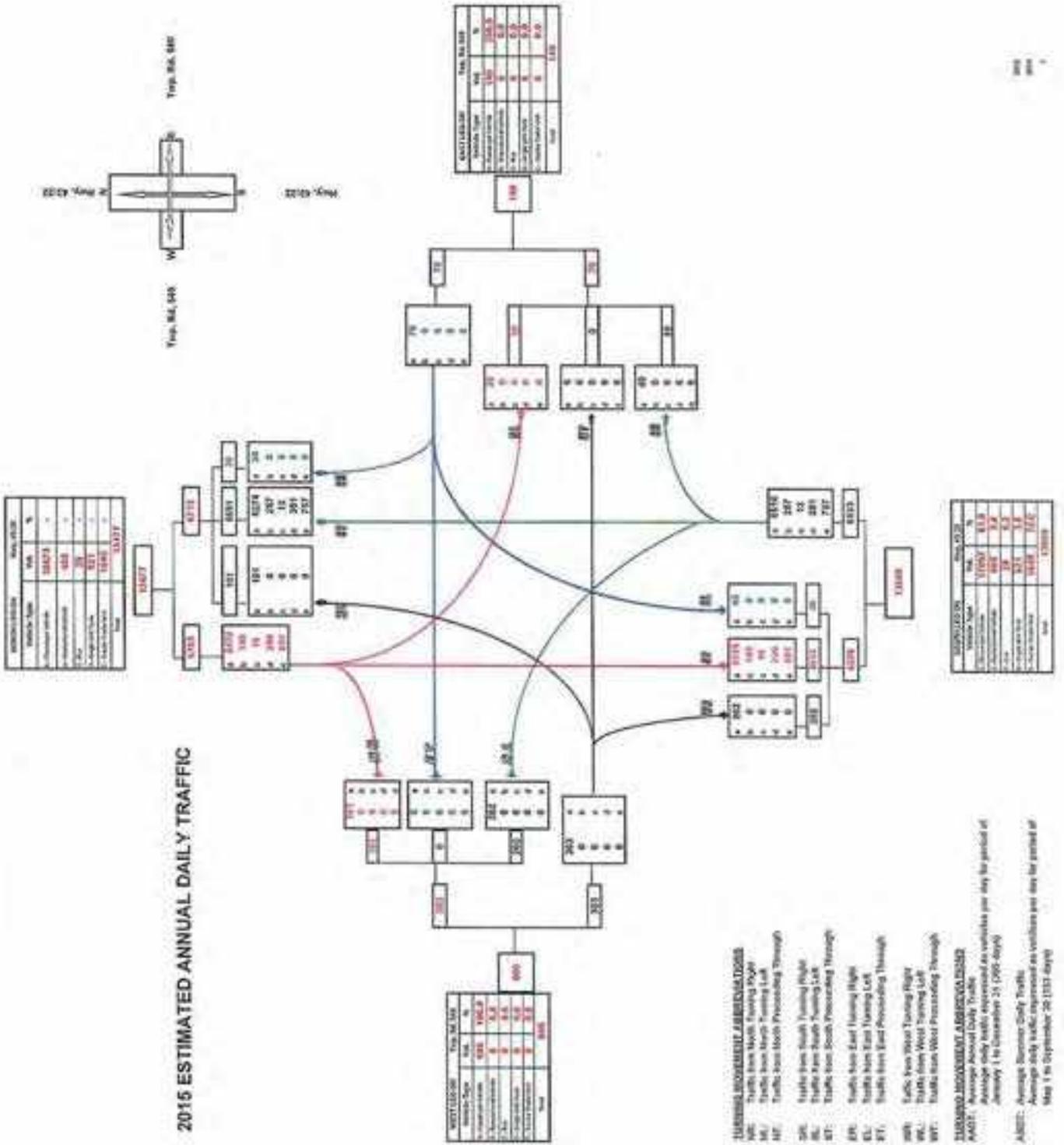
WEST LEGION		May 4/14	
Movement	Vol	%	%
Through	1000	40.0	50.0
Left	1000	40.0	50.0
Right	1000	40.0	50.0
Total	3000	100.0	100.0

EAST LEGION		May 4/14	
Movement	Vol	%	%
Through	1000	40.0	50.0
Left	1000	40.0	50.0
Right	1000	40.0	50.0
Total	3000	100.0	100.0

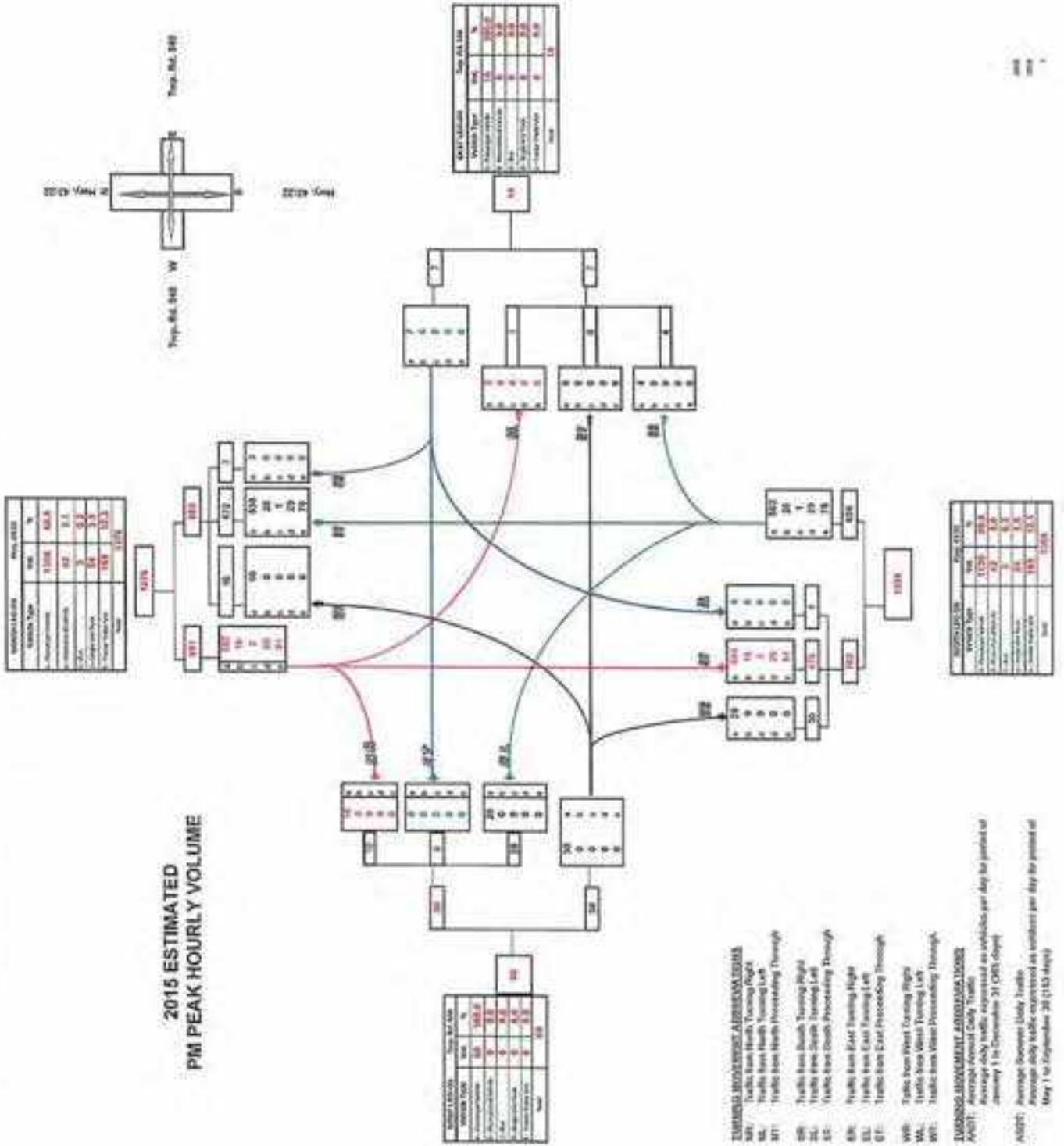
- TURNING MOVEMENT ABBREVIATIONS**
- THC: Thru from North Turning Right
 - TRC: Thru from North Turning Left
 - TRT: Thru from North Turning Through
 - TRL: Thru from South Turning Right
 - TRR: Thru from South Turning Left
 - TRT: Thru from South Turning Through
 - TRR: Thru from East Turning Right
 - TRR: Thru from East Turning Left
 - TRT: Thru from East Turning Through
 - TRR: Thru from West Turning Right
 - TRR: Thru from West Turning Left
 - TRT: Thru from West Turning Through

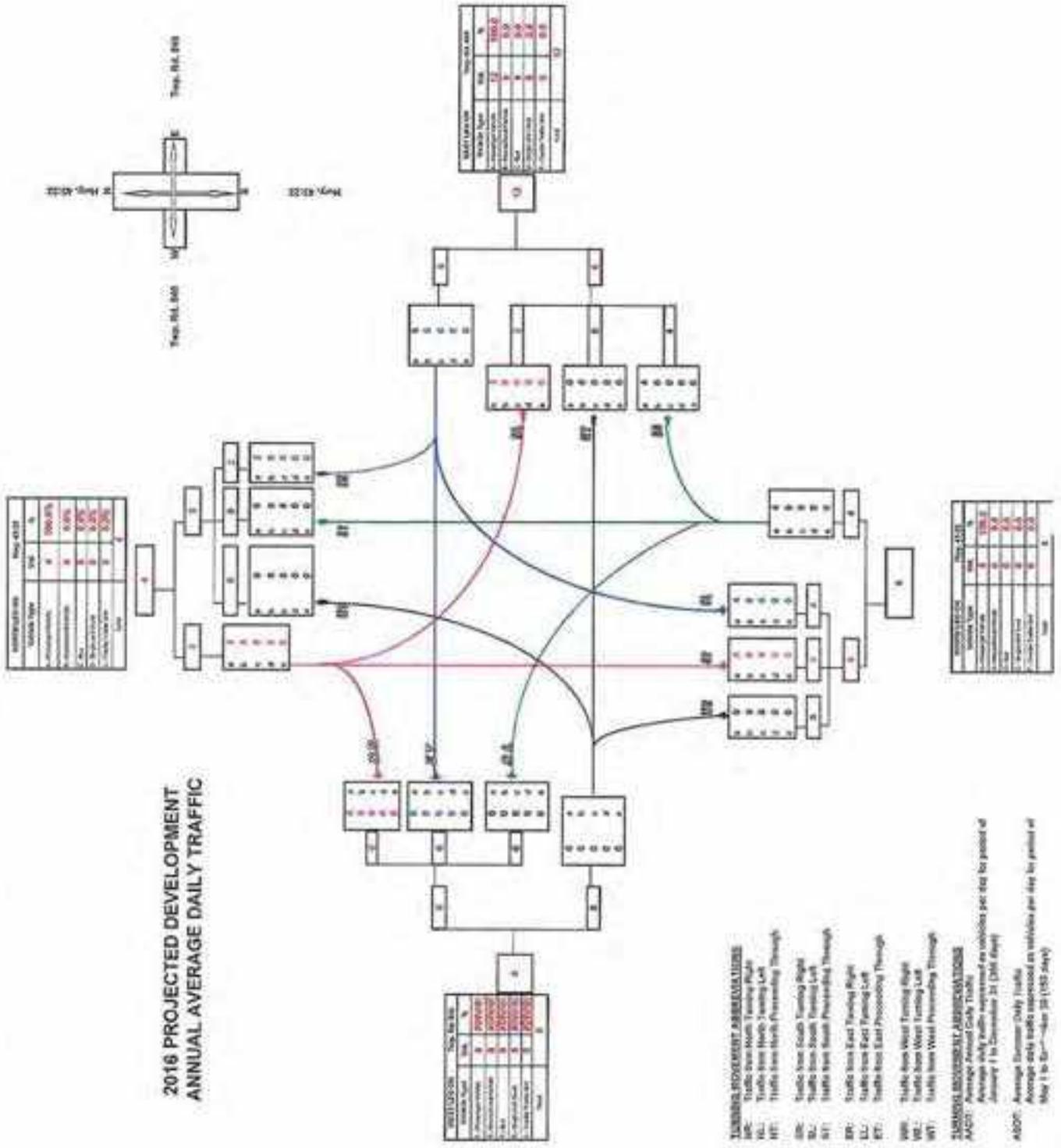
- TURNING MOVEMENT ABBREVIATIONS**
- AVLTC: Average Annual Daily Traffic
 - AVL: Average Daily Traffic, expressed as vehicles per day for period of January 1 to December 31 (2014 input)
 - AVLST: Average Summer Daily Traffic
 - AVLW: Average daily traffic, expressed as vehicles per day for period of May 1 to September 30 (2014 input)

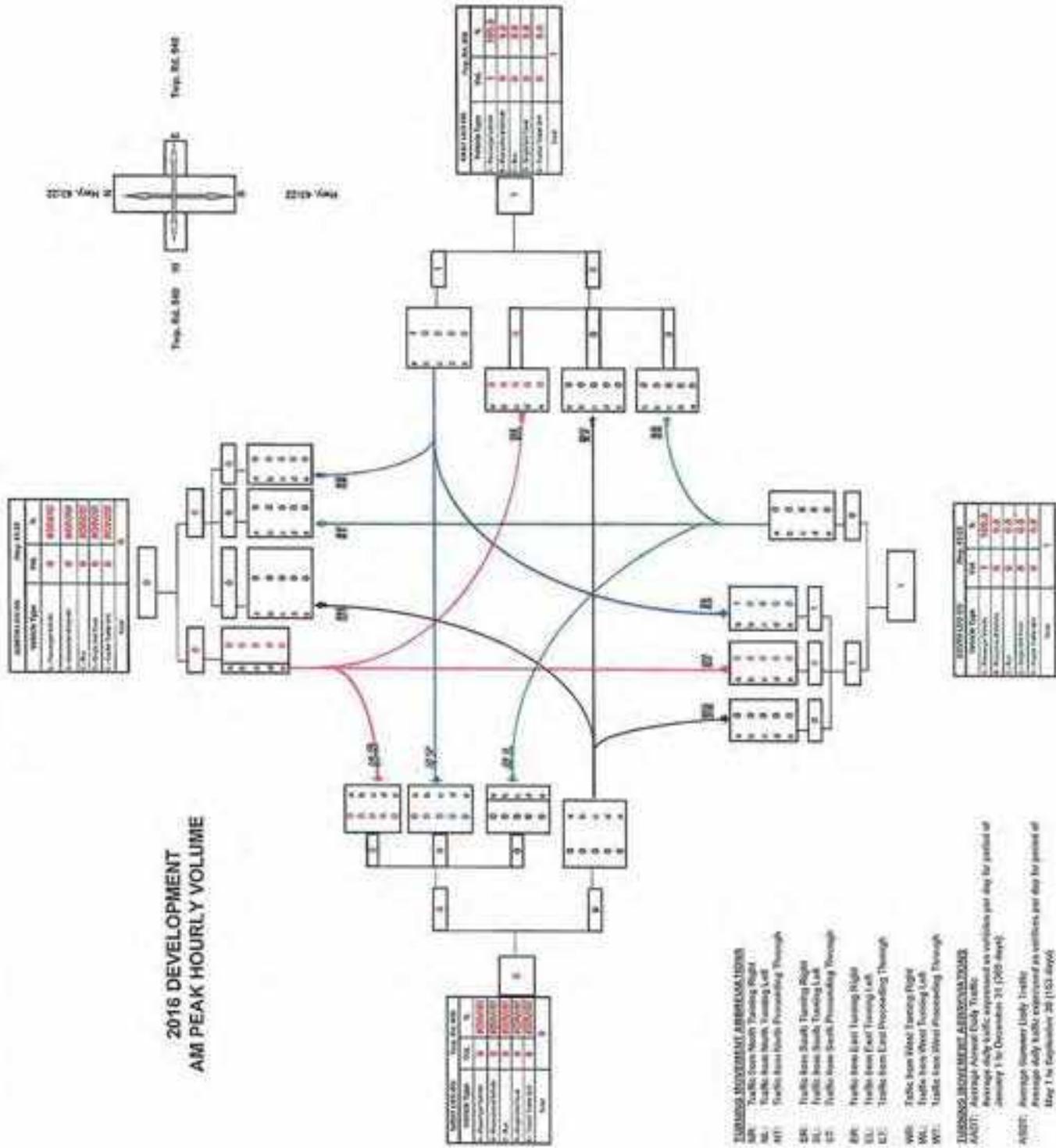
INTERSECTION		May 4/14	
Direction	Vol	%	%
Northbound	1200	50.0	60.0
Southbound	1000	40.0	50.0
Eastbound	1000	40.0	50.0
Westbound	1000	40.0	50.0
Total	4200	100.0	100.0

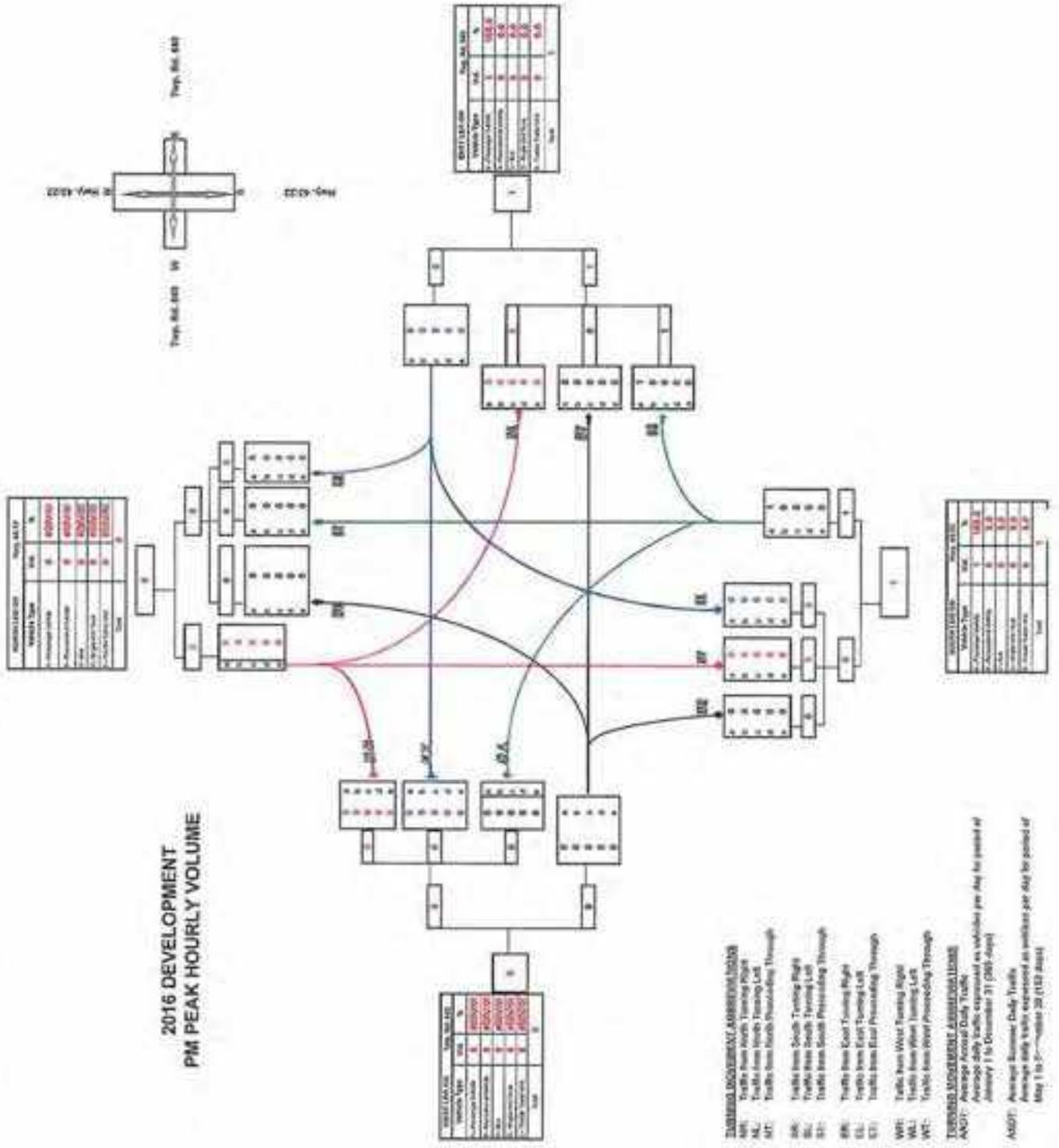


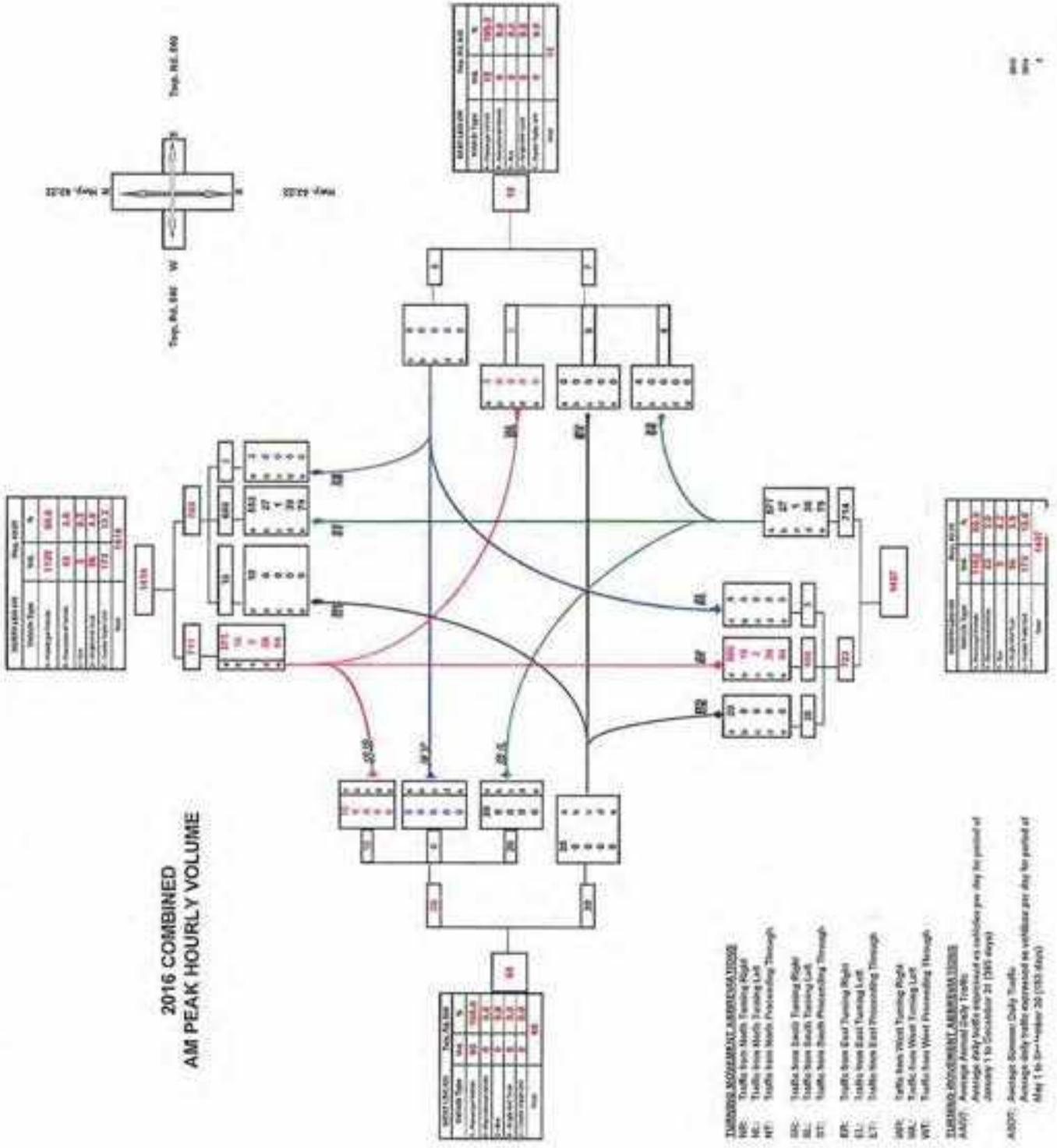
Turning Movement Summary Diagram



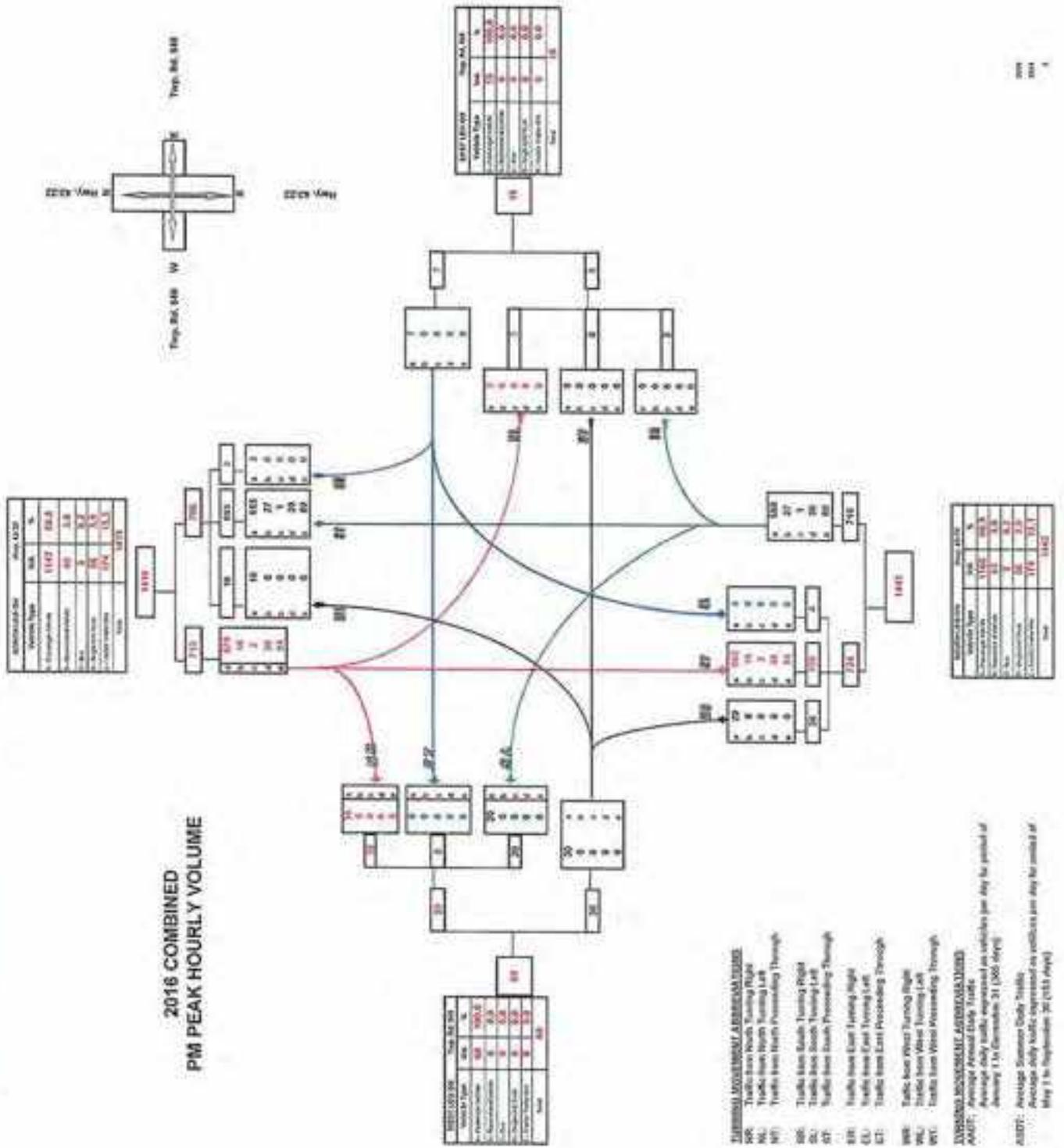




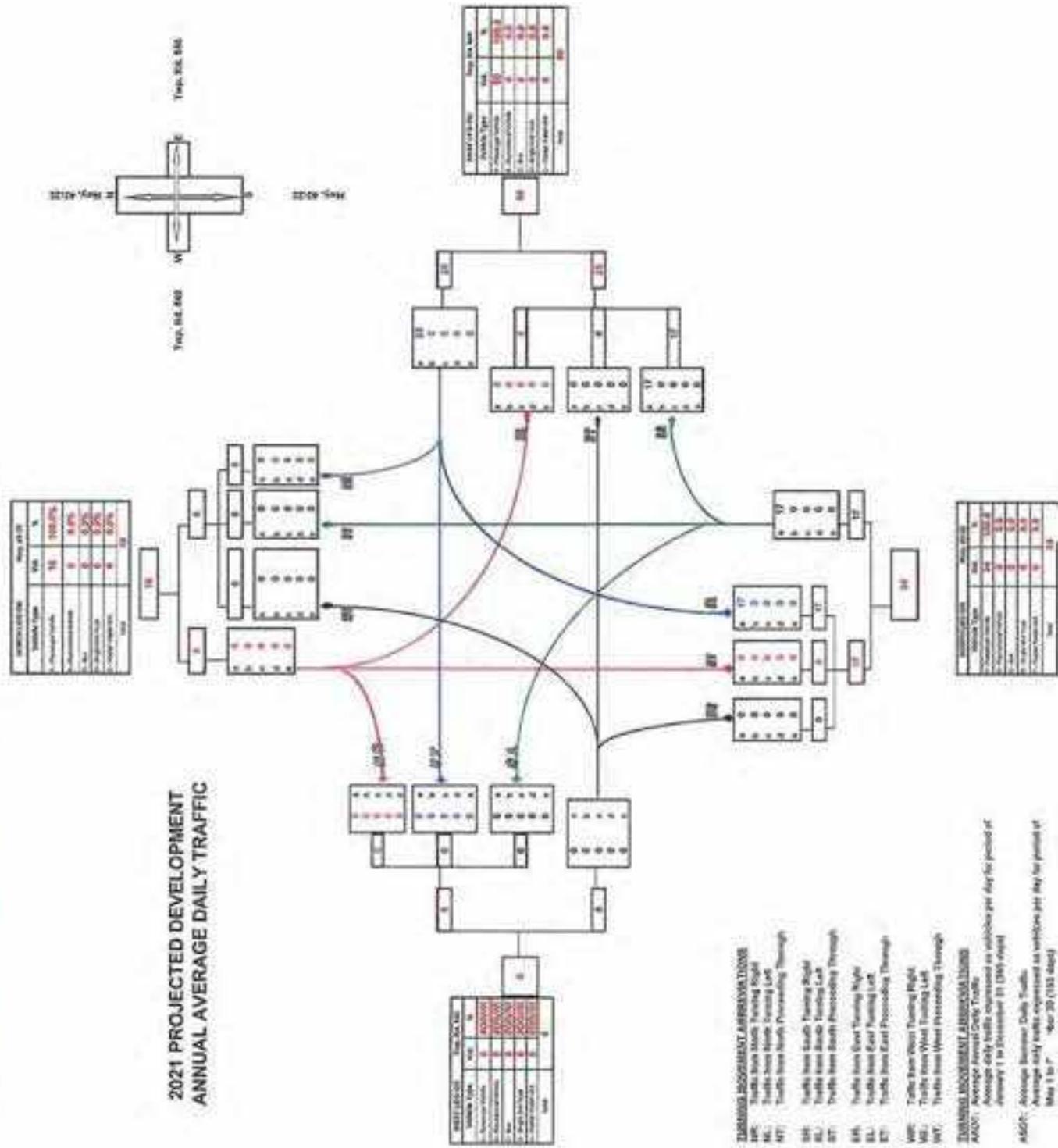




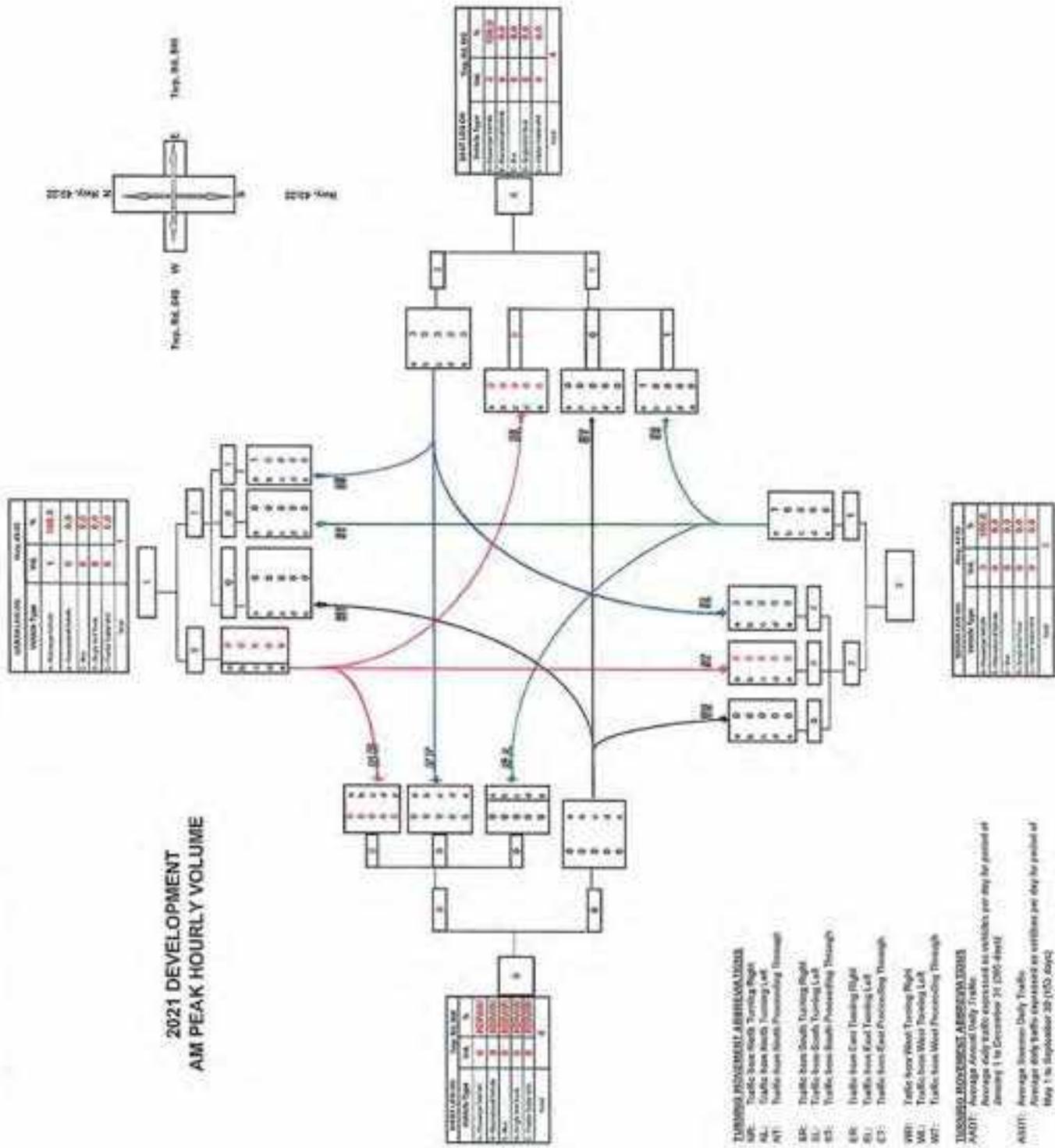
Turning Movement Summary Diagram

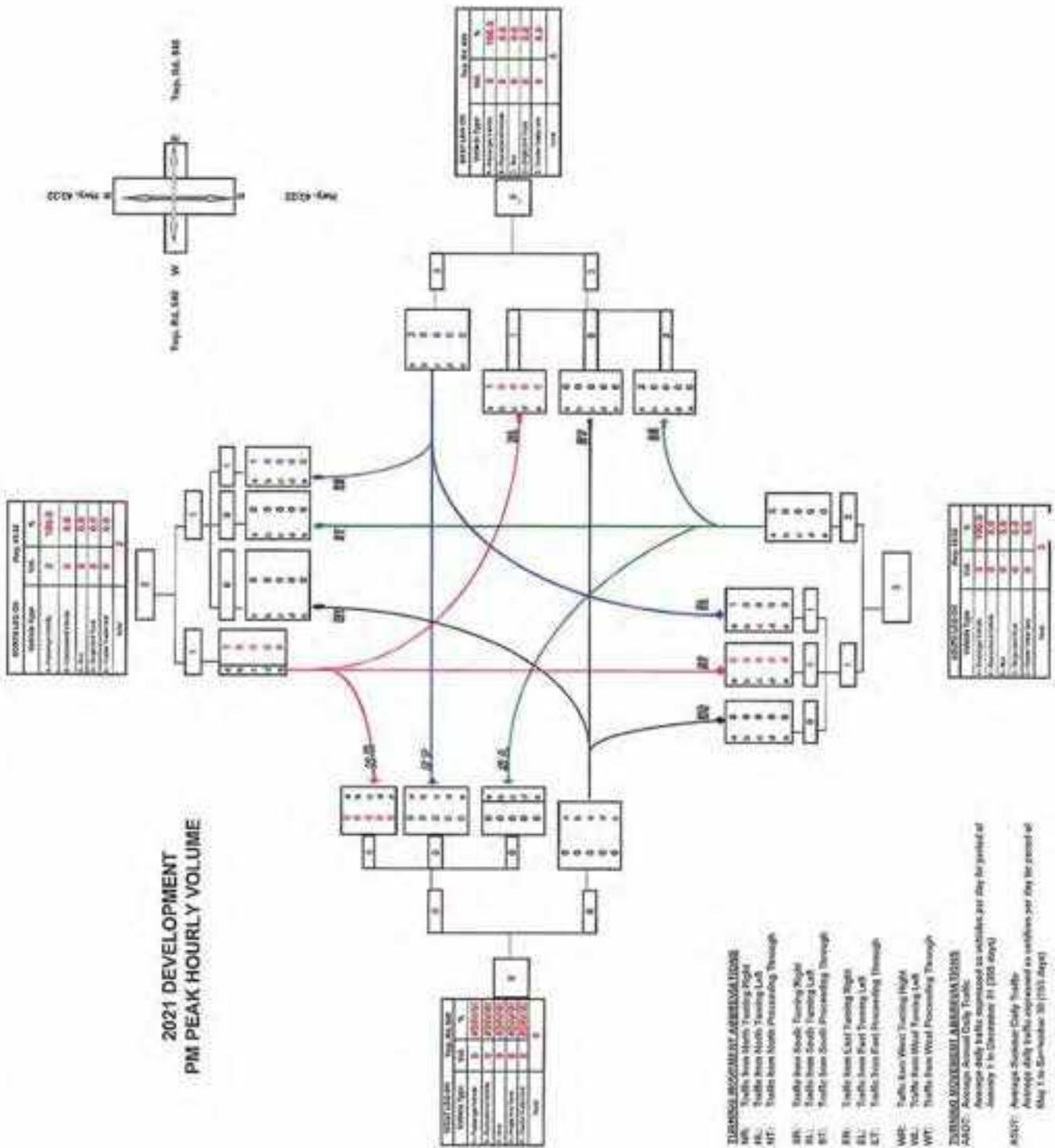


Turning Movement Summary Diagram

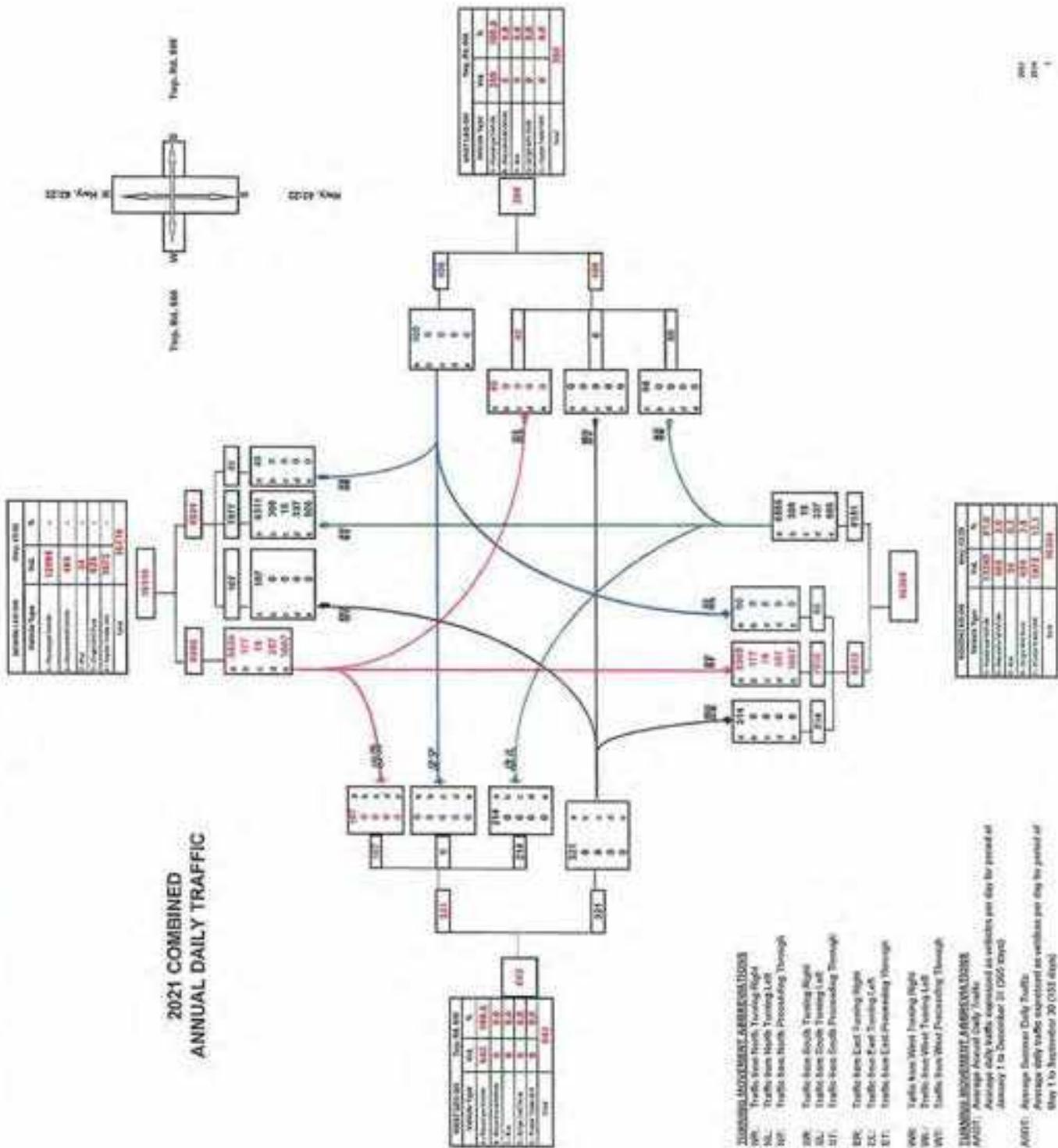


- TURNING MOVEMENT ABBREVIATIONS**
- TH: Traffic from South Turning Right
 - TL: Traffic from South Turning Left
 - TR: Traffic from North Proceeding Through
 - SH: Traffic from South Turning Right
 - SL: Traffic from South Turning Left
 - ST: Traffic from South Proceeding Through
 - TH: Traffic from East Turning Right
 - TL: Traffic from East Turning Left
 - TR: Traffic from East Proceeding Through
 - WH: Traffic from West Turning Right
 - WL: Traffic from West Turning Left
 - WR: Traffic from West Proceeding Through
- TURNING MOVEMENT ABBREVIATIONS**
- ADAD: Average Annual Daily Traffic
 - ADAD %: Average daily traffic expressed as a percentage of ADAD
 - ADAD %: Average daily traffic expressed as a percentage of ADAD
- Map 1 to 7 - 400-20-1181 (ext)

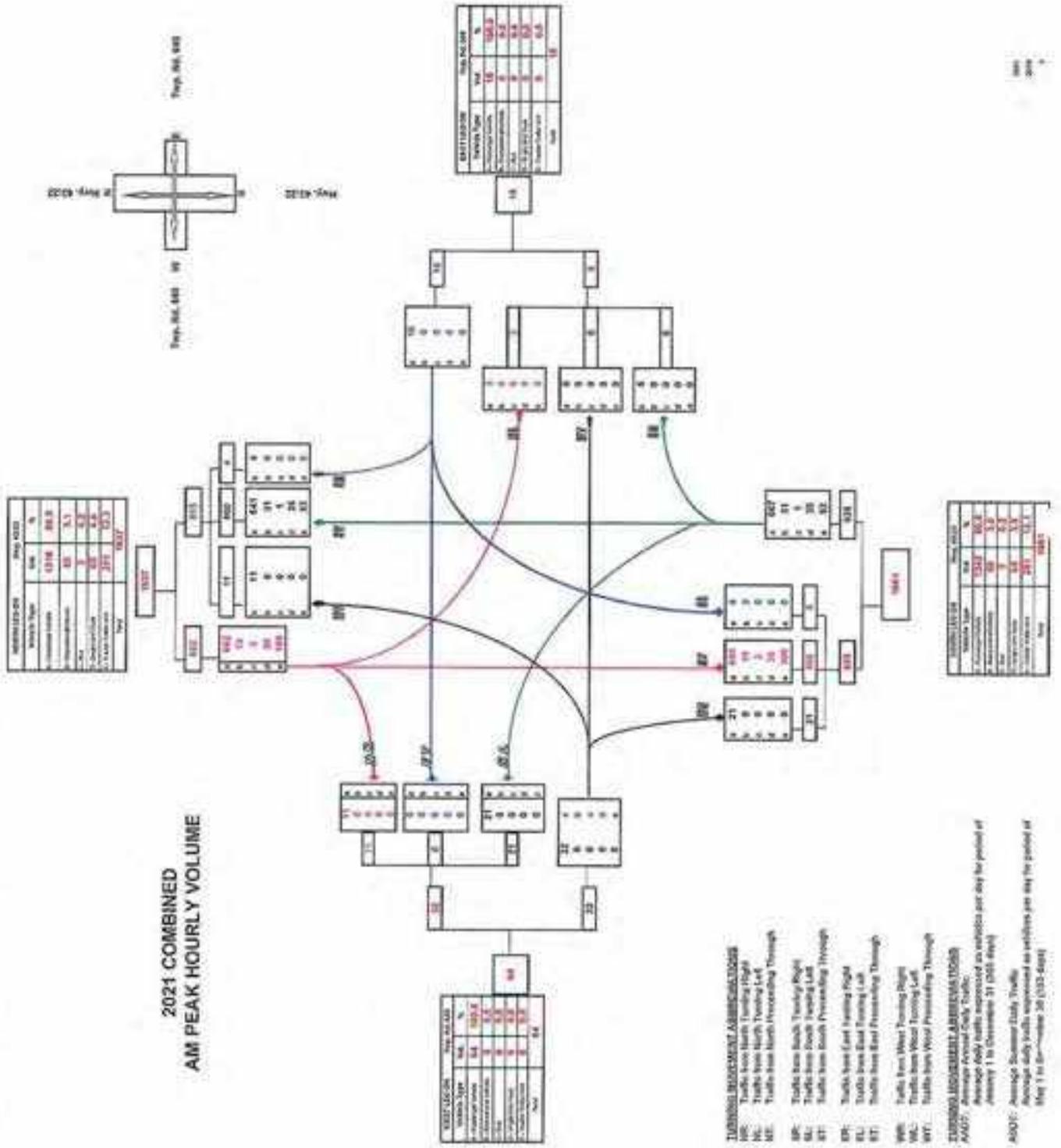


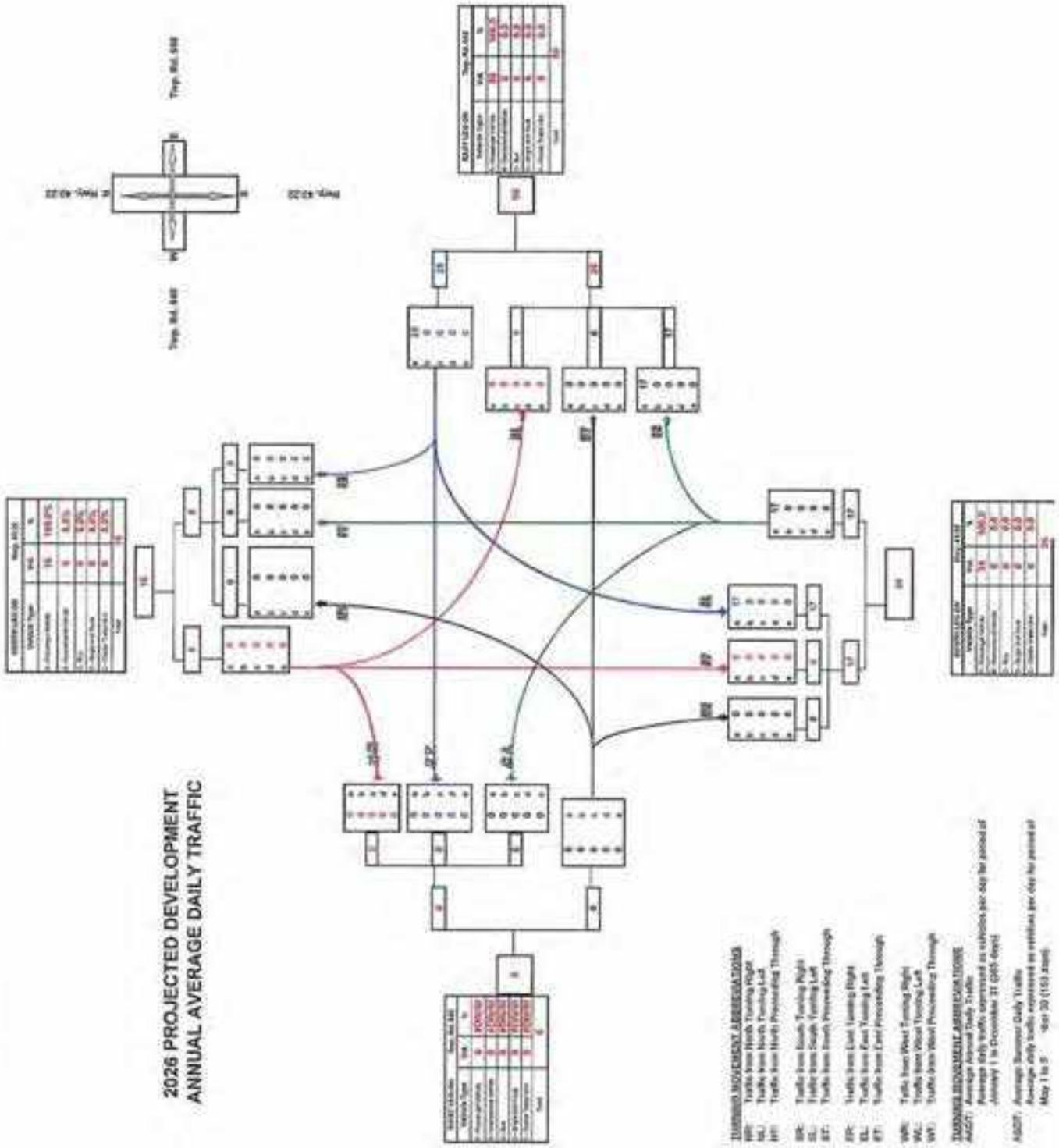


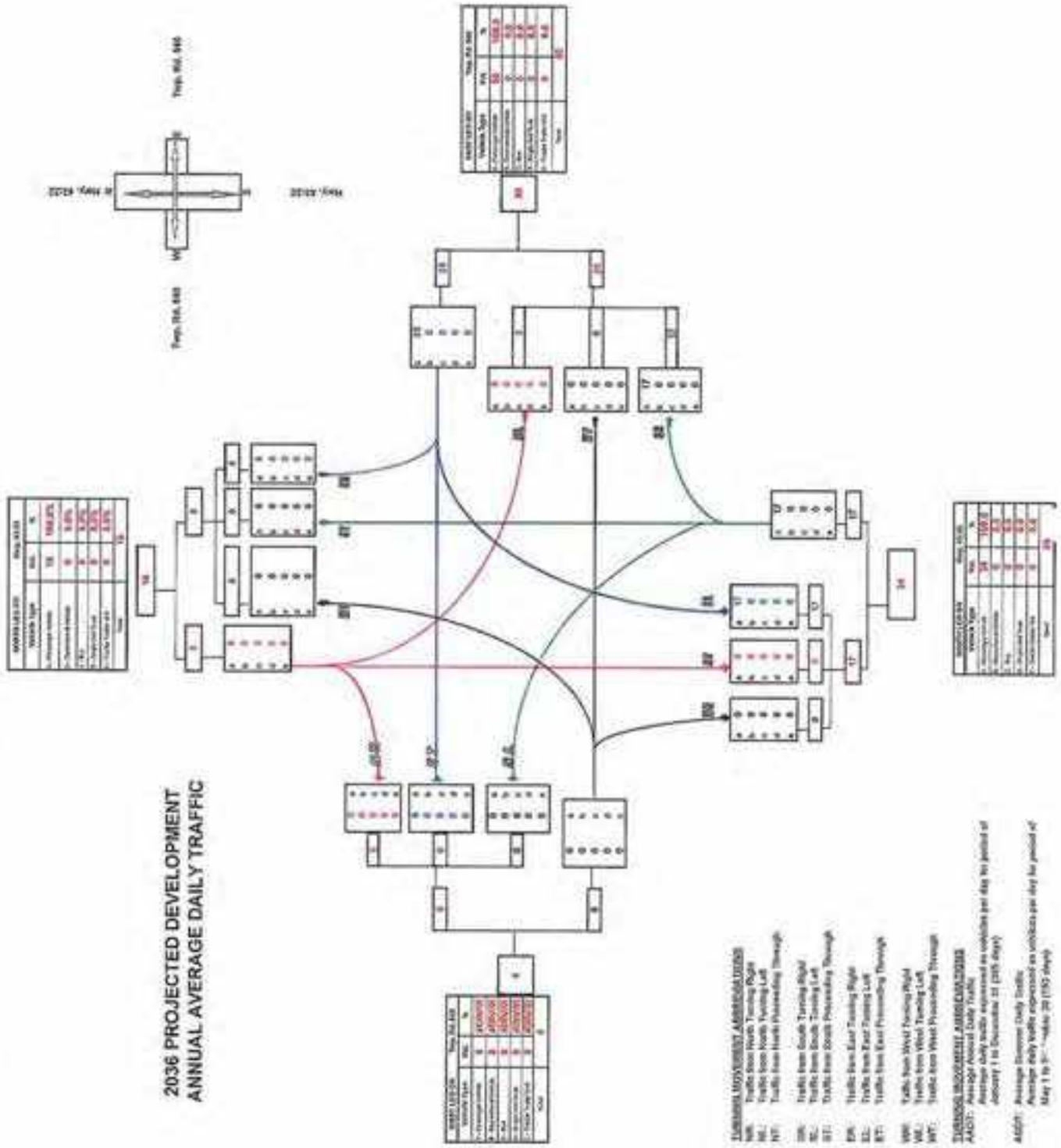
Turning Movement Summary Diagram

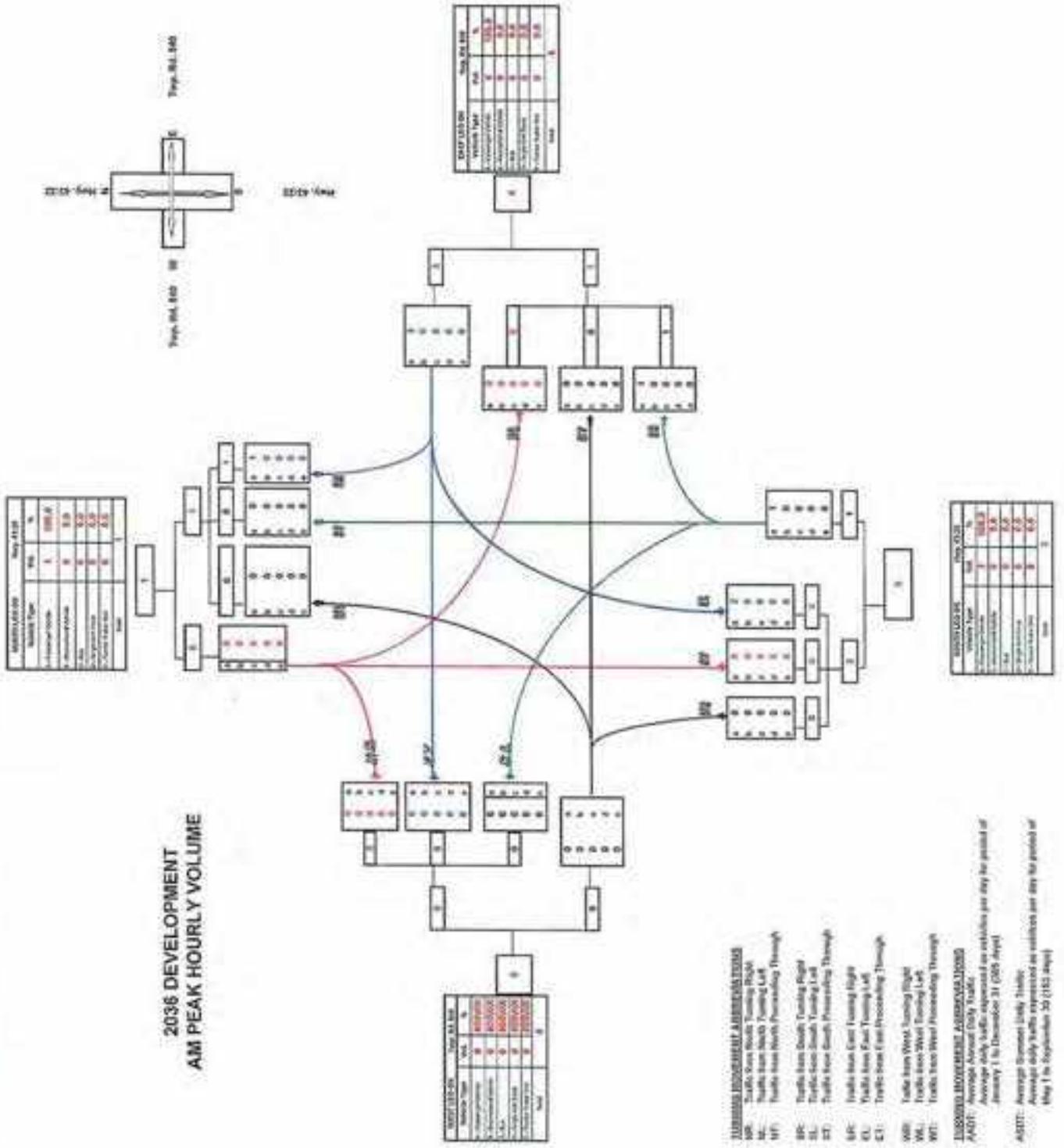


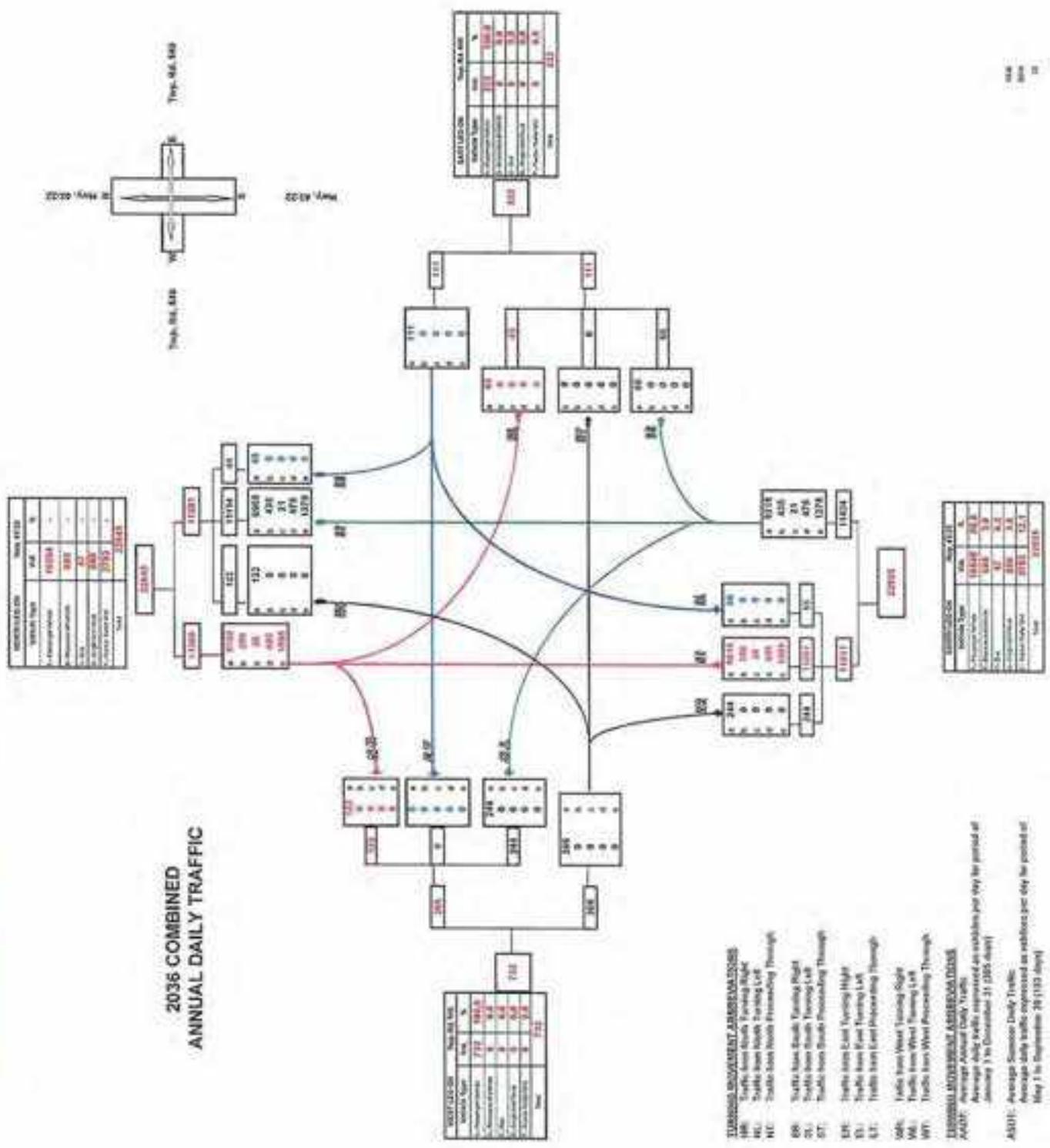
Turning Movement Summary Diagram



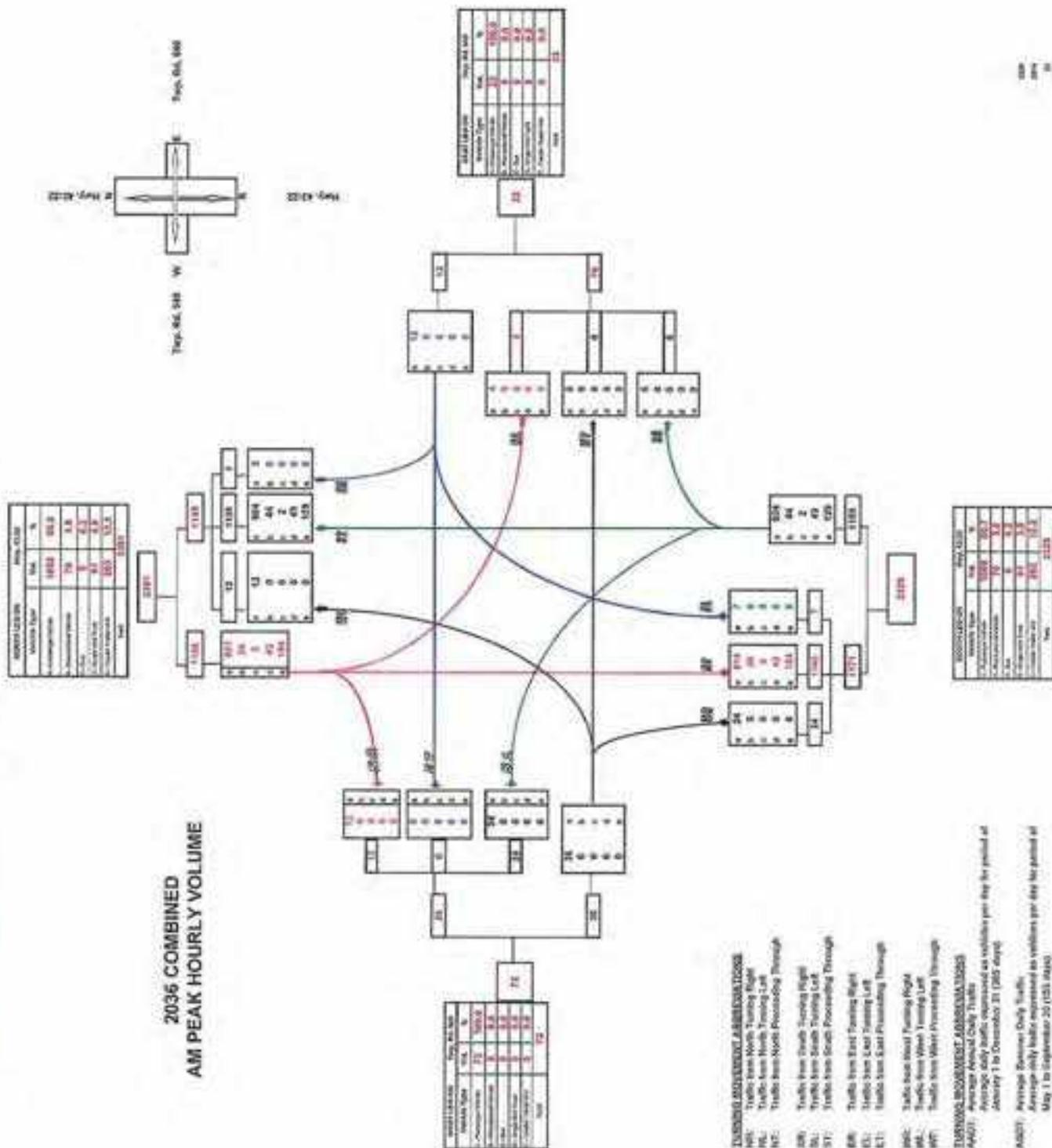








Turning Movement Summary Diagram



HCM Unsignalized Intersection Capacity Analysis
 3: Hwy. 43:22 & Twp. Rd. 540

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	10	0	20	5	0	3	20	690	4	3	698	10
Future Volume (Veh/h)	10	0	20	5	0	3	20	690	4	3	698	10
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	0	22	5	0	3	22	750	4	3	759	11
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								Raised			Raised	
Median storage (veh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1184	1563	380	1182	1572	377	770			754		
vC1, stage 1 conf vol	765	765		796	796							
vC2, stage 2 conf vol	419	798		386	776							
vCu, unblocked vol	1184	1563	380	1182	1572	377	770			754		
tC, single (s)	7.6	6.6	7.0	7.6	6.6	7.0	4.2			4.2		
tC, 2 stage (s)	6.6	5.6		6.6	5.6							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	96	100	96	98	100	100	97			100		
cM capacity (veh/h)	313	286	610	298	279	612	821			833		
Direction, Lane #	EB-1	WB-1	NB-1	NB-2	NB-3	SB-1	SB-2	SB-3	SB-4			
Volume Total	33	8	22	500	254	3	360	380	11			
Volume Left	11	5	22	0	0	3	0	0	0			
Volume Right	22	3	0	0	4	0	0	0	11			
cSH	454	369	821	1700	1700	833	1700	1700	1700			
Volume to Capacity	0.07	0.02	0.03	0.29	0.15	0.00	0.22	0.22	0.01			
Queue Length 95th (m)	1.8	0.5	0.7	0.0	0.0	0.1	0.0	0.0	0.0			
Control Delay (s)	13.4	15.0	9.5	0.0	0.0	9.3	0.0	0.0	0.0			
Lane LOS	B	B	A			A						
Approach Delay (s)	13.4	15.0	0.3			0.0						
Approach LOS	B	B										
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization			34.0%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis
 3: Hwy. 43:22 & Twp. Rd. 540

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	↕
Traffic Volume (veh/h)	10	0	20	4	0	3	20	693	5	3	700	10
Future Volume (Veh/h)	10	0	20	4	0	3	20	693	5	3	700	10
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	0	22	4	0	3	22	753	5	3	761	11
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								Raised			Raised	
Median storage (veh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1188	1569	380	1188	1578	379	772			758		
vC1, stage 1 conf vol	767	767		800	800							
vC2, stage 2 conf vol	420	802		386	778							
vCu, unblocked vol	1188	1569	380	1188	1578	379	772			758		
tC, single (s)	7.6	6.6	7.0	7.8	6.6	7.0	4.2			4.2		
tC, 2 stage (s)	6.6	5.6		6.6	5.6							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	96	100	96	99	100	100	97			100		
cM capacity (veh/h)	313	285	609	297	278	610	819			830		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3	SB 4			
Volume Total	33	7	22	502	256	3	380	380	11			
Volume Left	11	4	22	0	0	3	0	0	0			
Volume Right	22	3	0	0	5	0	0	0	11			
cSH	463	381	819	1700	1700	830	1700	1700	1700			
Volume to Capacity	0.07	0.02	0.03	0.30	0.15	0.00	0.22	0.22	0.01			
Queue Length 95th (m)	1.8	0.4	0.7	0.0	0.0	0.1	0.0	0.0	0.0			
Control Delay (s)	13.4	14.6	9.5	0.0	0.0	9.4	0.0	0.0	0.0			
Lane LOS	B	B	A			A						
Approach Delay (s)	13.4	14.6	0.3			0.0						
Approach LOS	B	B										
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization			34.0%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis
 3: Hwy. 43:22 & Twp. Rd. 540

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	11	0	21	6	0	4	21	800	5	3	808	11
Future Volume (Veh/h)	11	0	21	6	0	4	21	800	5	3	808	11
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	0	23	7	0	4	23	870	5	3	876	12
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								Raised			Raised	
Median storage (veh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1365	1805	439	1364	1814	438	890			875		
vC1, stage 1 conf vol	884	884		918	918							
vC2, stage 2 conf vol	481	921		445	886							
vCu, unblocked vol	1365	1805	439	1364	1814	438	890			875		
tC, single (s)	7.6	6.8	7.0	7.8	6.6	7.0	4.2			4.2		
tC, 2 stage (s)	6.6	5.8		6.6	5.6							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	95	100	96	97	100	99	97			100		
cM capacity (veh/h)	265	245	557	250	237	559	739			748		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3	SB 4			
Volume Total	35	11	23	580	295	3	439	439	12			
Volume Left	12	7	23	0	0	3	0	0	0			
Volume Right	23	4	0	0	5	0	0	0	12			
cSH	404	313	739	1700	1700	748	1700	1700	1700			
Volume to Capacity	0.09	0.04	0.03	0.34	0.17	0.00	0.26	0.26	0.01			
Queue Length 95th (m)	2.3	0.9	0.8	0.0	0.0	0.1	0.0	0.0	0.0			
Control Delay (s)	14.7	16.9	10.0	0.0	0.0	9.8	0.0	0.0	0.0			
Lane LOS	B	C	B			A						
Approach Delay (s)	14.7	16.9	0.3			0.0						
Approach LOS	B	C										
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization			37.7%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis
 3: Hwy. 43:22 & Twp. Rd. 540

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	11	0	21	5	0	4	21	803	6	4	811	11
Future Volume (Veh/h)	11	0	21	5	0	4	21	803	6	4	811	11
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	0	23	5	0	4	23	873	7	4	882	12
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								Raised			Raised	
Median storage (veh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1372	1816	441	1372	1824	440	894			880		
vC1, stage 1 conf vol	890	890		922	922							
vC2, stage 2 conf vol	482	928		449	902							
vCu, unblocked vol	1372	1816	441	1372	1824	440	894			880		
tC, single (s)	7.6	6.6	7.0	7.6	6.6	7.0	4.2			4.2		
tC, 2 stage (s)	6.6	5.6		6.6	5.6							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	95	100	96	98	100	99	97			99		
cM capacity (veh/h)	263	242	556	248	235	557	736			745		

Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3	SB 4
Volume Total	35	9	23	582	298	4	441	441	12
Volume Left	12	5	23	0	0	4	0	0	0
Volume Right	23	4	0	0	7	0	0	0	12
cSH	402	329	736	1700	1700	745	1700	1700	1700
Volume to Capacity	0.09	0.03	0.03	0.34	0.18	0.01	0.26	0.26	0.01
Queue Length 95th (m)	2.3	0.7	0.8	0.0	0.0	0.1	0.0	0.0	0.0
Control Delay (s)	14.8	16.2	10.0	0.0	0.0	9.9	0.0	0.0	0.0
Lane LOS	B	C	B			A			
Approach Delay (s)	14.8	16.2	0.3			0.0			
Approach LOS	B	C							

Intersection Summary		
Average Delay		0.5
Intersection Capacity Utilization	37.8%	ICU Level of Service
Analysis Period (min)	15	A

HCM Unsignalized Intersection Capacity Analysis
 3: Hwy. 43:22 & Twp. Rd. 540

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	11	0	22	6	0	4	22	908	5	3	919	11
Future Volume (Veh/h)	11	0	22	6	0	4	22	908	5	3	919	11
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	0	24	7	0	4	24	987	5	3	999	12
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								Raised			Raised	
Median storage (vsh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1548	2045	500	1543	2054	496	1011			992		
vC1, stage 1 conf vol	1005	1005		1038	1038							
vC2, stage 2 conf vol	542	1040		506	1017							
vCu, unblocked vol	1548	2045	500	1543	2054	496	1011			992		
tC, single (s)	7.6	6.6	7.0	7.6	6.6	7.0	4.2			4.2		
tC, 2 stage (s)	6.6	5.6		6.6	5.6							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	95	100	95	97	100	99	96			100		
cM capacity (veh/h)	224	210	509	210	202	511	664			675		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3	SB 4			
Volume Total	38	11	24	658	334	3	500	500	12			
Volume Left	12	7	24	0	0	3	0	0	0			
Volume Right	24	4	0	0	5	0	0	0	12			
cSH	357	267	664	1700	1700	675	1700	1700	1700			
Volume to Capacity	0.10	0.04	0.04	0.39	0.20	0.00	0.29	0.29	0.01			
Queue Length 95th (m)	2.7	1.0	0.9	0.0	0.0	0.1	0.0	0.0	0.0			
Control Delay (s)	16.2	19.0	10.6	0.0	0.0	10.4	0.0	0.0	0.0			
Lane LOS	C	C	B			B						
Approach Delay (s)	16.2	19.0	0.3			0.0						
Approach LOS	C	C										
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization			41.5%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis
 3: Hwy. 43:22 & Twp. Rd. 540

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔		↖	↕		↗	↕	↗
Traffic Volume (veh/h)	11	0	22	5	0	4	22	913	6	4	922	11
Future Volume (Veh/h)	11	0	22	5	0	4	22	913	6	4	922	11
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	0	24	5	0	4	24	992	7	4	1002	12
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								Raised			Raised	
Median storage (veh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1554	2057	501	1552	2066	500	1014			999		
vC1, stage 1 conf vol	1010	1010		1044	1044							
vC2, stage 2 conf vol	544	1047		509	1022							
vCu, unblocked vol	1554	2057	501	1552	2066	500	1014			999		
IC, single (s)	7.6	6.6	7.0	7.6	6.6	7.0	4.2			4.2		
IC, 2 stage (s)	6.6	5.6		6.6	5.6							
IF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	95	100	95	98	100	99	96			99		
cM capacity (veh/h)	222	208	508	208	200	509	662			671		

Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3	SB 4
Volume Total	36	9	24	661	338	4	501	501	12
Volume Left	12	5	24	0	0	4	0	0	0
Volume Right	24	4	0	0	7	0	0	0	12
cSH	355	282	662	1700	1700	671	1700	1700	1700
Volume to Capacity	0.10	0.03	0.04	0.39	0.20	0.01	0.29	0.29	0.01
Queue Length 95th (m)	2.7	0.8	0.9	0.0	0.0	0.1	0.0	0.0	0.0
Control Delay (s)	16.3	18.2	10.6	0.0	0.0	10.4	0.0	0.0	0.0
Lane LOS	C	C	B			B			
Approach Delay (s)	16.3	18.2	0.2			0.0			
Approach LOS	C	C							

Intersection Summary			
Average Delay		0.5	
Intersection Capacity Utilization	41.6%	ICU Level of Service	A
Analysis Period (min)	15		

HCM Unsignalized Intersection Capacity Analysis
 3: Hwy. 43:22 & Twp. Rd. 540

10/12/2015

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	12	0	24	7	0	5	24	1128	6	4	1140	12
Future Volume (Veh/h)	12	0	24	7	0	5	24	1128	6	4	1140	12
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	13	0	26	8	0	5	26	1226	7	4	1239	13
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								Raised			Raised	
Median storage (veh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1912	2532	620	1909	2542	616	1252			1233		
vC1, stage 1 conf vol	1247	1247		1282	1282							
vC2, stage 2 conf vol	665	1285		628	1260							
vCu, unblocked vol	1912	2532	620	1909	2542	616	1252			1233		
tC, single (s)	7.6	6.6	7.0	7.6	6.6	7.0	4.2			4.2		
tC, 2 stage (s)	6.6	5.6		6.6	5.6							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	92	100	94	95	100	99	95			99		
cM capacity (veh/h)	159	153	424	147	146	426	535			545		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3	SB 4			
Volume Total	39	13	26	817	416	4	620	620	13			
Volume Left	13	8	26	0	0	4	0	0	0			
Volume Right	26	5	0	0	7	0	0	0	13			
cSH	273	197	535	1700	1700	545	1700	1700	1700			
Volume to Capacity	0.14	0.07	0.05	0.48	0.24	0.01	0.36	0.36	0.01			
Queue Length 95th (m)	3.9	1.7	1.2	0.0	0.0	0.2	0.0	0.0	0.0			
Control Delay (s)	20.4	24.6	12.1	0.0	0.0	11.7	0.0	0.0	0.0			
Lane LOS	C	C	B			B						
Approach Delay (s)	20.4	24.6	0.2			0.0						
Approach LOS	C	C										
Intersection Summary												
Average Delay			0.6									
Intersection Capacity Utilization			49.1%		ICU Level of Service					A		
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis
 3: Hwy. 43:22 & Twp. Rd. 540

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	12	0	24	6	0	5	24	1134	7	5	1143	12
Future Volume (Veh/h)	12	0	24	6	0	5	24	1134	7	5	1143	12
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	13	0	26	7	0	5	26	1233	8	5	1242	13
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								Raised			Raised	
Median storage (veh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1920	2545	621	1920	2554	620	1255			1241		
vC1, stage 1 conf vol	1252	1252		1289	1289							
vC2, stage 2 conf vol	868	1293		631	1265							
vCu, unblocked vol	1920	2545	621	1920	2554	620	1255			1241		
tC, single (s)	7.6	6.6	7.0	7.6	6.6	7.0	4.2			4.2		
tC, 2 stage (s)	6.6	5.6		6.6	5.6							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	92	100	94	95	100	99	95			99		
cM capacity (veh/h)	158	152	423	145	144	423	534			541		

Direction, Lane #	EB-1	WB-1	NB-1	NB-2	NB-3	SB-1	SB-2	SB-3	SB-4
Volume Total	39	12	26	822	419	5	621	621	13
Volume Left	13	7	26	0	0	5	0	0	0
Volume Right	26	5	0	0	8	0	0	0	13
cSH	271	200	534	1700	1700	541	1700	1700	1700
Volume to Capacity	0.14	0.06	0.05	0.48	0.25	0.01	0.37	0.37	0.01
Queue Length 95th (m)	4.0	1.5	1.2	0.0	0.0	0.2	0.0	0.0	0.0
Control Delay (s)	20.5	24.1	12.1	0.0	0.0	11.7	0.0	0.0	0.0
Lane LOS	C	C	B			B			
Approach Delay (s)	20.5	24.1	0.2			0.0			
Approach LOS	C	C							

Intersection Summary

Average Delay		0.6							
Intersection Capacity Utilization		49.2%		ICU Level of Service				A	
Analysis Period (min)		15							

Illumination of Isolated Rural Intersections

LIGHTING WARRANT SPREADSHEET

This spreadsheet is to be used in conjunction with *Illumination of Isolated Rural Intersections*, Transportation Association of Canada, February 2001.

Please enter information in the cells with yellow background

INTERSECTION CHARACTERISTICS

Prop. 43/22
 Twp. Rd. 547
 Main Road
 Minor Road
 City/Town

Date December 9, 2015

Other YEAR 2006

GEOMETRIC FACTORS

	Value	Rating	Weight	Comments	Check	Score
Channelization Rating	Descriptive	0		Refer to Table 1(A) to determine rating value	OK	
Presence of raised channelization? (Y/N)	N				OK	
Highest operating speed on raised, channelized approach (km/h)	50		5		OK	0
Channelization Factor					OK	0
Approach Sight Distance on most constrained approach (m)	100	0	15	Relative to the recommended minimum sight distance	OK	0
Posted Speed limit (in 10's of km/h)	110				OK	
Radius of Horizontal Curve (m)	7			Enter "T" for tangent (no horizontal curve at the intersection)	OK	
Posted Speed Category =	A	0				
Posted Speed Category =		0				
Posted Speed Category =		0				
Posted Speed Category =		0				
Horizontal Curvature Factor		0	5		OK	0
Angle of Intersection (10's of Degrees)	90	0	5		OK	0
Downhill Approach Grade (x100)	1.8	0	3	Rounded to nearest tenth of a percent	OK	0
Number of Intersection Legs	4	2	3	Number of legs = 3 or more	OK	6
Geometric Factors Subtotal						6

OPERATIONAL FACTORS

Is the Intersection Signalized? (Y/N)	N			Calculate the Signalization Warrant Factor		
AAOT on Major Road (2-way)	22938	4	10	Either Use the two AAOT inputs OR the Descriptive Signalization Warrant (Unused values should be set to Zero) Refer to Table 1(B) for description and rating values for signalization warrant	OK	40
AAOT on Minor Road (2-way)	222	0	20		OK	0
Signalization Warrant	Descriptive	0	30		OK	0
Night-Time Hourly Pedestrian Volume	0	0	10	Refer to Table 1(B), note #2, to account for children and seniors	OK	0
Intersecting Roadway Classification	Descriptive	0	5	Refer to Table 1(B) for ratings	OK	0
Operating Speed or Posted Speed on Major Road (km/h)	110	4	5	Refer to Table 1(B), note #2	OK	20
Operating Speed or Minor Road (km/h)	50	3	5	Refer to Table 1(B), note #2	OK	15
Operational Factors Subtotal						75

ENVIRONMENTAL FACTOR

Lighted Developments within 150 m radius of intersection	0	0	5	Maximum of 4 quarters	OK	0
Environmental Factor Subtotal						0

COLLISION HISTORY

Average Annual night-time collision frequency due to inadequate lighting (collisions/yr, rounded to nearest whole #)	1.0	1	15	Enter either the annual frequency (See Table 1(C), note #4)	OK	15
OR				OR the number of collisions / MEV	OK	0
Collision Rate over last 3 years, due to inadequate lighting (MEV)	0	0	0	(Unused values should be set to Zero)	OK	0
Is the average ratio of all night to day collisions ≥ 1.5 (Y/N)	N	0			OK	
Collision History Subtotal						15

Check Intersection Signalization:
Intersection is not Signalized

LIGHTING IS NOT WARRANTED

SUMMARY

Geometric Factors Subtotal	6
Operational Factor Subtotal	75
Environmental Factor Subtotal	0
Collision History Subtotal	15

TOTAL POINTS: 96

HCM Unsignalized Intersection Capacity Analysis

3: Range Road 20 & Hwy. 16:14

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘
Traffic Volume (veh/h)	7	1655	2	19	1102	62	0	17	115	127	22	10
Future Volume (veh/h)	7	1655	2	19	1102	62	0	17	115	127	22	10
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	8	1799	2	21	1198	67	0	18	125	138	24	11
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised	Raised										
Median storage (veh)	3	3										
Upstream signal (m)												
pX, platoon unblocked	1265			1601			2479	3122	900	2290	3057	599
VC1, stage 1 cont vol							1815	1815	1240	1240	1817	
VC2, stage 2 cont vol							664	1307	1050	1050	1817	
VCU, unblocked vol	1265			1601			2479	3122	900	2290	3057	599
IC, single (s)	4.5			4.5			7.6	6.6	7.0	6.6	7.0	
IC, 2 stage (s)							6.6	5.6	6.6	5.6	6.6	
IF (s)	2.4			2.4			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	98			92			100	84	55	0	75	97
ch capacity (veh/h)	448			265			76	110	276	82	96	437
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	8	900	900	2	21	599	599	67	143	173		
Volume Left	8	0	0	0	21	0	0	0	0	0		
Volume Right	0	0	0	2	0	0	0	67	125	11		
ESH	448	1700	1700	265	1700	1700	1700	1700	232	68		
Volume to Capacity	0.02	0.53	0.53	0.00	0.08	0.35	0.35	0.04	0.62	1.98		
Queue Length 95th (m)	0.4	0.0	0.0	0.0	2.1	0.0	0.0	0.0	29.1	119.4		
Control Delay (s)	13.2	0.0	0.0	0.0	19.8	0.0	0.0	0.0	42.7	548.6		
Lane LOS	B				C				E	F		
Approach Delay (s)	0.1				0.3				42.7	548.6		
Approach LOS									E	F		
Average Delay	29.8											
Intersection Capacity Utilization	87.7%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	6	1352	10	28	1693	47	4	46	30	20	17	9
Future Volume (veh/h)	8	1352	10	28	1693	47	4	46	30	20	17	9
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	1470	11	30	1840	51	4	50	33	22	18	10
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (m)												
Px, platoon unblocked												
Vc, conflicting volume	1891			1481			2487	3439	735	2711	3399	920
Vc1, stage 1 cont vol							1488	1488	1488	1900	1900	920
Vc2, stage 2 cont vol							989	1951	811	1499		
Vcu, unblocked vol	1891			1481			2487	3439	735	2711	3399	920
Tc, single (s)	4.5			4.5			7.8	6.6	7.0	7.6	6.6	7.0
Tc, 2 stage (s)							6.6	5.6	6.6	6.6	5.6	
Tf (s)	2.4			2.4			3.5	4.0	3.3	3.5	4.0	3.3
pd queue free %	96			92			96	39	91	64	80	96
cd capacity (veh/h)	242			363			107	82	355	61	89	267
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	NB 2	SB 1	SB 2
Volume Total	9	735	735	11	30	920	920	51	87	50		
Volume Left	9	0	0	0	30	0	0	0	4	22		
Volume Right	0	0	0	11	0	0	0	51	33	10		
GSN	242	1700	1700	1700	363	1700	1700	1700	118	83		
Volume to Capacity	0.04	0.43	0.43	0.01	0.08	0.54	0.54	0.03	0.74	0.60		
Queue Length 95th (m)	0.9	0.0	0.0	0.0	2.1	0.0	0.0	0.0	32.8	21.8		
Control Delay (s)	20.4	0.0	0.0	0.0	15.8	0.0	0.0	0.0	93.5	98.6		
Lane LOS	C				C				F	F		
Approach Delay (s)	0.1			0.2					93.5	98.6		
Approach LOS									F	F		
Average Delay	3.9											
Intersection Capacity Utilization	74.6%											
Analysis Period (min)	15											

3: Range Road 20 & Hwy. 16:14
 HCM Unsignalized Intersection Capacity Analysis

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↘↘	↘	↘↘	↘↘	↘	↘	↘↘	↘	↘	↘	↘
Traffic Volume (veh/h)	8	1525	11	29	1984	48	4	48	31	21	18	10
Future Volume (veh/h)	8	1525	11	29	1984	48	4	48	31	21	18	10
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	1658	12	32	2157	52	4	52	34	23	20	11
Pedestrians												
Lane Width (m)												
Waking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		Raised			Raised							
Median storage (veh)		3			3							
Lipstream signal (m)												
PX, platoon unblocked	2209		1670			2840	3949	829	3128	3909	1078	
VC, conflicting volume												
WC1, stage 1 cont vol						1676	1676		2221	2221		
WC2, stage 2 cont vol						1164	2273		907	1688		
KU, unblocked vol	2209		1670			2840	3949	829	3128	3909	1078	
TC, single (s)	4.5		4.5			7.6	6.6	7.0	7.6	6.6	7.0	
TC, 2 stage (s)						6.6	5.6		6.6	5.6		
TC (s)	2.4		2.4			3.5	4.0	3.3	3.5	4.0	3.3	
PD queue free %	95		89			95	2	89	38	87	95	
CD capacity (veh/h)	176		301			75	53	308	37	60	209	
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	NB 2	SB 1	SB 2
Volume Total	9	829	829	12	32	1078	1078	52	90	54		
Volume Left	9	0	0	0	0	0	0	0	4	23		
Volume Right	0	0	0	12	0	0	0	52	34	11		
CSH	176	1700	1700	1700	301	1700	1700	1700	78	54		
Volume to Capacity	0.05	0.49	0.49	0.01	0.11	0.63	0.63	0.03	1.15	1.00		
Queue Length 95th (m)	1.3	0.0	0.0	0.0	2.8	0.0	0.0	0.0	52.6	38.1		
Control Delay (s)	26.5	0.0	0.0	0.0	18.4	0.0	0.0	0.0	241.2	247.1		
Lane LOS	D				C				F	F		
Approach Delay (s)	0.1				0.3				241.2	247.1		
Approach LOS	F				F				F	F		
Average Delay	8.8											
Intersection Capacity Utilization	84.8%											
ICU Level of Service	E											
Analysis Period (min)	15											

3: Range Road 20 & Hwy. 16:14
 HCM Unsignalized Intersection Capacity Analysis

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘
Traffic Volume (veh/h)	9	2504	2	22	1668	71	0	19	132	145	26	11
Future Volume (veh/h)	9	2504	2	22	1668	71	0	19	132	145	26	11
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	10	2722	2	24	1813	77	0	21	143	158	28	12
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (m)												
pX platoon unblocked	1890			2724			3722	4680	1361	3396	4605	906
VC, conflicting volume												
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	1890			2724			3722	4680	1361	3396	4605	906
TC, single (s)	4.5			4.5			7.6	6.6	7.0	7.6	6.6	7.0
TC, 2 stage (s)												
TC (s)	2.4			2.4			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue tree %	96			77			100	41	0	0	0	96
cd capacity (veh/h)	242			105			18	36	134	0	14	273
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	NB 1	SB 1	SB 1
Volume Total	10	1361	1361	2	24	906	906	77	164	198	158	12
Volume Left	10	0	0	0	24	0	0	0	0	0	158	12
Volume Right	0	0	0	2	0	0	0	77	143	198	0	0
CSH	242	1700	1700	1700	105	1700	1700	1700	99	0	0	0
Volume to Capacity	0.04	0.80	0.80	0.00	0.23	0.53	0.53	0.05	1.85	0	0	0
Queue Length 95th (m)	1.0	0.0	0.0	0.0	6.6	0.0	0.0	0.0	103.1	0	0	0
Control Delay (s)	20.5	0.0	0.0	0.0	49.4	0.0	0.0	0.0	409.2	0	0	0
Lane LOS	C			E					F			F
Approach Delay (s)	0.1			0.6					409.2			0
Approach LOS	F			F					F			F
Average Delay	Er			Er					Er			Er
Intersection Capacity Utilization	119.8%			119.8%					119.8%			119.8%
Analysis Period (min)	15			15					15			15

HCM Unsignalized Intersection Capacity Analysis

3: Range Road 20 & Hwy. 16:14

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘
Traffic Volume (veh/h)	10	1970	12	32	2564	53	5	52	34	22	19	10
Future Volume (veh/h)	10	1970	12	32	2564	53	5	52	34	22	19	10
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	2141	13	35	2787	58	5	57	37	24	21	11
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (m)												
PX, platoon unblocked												
VC, conflicting volumes	2845		2154			3648	5078	1070	4015	5033	1394	
VC1, stage 1 cont vol						2163	2915	1158	2857	2857	1394	
VC2, stage 2 cont vol						1485	2915	1158	2176	2176	1394	
VCU, unblocked vol	2845		2154			3648	5078	1070	4015	5033	1394	
IC, single (s)	4.5		4.5			7.6	6.6	7.0	6.6	6.6	7.0	
IC, 2 stage (s)						6.6	6.6	6.6	6.6	6.6	6.6	
IF (s)	2.4		2.4			3.5	4.0	3.3	3.5	4.0	3.3	
pl queue free %	88		81			82	0	83	0	11	91	
chl capacity (veh/h)	92		186			13	15	212	0	24	128	
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	NB 1	SB 1	SB 1
Volume Total	11	1070	1070	13	35	1394	1394	58	99	56		
Volume Left	11	0	0	0	35	0	0	0	5	24		
Volume Right	0	0	0	13	0	0	0	58	37	11		
CSH	92	1700	1700	186	1700	1700	1700	1700	23	0		
Volume to Capacity	0.12	0.63	0.63	0.01	0.19	0.82	0.82	0.03	4.31	EM		
Queue Length 50th (m)	3.1	0.0	0.0	0.0	5.4	0.0	0.0	0.0	EM	EM		
Control Delay (s)	49.2	0.0	0.0	0.0	28.8	0.0	0.0	0.0	EM	EM		
Lane LOS	E				D				F	F		
Approach Delay (s)	0.2				0.3				EM	EM		
Approach LOS	F				F				F	F		
Intersection Summary												
Average Delay	Er											
Intersection Capacity Utilization	104.9%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis

3: Range Road 20 & Hwy. 16:14

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘
Traffic Volume (veh/h)	7	7	2	18	18	18	914	914	59	0	16	110
Future Volume (veh/h)	7	7	2	18	18	18	914	914	59	0	16	110
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	8	8	2	20	20	20	993	993	64	0	17	120
Pedestrians												
Lane Width (m)												
Waiting Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (m)												
V/C, platooning volume	1057		1492				2063	2603	2603	745	1922	2541
V/C, conflicting volume							1506	1506	1506	1033	1033	1033
V/C, stage 1 conf vol							557	1097	1097	890	890	1508
V/C, stage 2 conf vol												
V/C, unblocked vol	1057		1492				2063	2603	2603	745	1922	2541
IC, single (s)	4.5		4.5				7.6	6.6	6.6	7.0	7.6	6.6
IC, 2 stage (s)												
IF (s)	2.4		2.4				3.5	4.0	3.3	3.5	4.0	3.3
P0 queue free %	99		94				100	89	66	8	84	98
CM capacity (veh/h)	548		359				119	158	350	139	145	511
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	NB 1	SB 1	SB 1
Volume Total	8	745	745	2	20	20	496	496	64	137	160	128
Volume Left	8	0	0	0	0	0	0	0	0	0	0	0
Volume Right	0	0	0	2	0	0	0	0	64	120	9	145
CSH	548	1700	1700	1700	359	1700	1700	1700	1700	303	145	145
Volume to Capacity	0.01	0.44	0.44	0.00	0.06	0.23	0.29	0.04	0.04	0.45	1.10	0.45
Queue Length 95th (m)	0.4	0.0	0.0	0.0	1.4	0.0	0.0	0.0	0.0	17.9	69.7	17.9
Control Delay (s)	11.7	0.0	0.0	0.0	15.6	0.0	0.0	0.0	0.0	28.3	165.3	28.3
Lane LOS	B				C					D		F
Approach Delay (s)	0.1				0.3					28.3		165.3
Approach LOS										D		F
Average Delay	10.6											
Intersection Capacity Utilization	76.6%											
Analysis Period (min)	15											

Intersection Summary

Highway 16:14 & Rge. Rd. 20 09/12/2015 Year 2016 AM Peak With Development Exist Treatment

3: Range Road 20 & Hwy. 16:14
 HCM Unsignalized Intersection Capacity Analysis

10/12/2015

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘	↘
Traffic Volume (veh/h)	7	1080	10	27	1405	42	4	44	29	18	16	8
Future Volume (veh/h)	7	1080	10	27	1405	42	4	44	29	18	16	8
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	8	1174	11	29	1527	46	4	46	32	20	17	9
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn lane (veh)												
Median type	Raised	Raised		Raised	Raised							
Median storage (veh)	3	3		3	3							
Upstream signal (m)												
PX, platoon unblocked												
VC, conflicting volume	1573			1185			2029	2821	587	2244	2786	764
VC1, stage 1 conf vol							1190	1190	1585	1585	1201	
VC2, stage 2 conf vol							839	1631	659	1201		
VCU, unblocked vol	1573			1185			2029	2821	587	2244	2786	764
IC, single (s)	4.5			4.5			7.8	6.6	7.0	7.6	6.6	7.0
IC, 2 stage (s)												
IF (s)	2.4			2.4			3.5	4.0	3.3	3.5	4.0	3.3
PQ queue free %	98			94			98	62	93	80	87	97
ch capacity (veh/h)	332			484			165	127	445	100	134	340
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	NB 1	SB 1	SB 1
Volume Total	8	587	587	11	29	764	764	46	84	46	20	46
Volume Left	8	0	0	0	0	0	0	0	0	4	20	46
Volume Right	0	0	0	11	0	0	0	46	32	4	9	46
CSH	332	1700	1700	484	1700	1700	1700	1700	177	177	130	130
Volume to Capacity	0.02	0.35	0.35	0.01	0.06	0.45	0.45	0.03	0.48	0.35	0.35	0.35
Queue Length 95th (m)	0.6	0.0	0.0	0.0	1.5	0.0	0.0	0.0	18.2	11.5	11.5	11.5
Control Delay (s)	16.1	0.0	0.0	0.0	12.9	0.0	0.0	0.0	42.6	47.1	47.1	47.1
Approach Delay (s)	0.1			0.2					42.6	47.1		
Approach LOS	C			B					E	E		
Average Delay	2.1											
Intersection Capacity Utilization	64.5%											
Analysis Period (min)	15											

“HYDROLOGICAL ASSESSMENT”

S.E. 1-54-2-5

PREPARED BY

GROUNDWATER INFORMATION TECHNOLOGIES LTD.

APPENDIX ‘D’

DECEMBER 2015



Groundwater Information
Technologies Ltd.

Phase I Aquifer Analysis

SE – 01 – 54 – 2W5

Lac Ste. Anne County

Prepared For:

Westcott Consulting Group

Prepared By:

Groundwater Information Technologies Ltd.



Ken Hugo, P.Geol.

APEGA P12077

August 13, 2015

File: 15 – 1217

Executive Summary

A review of available data was undertaken for a proposed 13 lot country residential subdivision in SE - 01 - 54 - 2W5 to determine expected water well yield and aquifer zones. Previously geological and hydrogeological studies were examined along with an examination of water well drillers' reports and groundwater quality analysis reports for the area.

According to the Water Act, if a well is installed on each lot, each lot is required to be supplied with water at a rate of 1250 m³/year and water wells should be capable of providing water at a minimum rate of 1 imperial gallon per minute. If a community well is installed water needs are less, approximately 400 m³/year for each lot.

Groundwater use in the area is primarily from domestic demands, with some wells installed for stock or industrial use.

The surficial deposits which immediately underlay the site are unconsolidated glacial deposits, which are underlain by the sandstones, siltstones and mudstones of the Horseshoe Canyon Formation. The sands and gravels of the surficial aquifer may be silty or dry underlying the site, and the primary aquifer present beneath the Site are bedrock sandstones and siltstones of the Horseshoe Canyon Formation. This formation is generally found at depths greater than 20 meters and is up to 100 metres in thickness.

Analysis of available pumping test data shows that the Horseshoe Canyon Aquifer is capable of supplying water at a rate of approximately 16.940 m³/year (7.1 imperial gallons per minute). This value is sufficient to supply individual lots by a well on each lot. Aquifer supplies for a single community well may not be available.

A review of water quality records for the area show that the groundwater is a sodium bicarbonate type water of Total Dissolved Solids concentration of around less than 500 mg/L and should be acceptable to most users. It is recommended that water samples be collected and analyzed for routine dissolved salts and bacterial parameters from each well prior to use as a potable supply in the house.

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Appendix 1 – Water Well Reconnaissance Report
Appendix 2 – Water Well Driller's Reports
Appendix 3 – Water Quality Analyses Reports



Water Well Drilling Report

View in Imperial Export to Excel

The data supplied for this report is for informational purposes only. The Province assumes no responsibility for its accuracy. The information on this report will be retained in a public database.

QC Well ID: 45842
QA Well Tag No:
Drilling Company Well ID:
Date Report Received:

1162023

SCWV 0

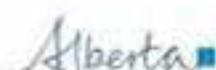
Well Identification and Location										Measurement in Metric	
Owner Name CLARKE DALE	Address RR 3 SITE 310 COMP 1				Town CRONWAY		Province AB	County CA	Phone Code 708 910		
Location	143180	500	TWP	R2E	W10M08	Lot	Block	Plan	Additional Description HEATHERDOWN		
	21	54	054	02	5	7	2	0026058			
Welcome from Boundary of			GPS Coordinates in Decimal Degrees (NAD 83)			Elevation			M		
_____ m from _____			Latitude 51.621528			Longitude -114.226543			_____ m		
_____ m from _____			How Location Obtained			How Elevation Obtained			Not Obtained		
			Map								

Additional Information										Measurement in Metric	
Distance from Top of Casing to Ground Level		_____ m		Is Risk Control Installed		_____		Describe		_____	
Is Artesian Flow		_____		Rate		L/min					
Recommended Pump Rate		_____ L/min		Pump Installed		_____		Depth		_____ m	
Recommended Fluid Intake Depth (From TOC)		_____ m		Type		_____		Material		_____ m	
								Made (Casing Rating)		_____	
Do you Encounter Saline Water (>4000 ppm TDS)		_____		Depth		_____ m		Well Drilled upon Completion		_____	
				Date		_____		Geophysical Log Taken		_____	
								Submitted to ESAC		_____	
								Samples Collected for Potability		_____	
								Submitted to ESAC		_____	
Additional Comments on Well 1800 LITRES OF WATER USED FOR DRILLING. BOREHOLE DIAMETER 8.75" & 9".											

Yield Test			Taken From Ground Level		Measurement in Metric	
Test Date	Start Time	Static Water Level	Drawdown (m)	Elapsed Time	Recovery (m)	
				Minutes/Sec		
20071004	9:35 AM	36.15 m	35.11	0:00	45.31	
Method of Water Removal				1:00	44.43	
Type Air				2:00	42.71	
Removal Rate			45.45 L/min	3:00	41.73	
Depth with Water From			54.90 m	4:00	40.91	
				5:00	40.11	
				6:00	39.33	
				7:00	38.58	
				8:00	38.81	
				9:00	38.53	
				10:00	38.28	
				12:00	37.83	
				14:00	37.67	
				16:00	37.46	
				20:00	37.30	
				25:00	37.00	
				30:00	36.96	
				35:00	36.77	
				40:00	36.69	
				50:00	36.50	
				60:00	36.54	
				75:00	36.47	
				90:00	36.41	
				105:00	36.38	
				120:00	36.38	
			45.31			

Water Diverted for Drilling		
Water Source	Amount Taken	Diversion Date & Time

Contractor Certification	
Name of Journeyman responsible for drilling/construction of well RANDY KIEDLINDER	Certification No. 45842
Company Name CLARKE DRILLING LTD	Copy of Well report provided to owner Date approval holder signed



CHEMICAL ANALYSIS REPORT

WELL NAME WADDELL PAT GIC WELL ID 456677
 LOCATION LSD NW SEC 35 TWP 053 R0 02 M 5 SAMPLE NO. 5246
 WELL DEPTH 124.00 ft WATER LEVEL ft
 AQUIFER LABORATORY AE
 SAMPLING DATE 1985-05-22

FIELD	MG/L	FIELD	MG/L
BICARBONATE		CARBONATE	
CHLORIDE		CONDUCTIVITY	
DISSOLVED OXYGEN		EH	
IRON		MANGANESE	
PH		SULPHATE	
S2		TEMPERATURE(C)	0
TOTAL ALKALINITY		TOTAL HARDNESS	
LABORATORY		Analysis Date	1985-06-04
COD		CONDUCTIVITY	779
DIC		FLUORIDE	0.1000
ION BALANCE	1.0100	PH	7.90
SAR		SIO2	13.9000
TOTAL ALKALINITY	407.0000	TC	
TDS	443	TN	
DOC		BICARBONATE	496.0466
AMMONIUM-N		CARBONATE	
CALCIUM	79.9997	MAGNESIUM	16.0125
CHLORIDE	-1.0011	NITRITE-N	-0.0504
NITRATE-N		POTASSIUM	1.9442
PHOSPHATE		SULPHATE	22.0098
SODIUM	77.9999	TOTAL HARDNESS	266.0000
NO2 + NO3	-0.0504	ARSENIC	
ALUMINUM		BERYLLIUM	
BARIUM		CHROMIUM	
CADMIUM		COPPER	
COBALT		LEAD	
IRON	2.1400	MERCURY	
MANGANESE		NICKEL	
MOLYBDENUM		STRONTIUM	
SELENIUM		ZINC	
VANADIUM		PESTICIDES	

Remarks:

NATURE OF ALK AND HARDNESS IS CaCO3.

Temperatures reported in Degree Centigrade. Conductivity reported in microsiemens/cm, pH in pH units. Alkalinity and Hardness expressed as Calcium Carbonate. FE, VA, PB, AL, AG expressed as extractable. FE in field measurements and all remaining metals expressed as total. - indicates concentrations less than.

EH - Oxidation-Reduction Potential SAR - Sodium Adsorption Ratio DIC - Dissolved Inorganic Carbon
 COD - Chemical Oxygen Demand DOC - Dissolved Organic Carbon TN - Total Particulate Nitrogen
 TDS - Total Dissolved Solids TC - Total Particulate Carbon

Note: this data may not be fully checked. The Province disclaims all responsibility for its accuracy



CHEMICAL ANALYSIS REPORT

WELL NAME	FOSEUM, PAIGE	GIC WELL ID	449552
LOCATION	L8D SW SEC 01 TWP 054 RG 02 M 5	SAMPLE NO.	7952
WELL DEPTH	130.00 ft	WATER LEVEL	ft
AQUIFER		LABORATORY	AE
SAMPLING DATE	1978-07-29		

FIELD	MG/L	FIELD	MG/L
BICARBONATE		CARBONATE	
CHLORIDE		CONDUCTIVITY	
DISSOLVED OXYGEN		EH	
IRON		MANGANESE	
PH		SULPHATE	
SD		TEMPERATURE(C)	3
TOTAL ALKALINITY		TOTAL HARDNESS	
LABORATORY		Analysis Date	1978-08-10
COO		CONDUCTIVITY	591
DIC		FLUORIDE	0.0600
ION BALANCE	1.0700	PH	8.12
SAR		SiO2	13.9000
TOTAL ALKALINITY	284.0000	TC	
TDS	315	TN	
DOC		BICARBONATE	348.0057
AMMONIUM-N		CARBONATE	
CALCIUM	55.9995	MAGNESIUM	10.0077
CHLORIDE	2.0033	NITRITE-N	-0.0504
NITRATE-N		POTASSIUM	1.2280
PHOSPHATE		SULPHATE	-10.0144
SODIUM	62.9993	TOTAL HARDNESS	182.0000
NO2 - NO3	0.2702	ARSENIC	
ALUMINIUM		BERYLLIUM	
BARIUM		CHROMIUM	
CADMIUM		COPPER	
COBALT		LEAD	
IRON	3.0800	MERCURY	
MANGANESE		NICKEL	
MOLYBDENUM		STRONTIUM	
SELENIUM		ZINC	
VANADIUM		PESTICIDES	
HYDROCARBONS			
PHENOLICS			

Remarks:

Temperature reported in Degree Centigrade. Conductivity reported in microsiemens/cm, pH in pH units. Alkalinity and Hardness expressed as Calcium Carbonate. FE, VA, PB, AL, AG expressed as extractable. FE in field measurements and all remaining metals expressed as total. * indicates concentrations less than:

EH - Oxidation-Reduction Potential	SAR - Sodium Adsorption Ratio	DIC - Dissolved Inorganic Carbon
COO - Chemical Oxygen Demand	DOC - Dissolved Organic Carbon	TN - Total Particulate Nitrogen
TDS - Total Dissolved Solids	TC - Total Particulate Carbon	

Note: this data may not be fully checked. The Province disclaims all responsibility for its accuracy

**“PIT REGISTRATION APPLICATION
-YEOMAN/PARKER PIT”**

S.E. 1-54-2-5

PREPARED BY

ASPEN LAND GROUP INC..

APPENDIX ‘E’

DECEMBER 2013

**Pit Registration Application
Yeoman/Parker Pit**

**October 2013
(Revised December 2013)**



Prepared For:

Westrock Aggregates Ltd.

Prepared by:



Executive Summary

For many years sand and gravel mining operations have been ongoing within the E 1-54-2-W5M. Based on a review of historical aerial photographs, mining operations began within the northern portion of the NE 1-54-2-W5M in the 70's. Mining operations then continued southward across the majority of the quarter and eventually into the northwest corner of the SE 1-54-2-W5M until the early 90's.

Although the history is somewhat unclear, it would appear that the pit was initially opened up and operated by Yeoman Aggregates Ltd. (Yeoman). In the late 80's the pit was then taken over by Westrock Aggregates Ltd. (Westrock). Approvals for the pit had been acquired under the former *Land Surface Conservation and Reclamation Act* (LSCRA) in 1986 and again in 1990. In 1990 a revised application was submitted under LSCRA and in 1997 an application was submitted under the *Environmental Protection and Enhancement Act* (EPEA). As neither of these applications received approval, mining operations were discontinued in the early to mid 90's and the pit has been inactive since that time.

Given that Westrock now plan to reclaim the pit, Aspen Land Group Inc. was retained to prepare a focused registration application that will allow Alberta Environment and Sustainable Resource Development (ESRD) to register the pit under the *Code of Practice for Pits* (the Code) for reclamation purposes.

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1.0 Introduction

1.1 Overview

Westrock Aggregates Ltd. (Westrock) is proposing to finish reclaiming the 56.29 ha disturbance associated with the sand and gravel pit located in the E 1-54-2-W5M. The pit is located along the west side of Range Road 20 approximately 5 km east of Hwy. 43 as shown on Drawing No. 1. Pit operations will include the removal of all remaining stockpiled product, the importation of topsoil and overburden material, and reclamation of the current disturbance.

The purpose of this report is to provide information required by Alberta Environment and Sustainable Resource Development (ESRD) to register this pit under the *Code of Practice for Pits* (the Code) for the sole purpose of reclaiming the pit. The schedules required as part of the registration process are included in Appendix B.

1.2 Land Ownership

Westrock currently owns the NE 1-54-2-W5M while Charles and Melanie Parker currently own the SE 1-54-2-W5M. Copies of the Certificate of Titles are included in Appendix A.

2.0 Existing Conditions

2.1 History

Based on a review of historical aerial photographs, mining within the NE 1-54-2-W5M commenced sometime between the 1973 and 1977 as shown on Drawing No. 4. On the 1977 aerial photograph it is evident that mining activities had begun to take place within the northern portion of the quarter and that an area of approximately 8.5 ha had been disturbed. As shown on the 1981 aerial photograph included on Drawing No. 4, this active mining area had expanded southward and that approximately 24.67 ha had been disturbed.

A Sand and Gravel Pit Inventory Report that was conducted by ESRD (formerly Alberta Environment) in 1985 indicated that 70 to 80 acres had been disturbed with approximately 40 acres of reserves remaining to be mined. Some reclamation was ongoing in terms of overburden placement while the older area within the eastern portion of the pit was beginning to re-vegetate itself naturally. Topsoil thickness was reported to be 6 to 8 inches with overburden varying from 10 to 25 feet.

In 1986, Yeoman Construction Ltd. (Yeoman) submitted an application for a Development and Reclamation Approval under the *Land Surface Conservation and Reclamation Act* (LSCRA). A five year approval was issued in March of 1986 (Approval No. SG-39-86) that sanctioned a 10 acre parcel out of a 120 acre development. Although it is not entirely clear why only 10 acres were sanctioned; it is assumed that the balance of the disturbed lands may have been considered as a pre-78 disturbance. Financial security was obtained in the amount of \$2,500.00. It is important to note that in the application submitted in support of

this approval that the topsoil depth was reported at 3 to 5 inches with the overburden depth ranging from 4 to 40 feet.

In the summer of 1989, a site inspection conducted by ESRD revealed a number of compliance issues associated with the pit. When Westrock had taken over the pit, they provided updated applications on October 24, 1989 and on February 9, 1990 in an attempt to address the compliance issues. On December 5, 1990 an approval was issued under LSCRA for a sanctioned area of 20 acres. This sanctioned area included future mining areas to the north of an intermittent drainage course within the SE 1-54-2-W5M as shown on Drawing No. 3. At that time financial security was assessed at \$5,000.00 but it was never submitted.

On January 24, 1991 Westrock submitted an application under LSCRA to amend their approval to allow mining operations to proceed along the western side of the SE 1-54-2-W5M south of the intermittent drainage course as shown on Drawing No. 3. This amendment was never finalized; however, a 1500 mm concrete culvert was installed across the creek and mining commenced within an area south of the creek.

An application for approval was submitted under EPEA on May 26, 1997 that was subsequently revised on September 26, 1997. Although the application was deemed sufficiently complete to initiate the public review process, an approval was never issued.

Pit sizes for the different years/milestones are outlined below:

Extraction area pre-1978	8.5 ha
Extraction area post-1978	24.67 ha
Sanctioned area 1986 LSCRA approval	4.1 ha (10 acres)
Sanctioned area 1990 LSCRA approval	8.1 ha (20 acres)
Current/overall disturbed area	56.29 ha

2.2 Current and Adjacent Land Use Activities

The total disturbance associated with pit activities to date is approximately 56.29 ha. As the pit has been relatively inactive for a number of years, some of the disturbed areas are beginning to revegetate themselves. This is especially evident in areas where the overburden material had been re-contoured along the eastern side of the pit within the NE 1-54-2-W5M. The majority of the disturbed areas where are vegetated with trees and typical understory species along with well established grasses.

Lafarge Canada Inc. (Lafarge) maintains a haul road that extends across the north central part of the NE 1-54-2-W5M in an east/west direction. This road is used to haul product from their pits to the west to their wash plant located on the eastside of Rge. Rd. 20 within the W 6-54-1-W5M. A large maintenance shop and associated buildings are located to the north of the Lafarge haul road. Although located in an area that once formed part of the pit, this shop was previously rented out and will remain in place as an improvement to the end land use.

The majority of the area located to the north of the Lafarge haul road represents a pre-78 disturbance while the area to the south of the Lafarge haul road within the NE 1-54-2-W5M for the most part forms the un-reclaimed pit area.

Apart from the treed area associated with the creek that extends across the northern portion of the SE 1-54-2-W5M and the area that was disturbed as a result of mining activities, the balance of the area within this quarter is under cultivation and is used for agricultural purposes.

Lafarge is currently mining a pit in the NW 1-54-2-W5M. Over the past couple years, Lafarge has mined the aggregate and reclaimed a portion of the common property line between the NE 1 and the NW 1-54-2-W5M as shown on Drawing No. 3. Yellowhead Aggregates (Yellowhead) is currently mining a pit within the SW 1-54-2-W5M. At this time Westrock and Yellowhead are in discussions about mining out the common property boundary between these two quarters, however, an agreement has not been made at this time. In the event that an agreement is reached to mine out this common property line, Westrock will submit an updated activities plan.

As depicted on Drawing No. 2 and 3, there are existing excavations associated with past mining activity currently located to the south within the NE 1-54-2-W5M and to the northwest in the SE 1-54-2-W5M. The remaining disturbed area has been stripped or had overburden material directly placed in low areas over undisturbed land to level out the area and to increase the arable land.

2.3 Topography and Surface Drainage

The topography of the area consists of a high relief landform or ridge sloping from west to east. Elevations range from 755 masl to 730 masl. Contours and existing surface drainage patterns are shown on Drawing No. 2.

The undisturbed land and partially reclaimed lands within the NE1-54-2-W5M generally slope from the west to the east or to the northeast. There is an intermittent drainage course crossing the northern part of the SE 1-54-2-W5M that conveys water in an easterly direction to a culvert under Rge. Rd. 20 and eventually to Kilini Creek. Although this intermittent drainage course also conveyed runoff from the lands to the west of the subject property, surrounding aggregate operations to the west have removed the majority of the upstream catchment basin, significantly reducing its natural flow. The majority of the surface runoff from within the SE 1-54-2-W5M drains directly to the east or to the northeast towards the intermittent drainage course located within the northern part of the quarter.

2.4 Soils

Based on the Alberta Soils Information Viewer, the soils within the general area of the E 1-54-2-W5M are comprised of three different soil polygons.

Polygon 18958 includes the west half of the SE 1-54-2-W5M and is comprised of well drained soils of the Dark Gray Luvisol and Orthic Gray Luvisol subgroups of the Uncas and

Cooking Lake soil series. Polygon 19049 includes the southwest corner of the NE 1-54-2-W5M and is comprised 80% of very poorly drained soils of the Rego Humic Gleysol subgroup of the Kerensky series and 20% of well drained soils of the Dark Grey Luvisol subgroup of the Elk Point Series. Polygon 19130 comprises the remaining area of the NE 1-54-2-W5M and is comprised of well drained miscellaneous coarse-textured soils in the black soil zone. This polygon also includes soils with Rego profiles that have highly variable landscape characteristics and soils series distribution that may be the result of past mining activities.

Based on a site assessment conducted on August 30, 2013 no subsoil stockpiles were found within the NE 1-54-2-W5M nor was there any evidence of replaced subsoil in the reclaimed areas. Considering when this part of the pit was operated, salvaging of topsoil and subsoil may have not been stripped to meet current criteria. Therefore, subsoil will not be available for reclamation in the NE 1-54-2-W5M.

As shown Drawing No. 3, topsoil and subsoil was salvaged and stockpiled separately for pit operations within the SE 1-54-02-W5M. The texture of the stockpiled topsoil and subsoil was primarily textured as a sandy loam; however, there were some sample locations that reflected more of a sandy clay loam texture.

2.5 Land Capability for Agriculture

The CLI capability for agriculture is rated as class 4M (60%) and O (40%) within the E 1-54-2-W5M. Soils that are rated as class 4 have severe limitations and have restrictions on the range of crops or require special conservation practices due to moisture limitations. The areas that are rated as O are considered organic soils.

2.6 Surficial Geology

Based on the Quarternary Geology, Central Alberta map (I. Shetsen 1990) the subject property is located in an area described as a stagnation moraine which typically exhibits till of uneven thickness with local water sorted material up to 30 m thick. This stagnation moraine also exhibits undulating to hummocky topography reflecting variations in till thickness. In this area, the topography is described as undulating with local relief generally less than 3 m.

Bedrock at the base of the deposit consists of grey feldspathic, clayey sandstone, grey bentonitic mudstone and carbonaceous shale of the Horseshoe Canyon Formation of the Cretaceous age. The bedrock contact is non marine and is often characterized by a concretionary ironstone beds, scattered coal and bentonite beds of variable thickness, minor limestone beds.

2.7 Stratigraphy

Drawing No. 6 through 9 depict the current surface conditions. Based on a review of ESRD's file material, the average undisturbed soil depths within the pit area would have been as follows:

Topsoil	- 6 to 8 in (0.15 to 0.20 m)
Subsoil	- not available
Overburden	- 10 to 25 ft (3.0 to 7.60 m)
Aggregate	- not available

As outlined in Section 2.4, subsoil was not appropriately salvaged within the NE 1-54-2-W5M. Subsoil material that was salvaged in the SE 1-54-2-W5M will be evenly distributed in that part of the pit prior to topsoil placement.

There is limited information available as to where and how much aggregate was removed by the previous Yeoman or Westrock operations. Although no formal aggregate test hole information is available, Westrock has concluded that no economical gravel remains to be mined within the pit. Consequently, as no further aggregate extraction is proposed, the provision of aggregate testing information is not considered required for the proposed reclamation activities.

2.8 Groundwater

The previous operations did not appear to have intercepted groundwater. As reclamation is the only activity proposed, groundwater is not expected to be encountered.

2.9 Vegetation and Wildlife

The proposed reclamation activities will only impact those areas that have already been previously disturbed and will not result in the loss of any naturally vegetated areas or negatively impact wildlife in the area. Common throughout the Onoway area, considerable tansy was noted onsite along with other noxious weeds such as Canada thistle and yellow toadflax.

3.0 Reclamation Plan

3.1 Current Development

Although at one time relatively active, as the aggregate resource within the pit has now been depleted Westrock would like to register the current pit disturbance of 56.29 ha for reclamation. All areas south of the Lafarge haul road will be reclaimed while the processing/shop area will be maintained. A portion of the Lafarge haul road from Rge. Rd. 20 to the processing/shop will also remain in order to provide secondary access to the shop. That portion of the haul road that extends from the shop area to the western boundary of the NE 1-54-2-W5M will be reclaimed and integrated with the surrounding landscape. In addition to the portion of the Lafarge haul road that will remain in place, the southern access road developed between 1977 and 1981 off of Rge. Rd. 20 will also remain in place.

In the unlikely event that aggregate material is encountered during reclamation that is economical to mine, an updated activities plan will be submitted prior to extraction.

3.2 Pit Access

As shown on Drawing No. 2, access to the pit is off of Twp. Rd. 541 and Rge. Rd. 20 with the pit being located approximately 8 km north of Highway 16.

3.3 Property Line Buffers, Pit Faces and Extraction Setbacks

A portion of the common property boundary between the NE and the NW 1-54-2-W5M has been mined out by Lafarge with their mining operations within the NW 1-54-2-W5M. In order to mine out this common property boundary, Lafarge extended their operations by approximately 130 to 150 m into the Yeoman Pit. As this mined out common property boundary has now been reclaimed and properly integrated with the adjacent lands, the standard 3:1 slopes that are typically constructed adjacent to undisturbed property boundaries are no longer required.

For pit activities located within the SE 1-54-2-W5M, there are stockpiles of reclamation material encroaching within the 3 m undisturbed buffer along the common property between the SE 1 and the SW 1-54-2-W5M. Unless an agreement to mine out this common property boundary can be made between Westrock and Yellowhead as outlined in Section 2.2, the 3 m undisturbed buffer will be reestablished with reclaimed slopes of 3:1 or gentler constructed along the pit face as shown on Drawing No. 8.

The culvert which provides access from the south portion of the SE 1-54-2-W5M to the north portion, will also remain in place as part of the reclaimed landscape.

If the reclamation of this pit should not proceed within the next two years, as an interim measure any areas within the pit that have slopes that are greater than 2:1 will be re-sloped to 2:1 or gentler to ensure public safety and to avoid the potential loss of reclamation material due to erosion or slumping.

3.4 Measures to Control Dust, Wind and Water Erosion

In terms of mitigation measures to control wind and water erosion, all stockpiles of reclamation material will remain vegetated until they are ready to be distributed over the re-contoured landscape. Once reclamation material has been put in place, re-vegetation of the area will occur as soon as possible. Surface runoff from the disturbed areas within the active pit area will, as much as possible, be maintained onsite and redirected to existing excavations or other low areas within the pit.

3.5 Contouring and Soil Replacement

The objective in terms of reclamation is to ensure that the disturbed lands associated with the pit are reclaimed to an equivalent land capability. Due to the timing difference between the operations within the NE 1 and the SE 1-54-2-W5M, the reclamation objectives will be slightly different while trying to meet equivalent land capability.

Contouring and Soil Replacement for the SE 1-54-2-W5M (Parker Pit)

The disturbance south of the channel represents approximately 6 ha of the current disturbed area. Slopes along the west property line will be reclaimed to no steeper than 3:1; while the remaining slopes along the pit boundary will be reclaimed to no steeper than 10:1.

That being said, Westrock may have the opportunity to import up to 100,000 m³ of fill material to raise the elevation of this area to match pre-disturbance elevations, as shown on Drawing No. 8 and 9. If Westrock is unable to bring or fill or brings in less than 100,000m³ of fill, the reclaimed landscape will be to a lower elevation, as shown on Drawing No. 8 and 9. Utilizing material stockpiled within this quarter, the topsoil replacement depth will be approximately 18 cm while the subsoil replacement depth will be approximately 9 cm.

Contouring and Soil Replacement for the NE 1-54-2-W5M (Yeoman Pit)

Areas where dense volunteer tree vegetation or hayland has developed will remain in place as shown on Drawing No. 3. Areas that are comprised of haphazard excavations and stockpiles of soil material will be leveled out and integrated to develop a harmonious landscape. It is anticipated that the slopes of these reclaimed areas will for the most part be 10:1 or gentler but there may be areas where the slopes are closer to 4:1 as conditioned in LSCRA Approval No. SG-39-86. It is anticipated that the land will be reclaimed back to an agricultural end land use (pasture/cultivation) with some treed areas.

As mentioned in Section 3.3, the property line buffer and associated 3:1 slopes are not required along the west portion of the NE1-54-2-W5M given that it has been mined out and reclaimed by Lafarge.

Once the site is re-contoured, all available topsoil that is currently stockpile or exhumed during reclamation activities will be spread evenly over all re-contoured areas. In order to increase agricultural production Westrock may import up to 100,000 m³ of topsoil to the pit. Imported topsoil will only be placed within the reclaimed areas in the NE 1-54-2-W5M.

Prior to the placement of topsoil material, any areas exhibiting compaction will be alleviated.

3.6 Revegetation

The reclaimed areas within the NE 1-54-2-W5M will be revegetated using a hayland seed mix while in the SE 1-54-2-W5M a seed mix will be used for the reclaimed area that matches the landowner's current crop rotation.

3.7 Importation of Reclamation Materials Handling Plan

As mentioned in Section 3.5, Westrock may import material to augment the reclamation of the pit. It is anticipated that up to 100,000 m³ of topsoil and 100,000 m³ of overburden material could be imported from around the Edmonton area.

All imported material will be from sites with no previous disturbance (i.e., agriculture to residential). An inventory of the material received will be maintained that will include such things as:

- the location where the material was obtained and placed;
- the quantity of the material received; and
- random sample analysis to confirm no contaminants.

As there is sufficient topsoil material in the SE 1-54-2-W5M to properly reclaim the site, no imported topsoil material will be applied to this area. Up to 100,000 m³ of imported overburden fill material will be used to fill the excavation area to reestablish positive drainage to the intermittent creek and avoid ponding of water.

To make up for the lack of topsoil material available for the NE 1-54-2-W5M, imported topsoil material may be utilized to establish a topsoil layer to a maximum depth of 30 cm.

Imported material will only be brought onsite when it can be direct placed. No imported material will be stockpiled.

3.8 Environmental Management Plan

To minimize the impact on the environment during reclamation, Westrock will implement a number of environmental management practices. These environmental management practices include:

- having spill kits on all equipment used for mining purposes;
- use of double wall fuel storage tanks and/or containment berms;
- properly storing and regularly hauling any industrial waste generated at the pit to an approved municipal or class II landfill;
- properly collecting and regularly hauling all sanitary waste to an approved waste management treatment facility;
- an active weed control program that includes mowing, hand pulling, spot spraying, seeding stockpiles of reclamation material, etc. to prevent the initial establishment of weeds;
- the proper application of herbicides; and
- ensuring that no herbicides, pesticides or any other hazardous substance will be stored onsite.

4.0 Financial Security

As Westrock has no future intentions of gravel extraction operations, the reclamation security estimate of \$190,705.46 represents the maximum conservation and reclamation costs (i.e., full-cost security) to reclaim the remainder of the pit. The main cost associated with the security estimate is the amount of reclamation material that is stockpiled. The maximum area of disturbance that will be secured at third party costs is the current limit area of disturbance less the process site/access roads (4.26 ha) of 52.03 ha.

Security Estimate

Mobilization and Demobilization	\$ 5,000.00
Third Party Costs for the NE 1-54-2-W5M (46.03 ha)	
Reclamation Material	
Topsoil Material Placement – 4,863 m ³ @ \$2.50/m ³	\$ 12,157.50
Overburden Material Placement – 10,314 m ³ @ \$2.50/m ³	\$ 25,785.00
Recontouring	
D8R Crawler Tractor (Non-Current) – 110 hrs @ \$264.00/hr	\$ 29,040.00
Decompaction	
D7H Crawler Tractor with ripper (Non-Current) – 40 hrs @ \$266.25/hr	\$ 10,650.00
Seedbed Preparation	
Tractor with disc – 20 hrs @ \$125.00/hr	\$ 2,500.00
Seeding	
Tractor with seed drill/seed – 46.03 ha @ \$135.00/ha	\$ 6,214.05
Fertilizing	
Tractor with fertilizer attachment – 46.03 ha @ \$135.00/ha	\$ 6,214.05
Subtotal	\$ 92,560.60
Third Party Costs for the SE 1-54-2-W5M (6.0 ha)	
Reclamation Material	
Topsoil Material Placement – 11,197 m ³ @ \$2.50/m ³	\$ 27,992.50
Subsoil Material Placement – 5,775 m ³ @ \$2.50/m ³	\$ 14,437.50
Overburden Material Placement – 10,334 m ³ @ \$2.50/m ³	\$ 25,835.00
Recontouring	
D8R Crawler Tractor (Non-Current) – 12 hrs @ \$264.00/hr	\$ 3,168.00
Decompaction	
D7H Crawler Tractor with ripper (Non-Current) – 8 hrs @ \$266.25/hr	\$ 2,130.00
Seedbed Preparation	
Tractor with disc – 5 hrs @ \$125.00/hr	\$ 625.00
Seeding	
Tractor with seed drill/seed – 6.0 ha @ \$135.00/ha	\$ 810.00
Fertilizing	
Tractor with fertilizer attachment – 6.0 ha @ \$135.00/ha	\$ 810.00
Subtotal	\$ 75,808.00
Total Subtotal at Third Party Costs	\$ 173,368.60
Administration @ 10 % of Total Third Party Costs	\$ 17,336.86
Total Security	\$ 190,705.46

Rates are based on the Alberta Roadbuilders and Heavy Construction Association 2013 Equipment Rental Rates Guide and Membership Roster in combination with ESRD's standard rate of \$2.50/m³ for the movement of reclamation material.

5.0 Limitations

This report has been prepared for the sole benefit of Westrock Aggregates Inc. This document may not be used by any other person or entity without the express written consent of Aspen Land Group Inc. and Westrock Aggregates Inc., with the exception of Alberta Environment and Sustainable Resource Development. Any use of this report by a third party, or any reliance on decisions made based on it, or damages suffered as a result of the use of this report are the sole responsibility of the user.

The information and conclusions contained in this report are based upon work undertaken by trained professional and technical staff in accordance with generally accepted scientific practices current at the time the work was performed. The conclusions and recommendations presented represent the best judgment of Aspen Land Group Inc. based on the data obtained during the assessment. Due to the nature of the assessment and the data available, Aspen Land Group Inc. cannot warrant against undiscovered environmental liabilities. Conclusions and recommendations presented in this report should not be considered legal advice.

Appendix A: Certificate of Titles



LAND TITLE CERTIFICATE

S
LINC SHORT LEGAL TITLE NUMBER
0035 230 200 5;2;54;1;SE 122 166 512 +1

LEGAL DESCRIPTION

MERIDIAN 5 RANGE 2 TOWNSHIP 54
SECTION 1
QUARTER SOUTH EAST
CONTAINING 64.7 HECTARES (160 ACRES) MORE OR LESS.
EXCEPTING THEREOUT :

		HECTARES	(ACRES)	MORE OR LESS
A) PLAN 7820078	ROAD	0.809	2.00	
B) PLAN 9924788	DESCRIPTIVE	1.81	4.47	
C) PLAN 1222170	SUBDIVISION	7.66	18.93	

EXCEPTING THEREOUT ALL MINES AND MINERALS

ESTATE: FEE SIMPLE

MUNICIPALITY: LAC STE. ANNE COUNTY

REFERENCE NUMBER: 992 218 766 +1

REGISTERED OWNER(S)
REGISTRATION DATE (DMY) DOCUMENT TYPE VALUE CONSIDERATION

122 166 512 29/05/2012 SUBDIVISION PLAN

OWNERS

CHARLES HERBERT PARKER

AND

MELANIE LOU PARKER

BOTH OF:

BOX 43, SITE 320, RR #3

STONY PLAIN

ALBERTA T7Z 1X3

AS JOINT TENANTS

(DATA UPDATED BY: CHANGE OF ADDRESS 122413751)

(CONTINUED)

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LAND TITLE CERTIFICATE

S
LINC SHORT LEGAL TITLE NUMBER
0011 350 360 5;2;54;1;NE 892 276 384

LEGAL DESCRIPTION

THE NORTH EAST QUARTER OF SECTION ONE (1)
TOWNSHIP FIFTY FOUR (54)
RANGE TWO (2)
WEST OF THE FIFTH MERIDIAN
CONTAINING 64.7 HECTARES (160 ACRES), MORE OR LESS.
EXCEPTING THEREOUT:

(A) 0.809 HECTARES (2 ACRES), MORE OR LESS, AS SHOWN
ON ROAD PLAN 1583T

(B) ALL THAT PORTION DESCRIBED AS FOLLOWS:
COMMENCING AT A POINT OF THE EASTERLY BOUNDARY OF SAID
QUARTER SECTION WHERE THE SOUTHERLY BOUNDARY OF A ROADWAY
AS SHOWN ON ROAD PLAN 1583T INTERSECTS THE SAID EASTERLY
BOUNDARY THENCE WESTERLY ALONG THE SAID SOUTHERLY BOUNDARY
OF SAID ROADWAY A DISTANCE OF THREE HUNDRED AND THIRTY
(330) FEET TO A POINT THENCE SOUTHERLY AND PARALLEL TO THE
SAID EASTERLY BOUNDARY A DISTANCE OF FIVE HUNDRED AND TWENTY
EIGHT (528) FEET TO A POINT THENCE EASTERLY AND PARALLEL TO
THE SAID SOUTHERLY BOUNDARY OF SAID ROADWAY A DISTANCE OF
THREE HUNDRED AND THIRTY (330) FEET MORE OR LESS TO A POINT
ON THE SAID EASTERLY BOUNDARY THENCE NORTHERLY ALONG SAID
EASTERLY BOUNDARY TO THE POINT OF COMMENCEMENT CONTAINING
1.62 HECTARES (4 ACRES), MORE OR LESS.

(C) 0.639 HECTARES (1.58 ACRES), MORE OR LESS, AS SHOWN
ON ROAD PLAN 7820078

EXCEPTING THEREOUT ALL MINES AND MINERALS

ESTATE: FEE SIMPLE

MUNICIPALITY: LAC STE. ANNE COUNTY

REFERENCE NUMBER: 792 031 710

REGISTERED OWNER(S)				
REGISTRATION	DATE (DMY)	DOCUMENT TYPE	VALUE	CONSIDERATION
892 276 384	25/10/1989	TRANSFER OF LAND	\$17,500	\$17,500

(CONTINUED)



Water Well Drilling Report

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The user assumes the data contained in this report. The Province disclaims responsibility for its accuracy. The information in this report will be retained in a public database.

GIC Well ID: 465878
GIC Well Tag No:
Drilling Company Well ID:
Date Report Received: 2020-01-20

SOVW ID:

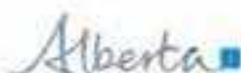
Well Identification and Location										Measurement in Metric	
Owner Name	Address		Town		Province	County				Rate Code	
DUDE, LEO	ONEWAY										
Location	T14 or T30	SEC	T10P	R2E	R of QPR	S10	Block	Plan	Additional Description		
	5E	21	054	2E	5						
Measured from Boundary of				GPS Coordinates in decimal degrees (WGS 84)				Elevation			
_____ m from _____				Latitude: <u>51.602161</u>				_____ m			
_____ m from _____				Longitude: <u>-114.155218</u>				_____ m			
				How Location Obtained:				How Elevation Obtained:			
				Not Verified				Not Obtained			

Drilling Information	
Method of Drilling: Rotary	Type of Well: New Well
Proposed Well Use: Domestic	

Formation Log			Measurement in Metric
Depth from ground level (m)	Water Bearing	Lithology Description	
5.38		Clay & Soulders	
11.89		Blue Clay	
15.24		Yellow Clay	
19.85		Light Brown Shale	
19.48		Green Shale	
19.29		Gray Shale	
11.85		Gray Silty Shale	
23.16		Green Shale	
26.85		Gray Shale	
31.26		Sandstone	
32.82		Gray Shale	

Yield Test Summary			Measurement in Metric
Recommended Pump Rate	40.91 L/min		
Test Date	Water Removal Rate (L/min)	Static Water Level (m)	
1999/09/14	40.91	15.48	
Well Completion			
Total Depth Drilled	Finished Well Depth	Start Date	End Date
32.82 m		1999/09/14	1999/09/14
Sanhole			
Diameter (cm)	From (m)	To (m)	
0.00	0.00	32.82	
Surface Casings (if applicable):		Well Casings/liner	
Size OD	18.24 cm	Size OD	11.43 cm
Wall Thickness	0.953 cm	Wall Thickness	0.635 cm
Bottom at	32.56 m	Top at	21.34 m
		Bottom at	31.09 m
Perforations			
From (m)	To (m)	Diameter or Slot Width (cm)	Slot Length (cm)
25.30	31.35	0.000	
How or Slot Interval: 0.33			
Perforated by: Hand Drn			
Annular Seal: Driven & Shale Trap			
Placed from	22.25 m	to	32.56 m
Amount	_____		
Other Seals			
Type	At (m)		
Screen Type			
Size OD	0.00 cm	From (m)	To (m)
			Slot Size (cm)
Attachment: _____			
Top Filings: _____		Bottom Filings: _____	
Pack			
Type	Grain Size: _____		
Amount	_____		

Contractor Certification	
Name of Journeyman responsible for drilling/construction of well: LAKYGAN NA DRILLER	Certification No: 1
Company Name: ROCCO DRILLING	Copy of Well report provided to client: <input type="checkbox"/> Date Approval Folder signed: _____



Water Well Drilling Report

View in Imperial Export to Excel

The data supplied in this report is the property of the Province of Alberta. The Province disclaims responsibility for its accuracy. The information in this report will be retained in a public database.

GC Well ID: 1145281
GC Well Tag No:
Drilling Company Well ID:
Date Report Received:

GC Well ID:

Well Identification and Location										Measurements in Metric	
Owner Name CLUKKE DALE		Address RR 3 SITE 310 COMP 7				Town ONDWAY		Province AB	County CA	Postal Code T0E 1A0	
Location	1/4 or L&C	SEC	T&P	R102	1/4 of NEP	Lot	Block	Plan	Additional Description HEATHERDOWN		
Well is from Boundary of					GPS Coordinates in Decimal Degrees (NAD 83)			Elevation		m	
					Latitude <u>53.831635</u>			Longitude <u>-114.322053</u>		How Elevation Obtained	
					How Location Obtained					Not Obtained	

Drilling Information	
Method of Drilling Rotary	Type of Well New Well
Proposed Well Use Domestic	

Formation Log		Measurements in Metric	
Depth from ground level (m) - Bearing	Lithology-Description		
1.32	Brownish Yellow Till		
17.89	Blue Gray Clay		
36.27	Blue Gray Till & Rock		
38.62	Gravel		
40.84	Green Soft Shale		
44.81	Gray Medium Grained Sandstone		
55.08	Green Shale		
64.31	Gray Medium Grained Sandstone		
68.49	Green Shale		
71.03	Gray Medium Grained Sandstone		
73.56	Green Shale		
75.33	Gray Medium Grained Sandstone		
79.25	Green Shale		

Yield Test Summary			Measurements in Metric	
Recommended Pump Rate	<u>27.25 L/min</u>			
Test Date	Water Removal Rate (L/min)	Static Water Level (m)		
2007/10/04	45.46	36.15		

Well Completion				Measurements in Metric	
Total Depth Drilled	Finished Well Depth	Start Date	End Date		
79.25 m	0.00	2007/10/03	2007/10/04		
Borehole					
Diameter (cm)	From (m)	To (m)			
22.22	0.00	79.25			
Surface Casing (if applicable)			Well Casing/liner		
Material	Size OD	Weight	Size OD	Weight	
Plastic	15.24 cm		11.43 cm		
Well Thickness	0.951 cm	Well Thickness	0.909 cm		
Bottom at	53.04 m	Top at	59.21 m		
		Bottom at	79.25 m		
Perforations					
From (m)	To (m)	Diameter or Slot Width (cm)	Slot Length (cm)	Hole or Slot Interval (m)	
55.08	64.31	0.129		30.48	
58.49	71.33	0.129		30.48	
73.59	78.33	0.129		30.48	
Performed by: Saw					
Annular Seal: Shale Trap & Bentonite					
Flow from	0.00 m	to	53.04 m		
Amount					
Other Seals					
Type					At (m)
Screen Type					
Size OD	cm				
From (m)	To (m)	Slot Size (cm)			
Attachment					
Top Fittings			Bottom Fittings		
Pack					
Type	Unknown	Grain Size			
Amount: Unknown					

Contractor Certification	
Name of Journeyman responsible for drilling/completion of well RANDY REDINGER	Certification No. 45940
Company Name CALIBRE DRILLING LTD.	Copy of Well report provided to owner: _____ Date approved/enter signed

Introduction

At the request of Robert Westcott of Westcott Consulting Group, an aquifer analysis was conducted for a proposed 13-lot residential subdivision located within SE¼ - 01- 54 - 2W5. Lots range in size from 0.89 to 2.48 hectares (ha). A site plan outlining the proposed subdivision is shown below:

Figure 1 – Site plan of proposed subdivision



Water is to be supplied by a groundwater source. According to the Water Act, if individual wells are located on each lot then water requirements are to be supplied at a rate of 1250 cubic metres per year ($m^3/year$) and wells should be capable of supplying water at a minimum rate of 1 Imperial gallon per minute (igpm). If water needs are supplied by a community well supply, water needs are based on house size and number of bedrooms, and are generally around 250 gallons per day per lot.

This review consists of an examination of currently available information and should provide an estimate into depths, yields and quality of water from the aquifer(s) underlying the site. As no drilling or pumping tests were performed as part of the investigation, the review provides a representative analysis; however,

no guarantee is provided as to actual water quantity or quality that may be obtained by a well on any particular parcel on the site.

Site Description

The site is located approximately 55 kilometers (km) east of the City of Edmonton within a mixed cropland, forested and country residential area of Lac Ste. Anne County. A portion of the 1:50,000 topographic map showing the topography and surface water features of the area is as follows:

Figure 2 - Topographic map showing subject location



The site is located in an area with gently undulating terrain at an elevation of approximately 730 meters above sea level (masl). Locally, topography dips slightly towards the east, towards the Kilini River which flows at an elevation of approximately 710 masl.

Geological Description

The nature of strata underlying the site was reviewed with the aid of the *Regional Groundwater Assessment of Lac Ste. Anne County* (Hydrogeological Consultants Ltd. 1998), *Surficial Geology of Wabamum Lake Alberta, NTS 83G* (Alberta Research Council, 1979). A review of the nearby water well records was also conducted to correlate the lithology record from the well with the published geological information. According to this assessment regional well capabilities from bedrock wells are on the order of 1.5 – 4.5 imperial gallons per minute.

The area is immediately underlain by approximately up to 20 m of quaternary age glaciofluvial deposits consisting of fine to coarse-grained sand and gravels. Also present are hummocky tills consisting of mixed sand, silt and clay with pebbles, cobbles and boulders. The presence of this formation may influence the migration of surface contaminants, such as septic field effluent, into underlying aquifers. Generally the gravels are dry or too poorly developed to serve as groundwater supply aquifers.

The Upper Horseshoe Canyon Formation of the Edmonton Group underlies the surficial deposits at the Site. This formation consists of interbedded sandstones, siltstones and mudstones and is classified as an aquifer. The Upper Horseshoe Canyon Formation is approximately 100 m in thickness and is underlain by the middle and lower sections of the Formation.

In the area, the Onoway Valley Channel represents a significant surficial aquifer, however this channel is located approximately 8 km north of the subject site and therefore cannot be expected to be used for water supply for the proposed subdivision.

Water Wells and Groundwater Usage in Area

A search of the Alberta Environment water well database indicates that 59 wells have been installed within 1.6 km of the site indicating a moderate amount of groundwater usage. Domestic Use well depths range from 2 – 129 m, with most wells obtaining water from the Upper Horseshoe Canyon aquifer from depths below 12 m to 129 m. Most wells are used for domestic purposes with some used for a combination of domestic and stock uses. Three oilfield monitoring wells were installed in NW-6-54-1 W5.

Initial well yields range from 2-20 igpm, with most well yields ranging around 5 – 10 igpm, indicating that moderate to good aquifers are generally encountered.

A review of Alberta Environment's water license and registration database was undertaken to determine large users of groundwater. A search of license and registrations of the subject section and adjoining eight sections was completed. One groundwater diversion licence was found within the area. Surface water diversion licenses are present in sections 11-54-2 W5, and 6&8-54-1 W5. Registrations are present in Sections 7-54-1 WSM. Overall groundwater usage in the area can be described as to moderate.

Wells on Subject Site

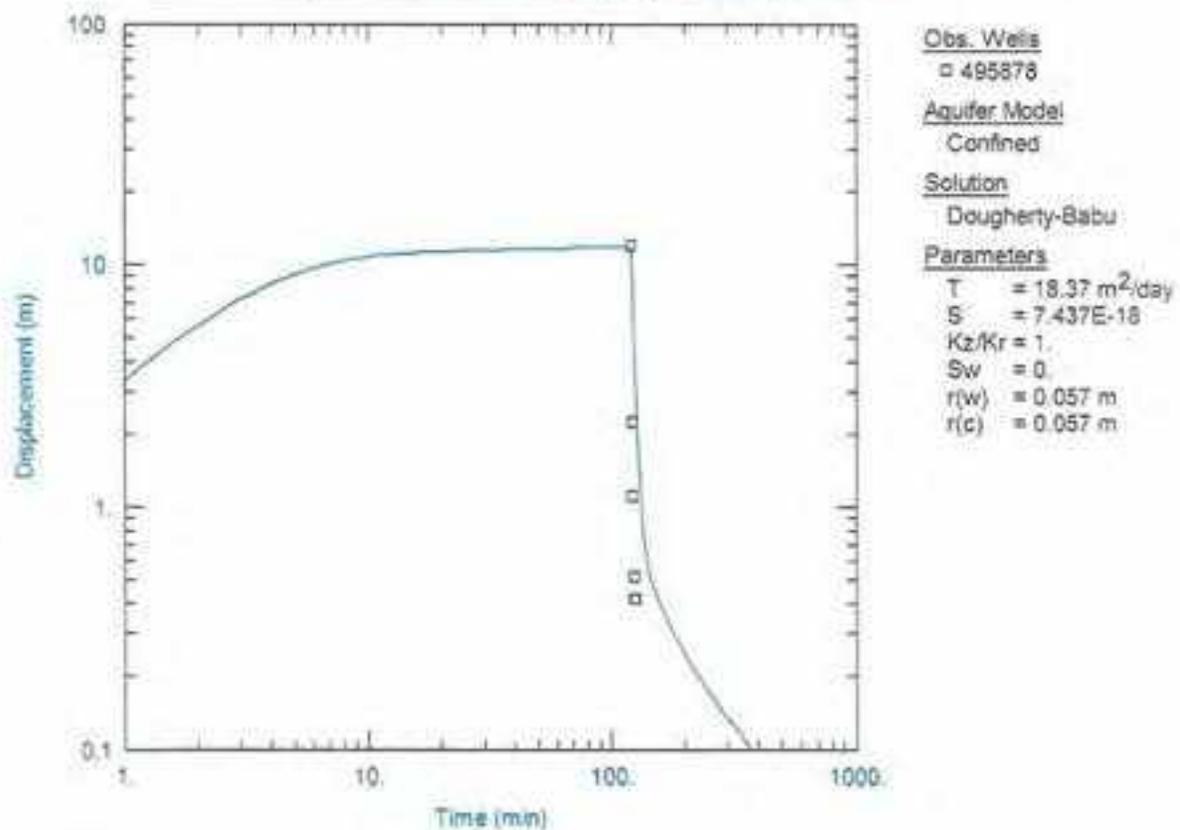
There are six domestic use wells listed within the same quarter section as the proposed subdivision, however their exact locations are unknown. All six wells are completed in the bedrock aquifer at depths of approximately 28-32 m. Static water levels in these wells ranged between 11.58 m and 20.73 m. Records indicate that water was removed from the wells at rates between 3.5 to 9 imperial gallons per minute (igpm), or 16 to 40 Litres per minute (L/min) indicating a productive aquifer.

Aquifer Parameters

The closest well with available data to allow calculation of aquifer parameters is 0495878, located within SE-01-54-2 W5. This well was installed in 1999 at a depth of 33 m, within the sandstones and shales of the Upper Horseshoe Canyon Formation. The well was pumped for 2 hours at a rate of 41 litres per minute (10 lpm), and data was collected during the recovery.

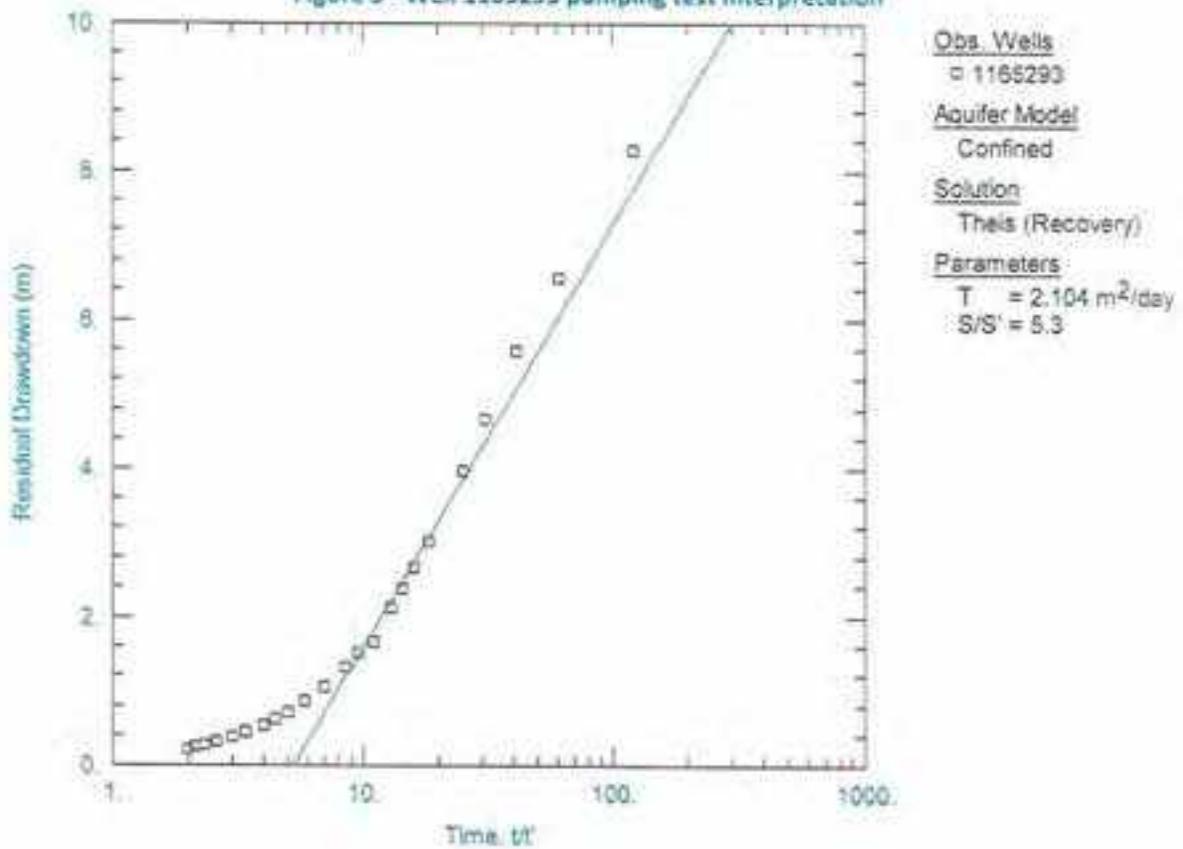
The pumping test data was interpreted with the aid of the AQTESOLV program developed by HydroSoft Inc. A confined radial flow model with full penetration and the Dougherty-Babu solution was applied. The water well drillers report for this well is attached in the appendix and a graph showing water displacement with time and the calculated aquifer parameters is as follows:

Figure 2 – Well 495878 Pumping Test Interpretation

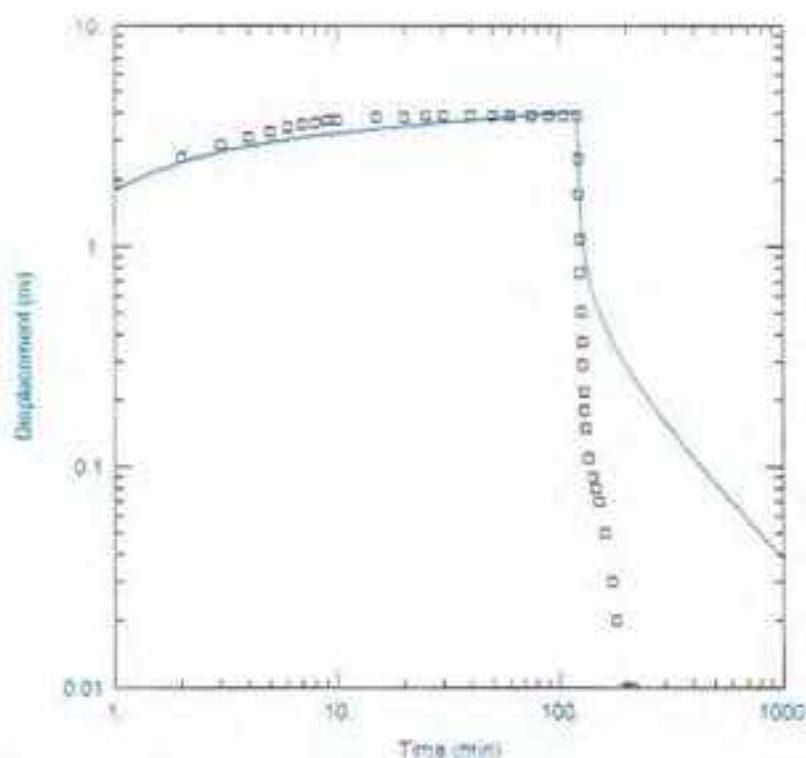


A 120 minute pumping test was undertaken on a well 01-04-54-02 W5 west of the site in 2007 (Well ID 1165293). This well also obtains water from the Horseshoe Canyon Aquifer from 56.08 – 79.25 m. The solution from the pumping test is as follows:

Figure 3 - Well 1165293 pumping test interpretation



A 120 minute pumping test followed by a 120 minute recovery test was undertaken on a well at SW-5-S4-1 WSM in 2012 (Well ID 1300394). This well also obtains water from the Horseshoe Canyon Aquifer from 58.83 – 73.15 m. A confined radial flow model with full penetration and the Dougherty-Babu solution was applied. The water well drillers report for this well is attached in the appendix and a graph showing water displacement with time and the calculated aquifer parameters is as follows:



Obs. Wells
 □ 1300394
 Aquifer Model
 Confined
 Solution
 Dougherty-Babu
 Parameters
 T = 11.98 m²/day
 S = 0.0009309
 Kz/Kr = 1
 Sw = 0
 r(w) = 0.0667 m
 r(o) = 0.054 m

Transmissivity values range from 2 to 18 m²/day from these tests. The average available head of the three wells is 12.7 m. The long term yield of a well, assuming no well bore losses, can be calculated by utilizing the Cooper and Jacob non-equilibrium equation:

$$Q = \frac{TH}{0.183} / \log (2.25 T \times t / r_w^2 \times S)$$

Where

- Q = Pumping rate
- H = Available Head (12.7 m)
- T = Transmissivity (Average of 10 m²/day)
- S = Aquifer Storativity (5 x 10⁻³ – representative of the Horseshoe Canyon Formation)
- t = Time (20 years or 7305 days)
- r_w = Well bore radius (0.07 m)

A 20 year yield of 66 m³/day (24,200 m³/year) is calculated. Alberta Environment and Sustainable Resource Development recommends a safety factor of 0.7 be applied, such that an average safe yield of 16,940 m³/year (7.1 imperial gallons per minute) is calculated. These values show that the average well should be able to supply sufficient water for individual lots.

The values indicated that the average well in the area may have sufficient capability to provide enough water to be used as a community well, however a fair risk exists that there may be insufficient capability for a community supply well.

Water Quality

Water quality analyses reports from the Alberta Environment and Parks water well were examined to illustrate water quality in Upper Horseshoe Canyon Formation. Chemical analysis reports from a well in SW-01-054-02 WSM (well ID 446859) and NW-36-053-02 WSM (well ID 458677), which are screened through the Upper Horseshoe River aquifer, were collected. The laboratory reports are attached in Appendix 3 with a summary of the results is as follows:

Table 1 - Water Quality Analysis

Parameter	446859	458677	Drinking Water Quality
Lab pH	N/A	N/A	6.5 – 8.5
Lab Electrical Conductivity	581	779	
Calcium	56	80	
Magnesium	10	16	
Sodium	63	78	200
Potassium	1	2	
Iron	<1	2	0.3
Manganese	N/A	N/A	0.05
Chloride	3	<1	250
Fluoride	<1	<1	1.5
Nitrate	N/A	N/A	10
Sulfate	<1	22	500
Bicarbonate	346	496	
Total Dissolved Solids	315	443	500

All results in mg/L except conductivity in $\mu\text{S}/\text{cm}$ and pH in pH units. N/A = Not Analyzed

The results show that the groundwater from both wells within the aquifer are a sodium bicarbonate type water with total dissolved solids concentration of less than 500 mg/L and should generally be acceptable for drinking water use.

It is recommended that water samples be collected and analyzed for routine dissolved salts and bacterial parameters from each well prior to use as a potable supply.

Reconnaissance Report

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WELL ID	LD	SEC	TWP	RGE	M	DRILLING COMPANY	DATE COMPLETED	DEPTH (m)	TYPE OF WORK	USE	CHM. LT	PT	WELL OWNER	STATIC LEVEL (m)	TEST RATE (L/min)	SC. DIAM (m)	
452526	NW	31	053	01	5	GERALD MCGINN DRILLING LTD.	1974-10-25	10.67	New Well	Domestic	7		MILLER, GORDON	3.35	20.55	11.43	
452526	NW	31	053	01	5	GERALD MCGINN DRILLING LTD.	1970-06-05	12.20	New Well	Domestic	5		MILLER, GORDON	5.49	45.46	0.00	
452527	NW	31	053	01	5	GERALD MCGINN DRILLING LTD.	1978-09-15	46.94	New Well	Domestic	15		MILLER, GORDON	4.27	34.10	11.43	
452528	NE	31	53	1	5	BIG IRON DRILLING LTD.	1975-09-03	57.91	New Well	Domestic	8		STEFFLER, CLAYTON	13.72	22.73	14.32	
452528	NE	31	53	1	5	UNKNOWN DRILLING COMPANY	57.91	Old Well - Abandoned			1		STEFFLER, ROBERT				
452529	NE	31	053	01	5	UNKNOWN DRILLER	12.10	Chemistry		Domestic	1		WERNICOMBS, R.	9.45		0.00	
452530	NE	31	053	01	5	GERALD MCGINN DRILLING LTD.	1978-10-24	63.40	New Well	Domestic	1	8	STEFFLER, DOUG	0.00	11.37	11.43	
452531	NE	31	053	01	5	UNKNOWN DRILLER	63.10	Chemistry		Domestic	1		STEFFLER, D.	56.08		0.00	
452533	NE	31	053	01	5	GERALD MCGINN DRILLING LTD.	1979-04-12	129.24	New Well	Domestic	24		STEFFLER, DOUG	45.72	6.82	11.43	
452534	NE	31	053	01	5	GERALD MCGINN DRILLING LTD.	1979-10-13	60.66	New Well	Domestic	10		GAGNON, LOI	24.28	15.91	11.43	
452535	NE	31	053	01	5	GERALD MCGINN DRILLING LTD.	1984-05-28	28.04	New Well	Domestic	8		STEFFLER, CLAYTON	0.00	15.01	11.58	
452536	NE	31	053	01	5	MCCALLY DRILLING CO. LTD.	1977-12-21	95.10	New Well	Domestic	11					11.58	
452537	NE	31	053	01	5	UNKNOWN DRILLER	1076-03-12	71.25	Chemistry		Domestic	1		CONNER, L.D.			0.00
452538	NE	31	053	01	5	DND WATER WELL DRILLING & SERVICING LTD.	1995-07-19	67.78	New Well	Domestic	10	16	GODOL, LEONARD	43.59	90.92	15.24	
452538	SE	01	054	02	5	REDWOOD DRILLING	1999-09-14	32.92	New Well	Domestic	11	7	DAIR, IEO	15.40	40.91	15.24	
452538	NW	31	053	01	5	DND WATER WELL DRILLING & SERVICING LTD.	2000-09-26	23.16	New Well	Domestic	3	10	STEFFLER, ROBERT	12.19	68.19	15.24	
1715115	NE	31	053	01	5	SUMMERS DRILLING LTD.	2000-10-23	82.48	New Well	Domestic	6	25	BELMERT, EDWARD	16.00	18.18	15.24	
1716143	NW	6	54	1	5	SUMMERS DRILLING LTD.	2015-02-11	15.24	Reconnaissance	Monitoring	3		LAFARGE CANADA INC.				
1716143	NW	6	54	1	5	SUMMERS DRILLING LTD.	2015-02-11	9.45	Reconnaissance	Monitoring	3		LAFARGE CANADA INC.				
1716145	NW	6	54	1	5	SUMMERS DRILLING LTD.	2015-02-11	7.90	Reconnaissance	Monitoring	3		LAFARGE CANADA INC.				

Reconnaissance Report

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WELL ID	LSO	SEC	TWP	RGE	M	DRILLING COMPANY	DATE COMPLETED	DEPTH (m)	TYPE OF WORK	USE	CNH	LY	PT	WELL OWNER	STATIC LEVEL (m)	TEST RATE (L/Min)	SC DIAM (m)
455505	NW	31	053	01	5	GERALD MCGINN DRILLING LTD.	1974-10-25	10.67	New Well	Domestic		2		MILLER, GORDON	3.26	29.55	11.43
455506	NW	31	053	01	5	GERALD MCGINN DRILLING LTD.	1970-06-05	12.80	New Well	Domestic		5		MILLER, GORDON	5.49	45.46	0.00
455507	NW	31	053	01	5	GERALD MCGINN DRILLING LTD.	1978-09-15	46.54	New Well	Domestic		35		MILLER, GORDON	4.27	34.10	11.43
455508	NE	31	53	1	5	BIG ROCK DRILLING LTD.	1975-09-03	52.91	New Well	Domestic		0		STEFFLER, CLAYTON	13.22	22.23	14.12
455509	NE	31	53	1	5	UNKNOWN DRILLING COMP11	52-01	Old Well	Abandoned	Unknown		4		STEFFLER, WARREN			
455510	NE	31	053	01	5	UNKNOWN DRILLER		12.39	Chemistry	Domestic	1	1		WATKINSON, M.	0.40		0.00
455511	NE	31	053	01	5	GERALD MCGINN DRILLING LTD.	1978-10-24	63.40	New Well	Domestic	1	8		STEFFLER, DOUG	0.00	13.32	11.43
455512	NE	31	053	01	5	UNKNOWN DRILLER		62.18	Chemistry	Domestic	1	1		STEFFLER, D.	56.08		0.00
455513	NE	31	053	01	5	GERALD MCGINN DRILLING LTD.	1979-04-12	126.24	New Well	Domestic		24		STEFFLER, DOUG	45.72	6.80	11.43
455514	NE	31	053	01	5	GERALD MCGINN DRILLING LTD.	1979-10-13	60.66	New Well	Domestic		10		GAGNAUK, LOH	24.38	15.01	11.43
455515	NE	31	053	01	5	GERALD MCGINN DRILLING LTD.	1984-05-28	28.04	New Well	Domestic		8		STEFFLER, CLAYTON	0.00	15.91	11.28
455516	NE	31	053	01	5	HOCKLEY DRILLING CO. LTD.	1977-12-31	95.10	New Well	Domestic		11					11.28
455517	SE	31	053	01	5	UNKNOWN DRILLER	1000-01-11	20.25	Chemistry	Domestic	1	1		COOPER, J.D.			0.00
455518	NE	31	053	01	5	OLD WATER WELL DRILLING & SERVICING LTD.	1995-07-19	82.78	New Well	Domestic		10		DE GOSU, LEONARD	41.59	98.02	15.24
455519	SE	01	054	02	5	RODCO DRILLING	1999-09-14	32.92	New Well	Domestic		11		7 BUSE, LEO	15.28	40.91	15.24
455520	NW	31	053	01	5	OLD WATER WELL DRILLING & SERVICING LTD.	2000-09-26	23.16	New Well	Domestic		3		10 STEFFLER, ROBERT	11.19	68.19	15.24
1210115	NE	31	053	01	5	SUMMERS DRILLING LTD.	2000-10-23	62.48	New Well	Domestic		6		25 BELANGER, EDWARD	16.00	18.10	15.24
1210113	NW	6	54	1	5	SUMMERS DRILLING LTD.	2015-03-11	15.24	Permeability	Permeability		3		LAFAIRE CANADA INC			
1210114	NW	6	54	1	5	SUMMERS DRILLING LTD.	2015-03-11	9.45	Permeability	Permeability		3		LAFAIRE CANADA INC			
1210115	NW	6	54	1	5	SUMMERS DRILLING LTD.	2015-03-11	2.92	Permeability	Permeability		3		LAFAIRE CANADA INC			

Reconnaissance Report

View in Imperial
Export to Excel

WELL ID	LSD	SEC	TWP	RGE	M	DRILLING COMPANY	DATE COMPLETED	DEPTH (m)	TYPE OF WELLS	USE	COM	LT	PT	WELL OWNER	STATIC LEVEL (m)	TEST RATE (L/mo)	SC-DIAM (m)
455025	NW	21	053	01	5	GERALD MCGINN DRILLING LTD.	1974-10-25	10.67	New Well	Domestic		7		MILLER, GORDON	3.35	20.55	11.43
455026	NW	21	053	01	5	GERALD MCGINN DRILLING LTD.	1970-05-05	12.80	New Well	Domestic		5		MILLER, GORDON	5.49	45.46	0.00
455027	NW	21	053	01	5	GERALD MCGINN DRILLING LTD.	1978-09-15	46.04	New Well	Domestic		35		MILLER, GORDON	4.27	24.10	11.43
455028	NE	21	53	1	5	WIC IRON DRILLING LTD.	1975-09-03	57.51	New Well	Domestic		8		STEFFLER, CLAYTON	13.72	22.73	14.12
455029	NE	21	53	1	5	UNKNOWNDRILLING	57/81	Oil Well	Abandoned			1		STEFFLER, ROBERT			
455029	NE	21	053	01	5	UNKNOWNDRILLING	12/81	Chemistry		Domestic	1			WECROSKO, R.	9.95		0.00
455030	NE	21	053	01	5	GERALD MCGINN DRILLING LTD.	1978-10-24	63.40	New Well	Domestic		1	8	STEFFLER, DOGG	0.00	11.37	11.43
455031	NE	21	053	01	5	UNKNOWNDRILLING	02/58	Chemistry		Domestic	1			STEFFLER, D.	29.08		0.00
455032	NE	21	053	01	5	GERALD MCGINN DRILLING LTD.	1979-04-12	120.24	New Well	Domestic		24		STEFFLER, DOGG	45.72	6.82	11.43
455033	NE	21	053	01	5	GERALD MCGINN DRILLING LTD.	1979-10-13	65.66	New Well	Domestic		30		GAGNARIK, LOU	24.38	15.91	11.43
455034	NE	21	053	01	5	GERALD MCGINN DRILLING LTD.	1984-05-28	28.04	New Well	Domestic		8		STEFFLER, CLAYTON	0.00	15.94	11.58
455035	NE	21	053	01	5	MICALLEY DRILLING CO. LTD.	1977-12-21	95.10	New Well	Domestic		11					11.58
455036	NE	21	053	01	5	UNKNOWNDRILLING	1999-03-17	29.25	Chemistry		Domestic	1		COOPER, L.D.			0.00
455037	NE	21	053	01	5	DRG WATER WELL DRILLING & SERVICING LTD.	1995-07-19	87.70	New Well	Domestic		30	10	GOSAL, LEONARD	43.59	90.92	15.24
455038	SE	01	054	02	5	WOOD DRILLING	1999-09-14	22.02	New Well	Domestic		31	7	DURE, LEO	15.48	40.91	15.24
455039	NW	21	053	01	5	DRG WATER WELL DRILLING & SERVICING LTD.	2000-09-26	23.16	New Well	Domestic		3	10	STEFFLER, ROBERT	12.19	68.19	15.24
1215115	NE	21	053	01	5	SUPPERS DRILLING LTD.	2000-10-23	62.46	New Well	Domestic		6	25	BEAUFORT, EDWARD	16.00	16.18	15.24
1215113	NW	6	54	1	5	SUPPERS DRILLING LTD.	2015-02-11	15.24	Parameter	Parameter		3		LAFARGE CANADA INC.			
1215144	NW	6	54	1	5	SUPPERS DRILLING LTD.	2015-02-11	9.95	Parameter	Parameter		3		LAFARGE CANADA INC.			
1215145	NW	6	54	1	5	SUPPERS DRILLING LTD.	2015-02-11	7.92	Parameter	Parameter		3		LAFARGE CANADA INC.			

LEGAL LAND DESCRIPTION
 (F 04-64-02-1648)



LEGEND

- SECTION
- SECTION
- SECTION

LOCATION PLAN

LOCAL HIGHWAY - 1ST
 WYOM. RD. - 80-4000

ASPIEN CABLE
 PHOTOGRAPHIC RECORDS

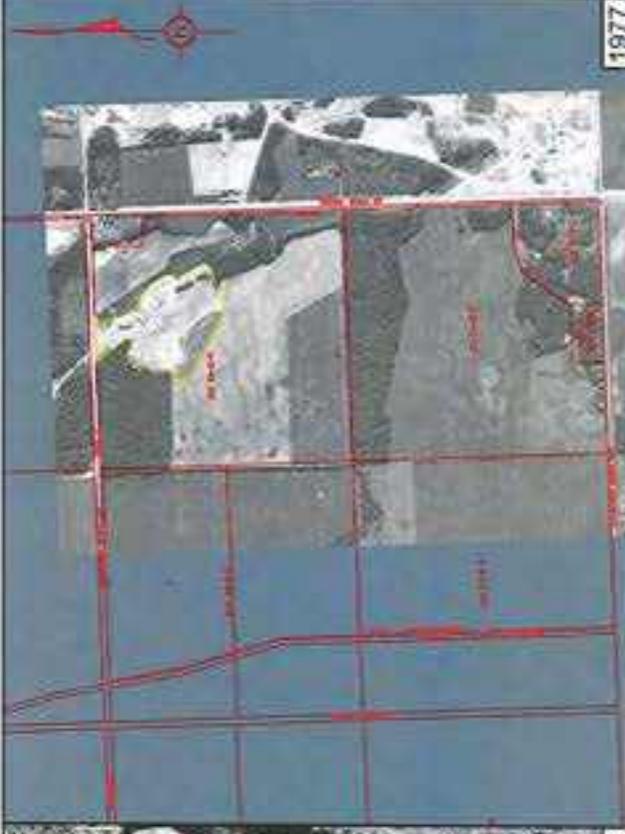
YEAR	AGES	RECORDS
1971	1A	1A
1977	1A/B	2A/B
1981	1A/B	2A/B
1985	1A/B	2A/B



WESTROCK AGGREGATES LTD.

YECMAN PARKER PIT
 PHOTOGRAPHIC RECORDS
 AERIAL PHOTOGRAPHY
 HISTORICAL OVERVIEW
 1971 - 1985

Project No. 100000
 Drawing No. 4-11
 Scale 1:10000
 Date 10/11/00
 Author J. J. J.
 Designer J. J. J.
 Checker J. J. J.
 Date 10/11/00



1977



1973



1985



1981

LEGAL LAND DESCRIPTION
01-54-02-9090



LOCAL WESTROCK LAND
PARCEL 01 - 04-00-0000

LEGEND

- LOCATIONS
- HISTORICAL
- PROPOSED

LOCATIONS

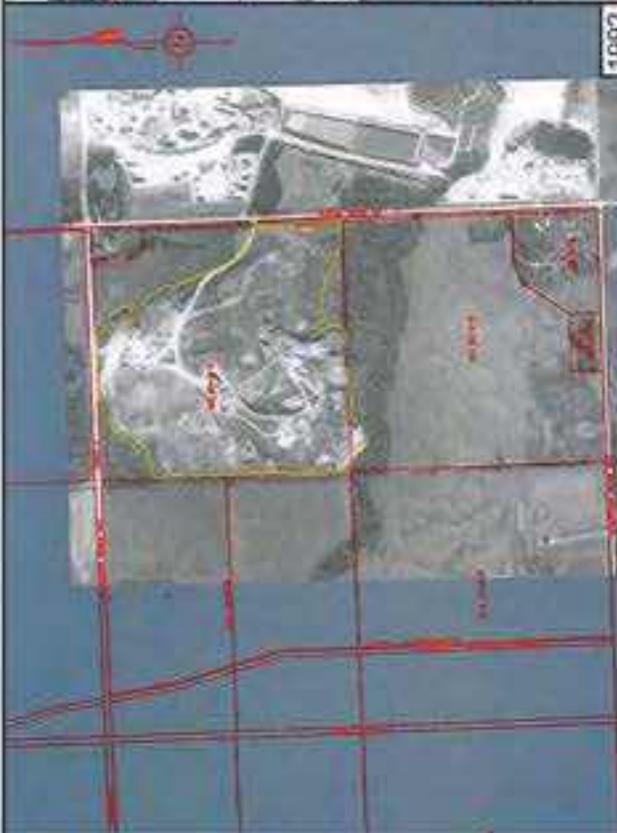
YEAR	ACRES	PERCENTAGE
1992	10.24	48.34
1994	10.24	48.34
2000	10.24	48.34
2005	10.24	48.34

ASPIEN
LAND GROUP INC.

WESTROCK AGGREGATES LTD.

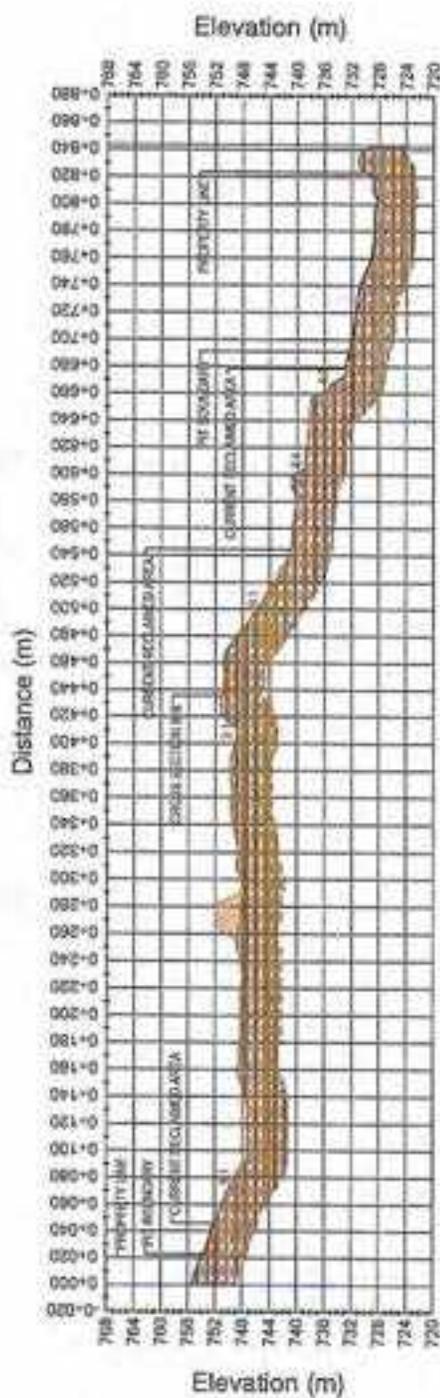
YEOMAN PARKER PIT
INVESTMENT APPLICATION
AERIAL PHOTOGRAPHY
HISTORICAL OVERVIEW
(1992 - 2005)

Project No. 1000000
Map No. 1000000
Scale 1:5000
Date 5-11-05



SCALE:
V=1:700
H=1:3000

Cross Section A-A' Existing



LEGAL LAND DESCRIPTION
(E 61-54-03 900M)



LEGEND

[Hatched Box]	AREA
[Dotted Box]	EXISTING RECLAIMED AREA
[Dotted Box]	EXISTING LAND AREA
[Dotted Box]	PROPERTY LINE
[Dotted Box]	PROPOSED PI

LOCATION PLAN



WESTROCK AGGREGATES LTD.

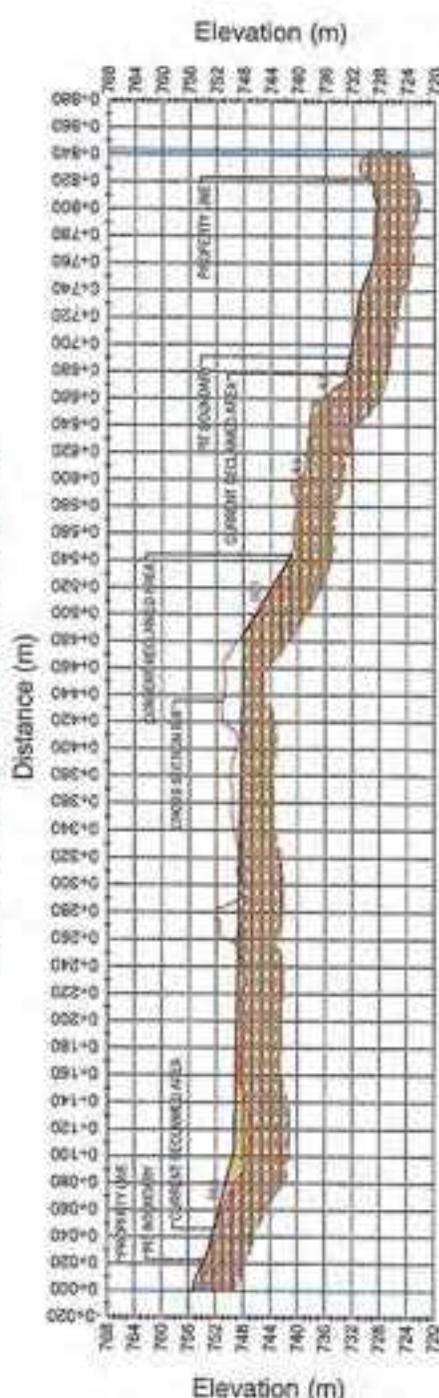
Project: YEOMAN PARKER PIT
PROPOSED RECLAIMATION

CROSS SECTION A-A'
EXISTING AND RECLAIMED CONDITIONS

Drawn By: L. J. ...
Checked By: ...
Scale: V=1:700
H=1:3000
Date: ...
Sheet No. 6-11

SCALE:
V=1:700
H=1:3000

Cross Section A-A' Reclaimed



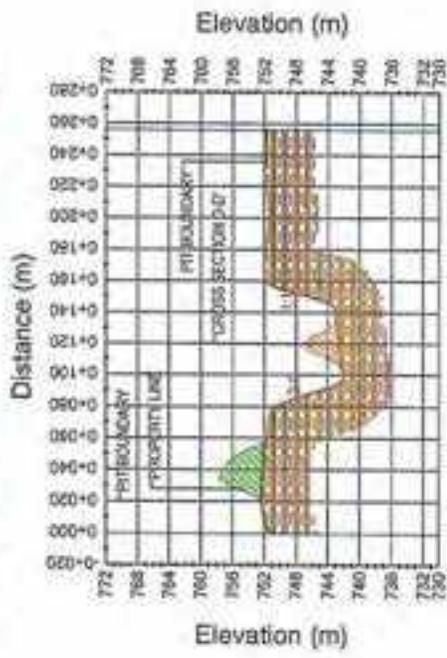
SCALE:
V=1:500
H=1:3000

LEGAL LAND DESCRIPTION
01-54-03 100M



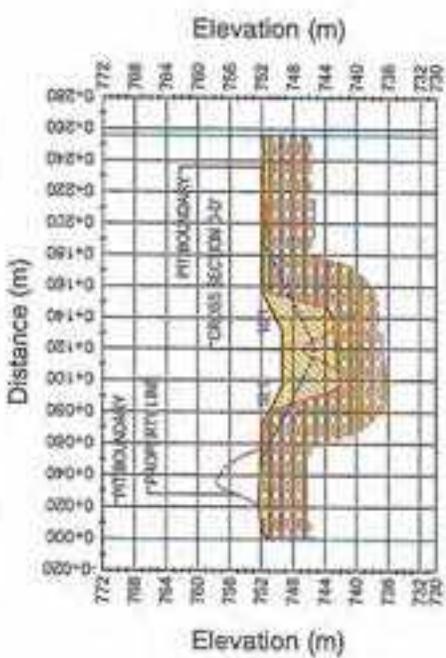
- LEGEND
- BOUNDARY
 - PROPERTY LINE
 - EXISTING GRAVEL PAVEMENT
 - EXISTING ASPHALT PAVEMENT
 - EXISTING CONCRETE PAVEMENT
 - EXISTING GRAVEL DRIVE
 - EXISTING ASPHALT DRIVE
 - EXISTING CONCRETE DRIVE
 - EXISTING GRAVEL DRIVE
 - EXISTING ASPHALT DRIVE
 - EXISTING CONCRETE DRIVE
 - EXISTING GRAVEL DRIVE
 - EXISTING ASPHALT DRIVE
 - EXISTING CONCRETE DRIVE

Cross Section C-C' Existing



SCALE:
V=1:500
H=1:3000

Cross Section C-C' Reclaimed



WESTROCK AGGREGATES LTD.

YEOMAN PARKER PIT
REGULATORY APPLICATION

CROSS SECTION C-C'
EXISTING AND RECLAIMED CONDITIONS

Project No. 2011-01-001
Scale: AS2 300MM
Drawing No. B-11
Date: 08/11/11

SCALE:
V=1:500
H=1:3000

LEGAL LAND DESCRIPTION
R/F 61-54-62 W/45
LEGAL SUBDIVISION-LTD
W/45 L/20 - 40 ACRES

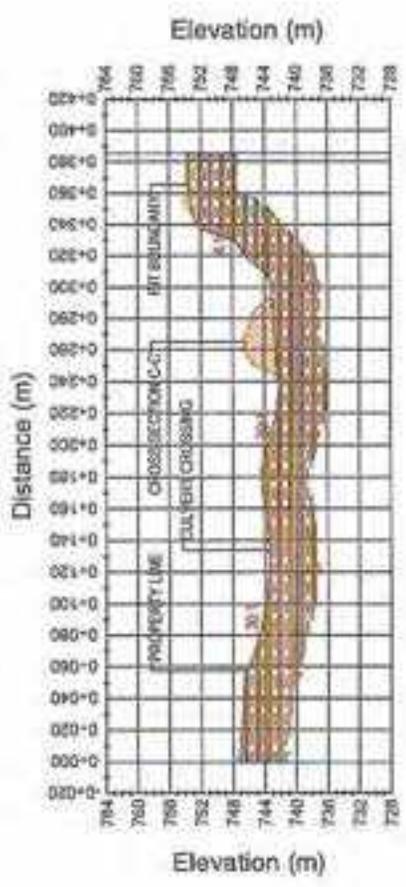


LEGEND

- WATER
- EXISTING GRADE
- EXISTING ROAD/PAVEMENT/ASPH/CONC
- EXISTING/NEW/PROPOSED SIDEWALKS
- EXISTING/PROPOSED VEGETATION
- EXISTING/PROPOSED CONCRETE
- EXISTING/PROPOSED ASPHALT

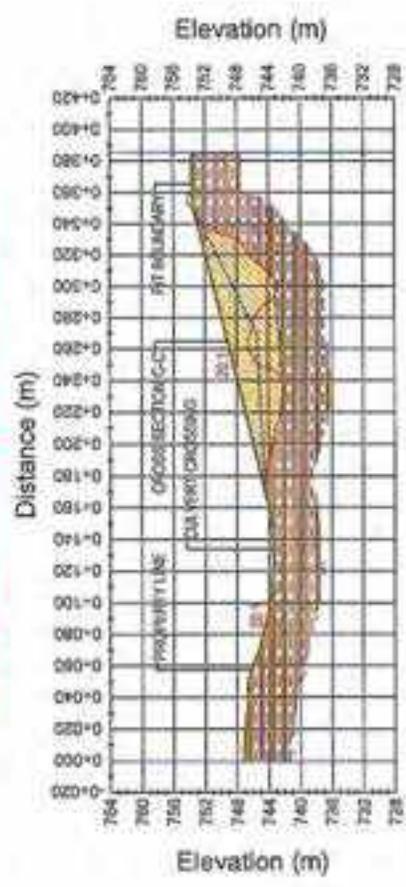
LOCATION PLAN

Cross Section D-D' Existing



SCALE:
V=1:500
H=1:3000

Cross Section D-D' Reclaimed



Prepared by: **ASPEN**
LAND GROUP INC.

Client: **WESTROCK AGGREGATES LTD.**

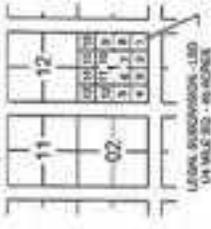
Project: **YEOMAN PARKER PIT**
RECONSTRUCTION APPLICATION

Site: **CROSS SECTION D-D'**
EXISTING AND RECLAIMED CONDITIONS

Checked by: J. L. ... Date: 05/20/2018
Drawing No.: **9-11**
Scale: 1:500
Sheet: 9-11

LEGAL LAND DESCRIPTION

E 01 54 00 W006



LEGAL DESCRIPTION - LEO
VA M.L.S. 80 - 90 - 40000

LOCATION PLAN

- LEGEND**
- EXISTING
 - WESTROCK PITS
 - WESTROCK PITS BUFFER ZONE
 - EXISTING ROADWAY
 - PROPOSED ROADWAY
 - PROPOSED FENCE LINE
 - PROPERTY BOUNDARY



WESTROCK AGGREGATES LTD.

Project: YEOMAN PARKER PIT
REGISTRATION APPLICATION

PLAN SHOWING EXISTING PROPOSED RECLAMATION LOCATIONS

Author: J. [unclear]	Scale: 1:75000
Approved By: [unclear]	Working No.
Date: [unclear]	
AS PLOTTED: [unclear]	
Date: [unclear]	



OWNERS

WESTROCK AGGREGATES LTD.
OF 10410 - 81 AVENUE, EDMONTON
ALBERTA T6E 1X5

ENCUMBRANCES, LIENS & INTERESTS

REGISTRATION

NUMBER	DATE (D/M/Y)	PARTICULARS
882 016 255	26/01/1988	MORTGAGE MORTGAGEE - PROVINCE OF ALBERTA TREASURY BRANCHES. 1764, 8770 - 170 STREET, EDMONTON ALBERTA T5T4J2 ORIGINAL PRINCIPAL AMOUNT: \$1,000,000
892 276 385	25/10/1989	MORTGAGE MORTGAGEE - PROVINCE OF ALBERTA TREASURY BRANCHES. 1764, 8770 - 170 STREET, EDMONTON ALBERTA T5T4J2 ORIGINAL PRINCIPAL AMOUNT: \$750,000
992 299 717	05/10/1999	UTILITY RIGHT OF WAY GRANTEE - STE ANNE NATURAL GAS CO-OP LIMITED.

TOTAL INSTRUMENTS: 003

THE REGISTRAR OF TITLES CERTIFIES THIS TO BE AN
ACCURATE REPRODUCTION OF THE CERTIFICATE OF
TITLE REPRESENTED HEREIN THIS 26 DAY OF AUGUST,
2013 AT 02:13 P.M.

ORDER NUMBER: 24248750

CUSTOMER FILE NUMBER:



END OF CERTIFICATE

(CONTINUED)

THIS ELECTRONICALLY TRANSMITTED LAND TITLES PRODUCT IS INTENDED FOR THE SOLE USE OF THE ORIGINAL PURCHASER, AND NONE OTHER, SUBJECT TO WHAT IS SET OUT IN THE PARAGRAPH BELOW.

THE ABOVE PROVISIONS DO NOT PROHIBIT THE ORIGINAL PURCHASER FROM INCLUDING THIS UNMODIFIED PRODUCT IN ANY REPORT, OPINION, APPRAISAL OR OTHER ADVICE PREPARED BY THE ORIGINAL PURCHASER AS PART OF THE ORIGINAL PURCHASER APPLYING PROFESSIONAL, CONSULTING OR TECHNICAL EXPERTISE FOR THE BENEFIT OF CLIENT(S).

Appendix B: Schedules under the *Code of Practice for Pits*

**Code of Practice for Pits
Registration Application (Schedule 1)**

Date: October 11, 2013 (Revised December 27, 2013)

Previous *Environmental Protection and Enhancement Act* Registration Number: SG-39-86
Water Act authorization required? Yes No

If Yes, application submitted and current *Water Act* Authorization Numbers: application submitted

Name of Applicant (company or person in whose name the pit will be registered):

Company: Westrock Aggregates Ltd.
Address: 10410 - 81 Ave Edmonton, AB T6E 1X5
Phone: (780) 434-8555 Facsimile: (780) 438-1390
e-mail: thomasfath@ohanlonpaving.com

Name of Person Submitting Application:

Name: Lesley Foy
Company Name: Aspen Land Group Inc.
Job Title: Senior Agrologist
Address: #201 18311 - 105 Avenue, Edmonton, Alberta T5S 2K9
Phone: (780) 667-7081 Facsimile: Not Available
e-mail: lfoy@aspenlandgroup.com

Signature: _____



Name of Primary Contact for Pit:

Name: Dean Maurier
Address: 10410 - 81 Ave Edmonton, AB T6E 1X5
Phone: (780) 434-8555 Facsimile: (780) 438-1390
e-mail: dmaurier@fathindustries.com

Pit Location Municipal Address or LSD-Sec-Twp-Rge-Mer	Registered Owners Name, Address and Phone Number	Occupants Name, Address and Phone Number
NE 1-54-2-W5M	Westrock Aggregates Ltd. 10410 – 81 Ave Edmonton AB, T6E 1X5	N/A
SE 1-54-2-W5M	Charles and Melanie Parker Box 43, Site 320, RR#3 Stony Plain AB, T7Z 1X3 780-967-5443	N/A

Activities Plan (Schedule 2)

Part 1 Information

Aggregate Type (check off all that apply): Gravel Sand Clay Marl

Current Size of Pit: 56.29 ha

Average Thickness (indicate metres or centimetres for each one):

Topsoil: 6-8 inches*

Subsoil: n/a*

Overburden: 10 - 25 feet*

Aggregate: 25 – 50 feet*

**Information obtained from Yeoman Construction Ltd. – June 1985 Sand and Gravel Pit Inventory Report*

Topsoil Texture (check all that apply):

organic soil mineral soil clay loam silty loam sand sandy loam
 loam clay silt other

Description of techniques to prevent wind and water erosion, and to limit the movement of dust from the pit: Refer to Section 3.4 of the report.

Participation in local or regional air monitoring initiative: n/a

Inactive pit conservation and reclamation techniques: All inactive pit slopes will be backsloped to a 2:1 slope.

Scale drawings and cross-sections of existing pit conditions and planned sequence of operation attached.

Part 2 Information

Maximum planned size of pit: 56.29 ha

Depth to groundwater (metres) in test holes (indicate each depth if multiple holes): n/a

Planned activities at the pit (check off all that apply): wet pit excavation
 concrete production mixing salt and aggregate mixing recycled asphalt and concrete with aggregate spraying truck boxes aggregate washing use of alternative materials for reclamation other: reclamation and importing topsoil and overburden material

Mitigative measures for all of the above activities: Refer to Section 3.7 of the report.

Proposed land uses for reclaimed pit (check all appropriate boxes):

<input type="checkbox"/> cultivation _____ %	<input checked="" type="checkbox"/> hayland <u>92.4</u> %	<input type="checkbox"/> pasture _____ %
<input type="checkbox"/> native range _____ %	<input type="checkbox"/> grassland _____ %	<input type="checkbox"/> forest _____ %
<input type="checkbox"/> wildlife habitat _____ %	<input type="checkbox"/> waterbody _____ %	<input type="checkbox"/> proposed subdivision _____ %
<input checked="" type="checkbox"/> other <u>Processing Site & Access Roads - 7.6</u> %		

Pit water release (rationale for release, techniques and discharge points): n/a

Yeoman Property

Average topsoil replacement depth (cm): approximately 2 cm

Average subsoil replacement depth (cm): no subsoil available for replacement

Parker Property

Average topsoil replacement depth (cm): 18 cm

Average subsoil replacement depth (cm): 9 cm

Scale drawings and cross-sections of reclaimed pit conditions attached.

Description of surface water bodies in the reclaimed pit: n/a

Design: n/a

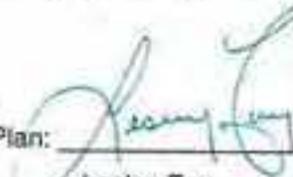
Intended use: n/a

Water elevations at full supply level: n/a

Slope of land one metre above and one metre below full supply level: n/a

Slope of land one metre below full supply level: n/a

Signature and title of person who developed Activities Plan: _____


Lesley Foy
Senior Agrologist

Security Estimate (Schedule 3)

Acres of land secured at \$250/acre: _____ Acres x \$250 = \$ _____ (a)

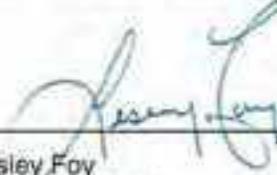
Detailed full-cost security calculation attached Total full-cost = \$190,705.46 (b)

Area of land at full-cost: 52.03 (hectares/acres) Cost/hectare = \$3,865.30

Total security required ((a) + (b)): \$190,705.46

Previous method of payment: Letter of Credit Cash Other (explain) _____

Signature and title of person submitting estimate: _____


Lesley Foy
Senior Agrologist

See Section 4.0 of the report for the full-cost security calculation.

Appendix C: Conservation and Reclamation Plans



LEGAL LAND DESCRIPTION
 S1 01-04-02 W54N

11	12
01	02
03	04
05	06
07	08
09	10
11	12

LEGAL DESCRIPTION: S1 01-04-02 W54N
 100 ACRES (4048.58 SQ. METERS)

LEGEND

- PROPERTY
- BOUNDARIES

LOCATION SUMMARY

ASBURLAND COUNTY

541
540



WESTROCK AGGREGATES LTD.

YEOMAN PARKER PIT
 PROPOSED OPERATIONS

LOCATION PLAN

Project No. 1000000000
 Drawing No. 1000000000
 Scale: 1:10000
 Date: 1-11



LEGAL LAND DESCRIPTION
 (P 21-54-02 W3M)

11	12
02	03

LOCAL SUBDIVISION - L2D
 1/4 1/4 1/4 1/4 1/4 1/4 1/4 1/4

LEGEND

- PROPERTY
- BOUNDARIES

LOCATION/SCALE



WESTROCK AGGREGATES LTD.

YEOMAN PARKER PIT
 RECONSTRUCTION APPROVAL

CONTOUR PLAN

Scale: 1:20000
 Drawing No. 2-11

